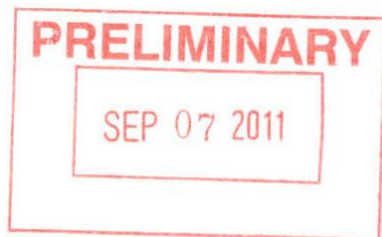
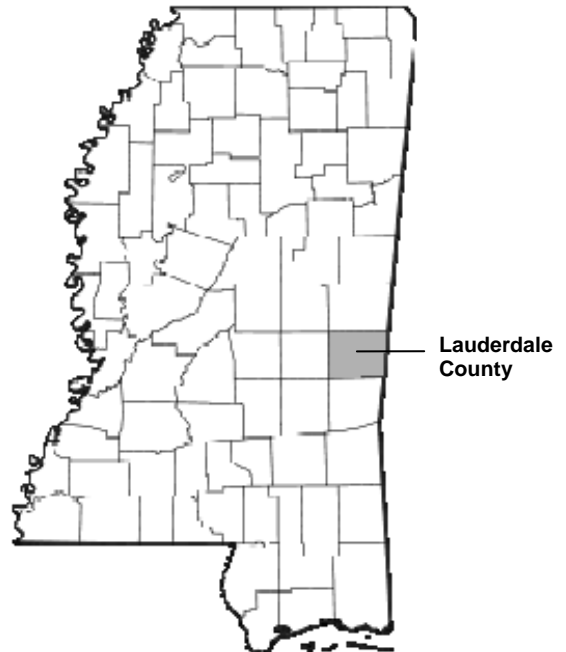


FLOOD INSURANCE STUDY



LAUDERDALE COUNTY, MISSISSIPPI AND INCORPORATED AREAS

Community Name	Community Number
LAUDERDALE COUNTY (UNINCORPORATED AREAS)	280224
MARION, TOWN OF	280095
MERIDIAN, CITY OF	280096



REVISED



Federal Emergency Management Agency

FLOOD INSURANCE STUDY NUMBER
28075CV000B

**NOTICE TO
FLOOD INSURANCE STUDY USERS**

Communities participating in the National Flood Insurance Program have established repositories of flood hazard data for floodplain management and flood insurance purposes. This Flood Insurance Study may not contain all data available within the repository. It is advisable to contact the community repository for any additional data.

This preliminary revised Flood Insurance Study contains profiles presented at a reduced scale to minimize reproduction costs. All profiles will be included and printed at full scale in the final published report.

Part or all of this Flood Insurance Study may be revised and republished at any time. In addition, part of this Flood Insurance Study may be revised by the Letter of Map Revision process, which does not involve republication or redistribution of the Flood Insurance Study. It is, therefore, the responsibility of the user to consult with community officials and to check the community repository to obtain the most current Flood Insurance Study components.

Initial Countywide FIS Effective Date: September 29, 1989

Revised Countywide FIS Date: August 16, 1995
March 21, 2000
February 3, 2010

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Exhibit 2 – Flood Insurance Rate Map Index (Published Separately) Flood Insurance Rate Maps (Published Separately)

FLOOD INSURANCE STUDY
LAUDERDALE COUNTY, MISSISSIPPI AND INCORPORATED AREAS

1.0 INTRODUCTION

1.1 Purpose of Study

This Flood Insurance Study (FIS) revises and updates information on the existence and severity of flood hazards in the geographic area of Lauderdale County, including the incorporated areas of the City of Meridian, the Town of Marion, and the unincorporated areas of Lauderdale County (referred to collectively herein as Lauderdale County), and aids in the administration of the National Flood Insurance Act of 1968 and the Flood Disaster Protection Act of 1973. This study has developed flood-risk data for various areas of the community that will be used to establish actuarial flood insurance rates and to assist the community in its efforts to promote sound floodplain management. This information will also be used by Lauderdale County to update existing floodplain regulations as part of the Regular Phase of the National Flood Insurance Program (NFIP). Minimum floodplain management requirements for participation in the National Flood Insurance Program (NFIP) are set forth in the Code of Federal Regulations at 44 CFR, 60.3.

In some states or communities, floodplain management criteria or regulations may exist that are more restrictive or comprehensive than the minimum Federal requirements. In such cases, the more restrictive criteria take precedence, and the State (or other jurisdictional agency) will be able to explain them.

The Digital Flood Insurance Rate Map (DFIRM) and FIS Report for this countywide study have been produced in digital format. Flood hazard information was converted to meet the Federal Emergency Management Agency (FEMA) DFIRM database specifications and geographic information standards and is provided in a digital format so that it can be incorporated into a local Geographic Information System and be accessed more easily by the community.

1.2 Authority and Acknowledgments

The sources of authority for this FIS report are the National Flood Insurance Act of 1968 and the Flood Disaster Protection Act of 1973.

The hydrologic and hydraulic analyses for the initial September 29, 1989 countywide FIS were performed by the U.S. Army Corps of Engineers (USACE), Mobile District, for FEMA, under Inter-Agency Agreement No. EMW-85-E-1822. That work was completed in March 1987.

The hydrologic and hydraulic analyses for the August 16, 1995 FIS revision of Sowashee Creek, Gallagher Creek, Magnolia Creek, Nanabe Creek, Robbins Branch, and Shear's Branch were prepared by the USACE, Mobile District. That work was completed in January 1993.

The hydrologic and hydraulic analyses for the March 21, 2000 revision, of Gallagher Creek, Magnolia Creek, Robbins Branch, Harbour Creek, and Newell Branch were prepared by the USACE, Mobile District, for FEMA, under Inter-Agency Agreement No. EMW-94-E-4371. That work was completed in February 1998.

The hydrologic and hydraulic analyses for the February 3, 2010 countywide revision were performed by the State of Mississippi for FEMA. That study was completed in September 2008 under Contract No. EMA-2005-CA-5215.

For this revision of the countywide FIS, new hydrologic and hydraulic analyses were prepared by Mississippi Geographic Information, LLC (MGI, LLC), the Study Contractor, for FEMA, under Contract No. EMA-2009-CA-5932. This revised study was completed in April 2011.

The following streams were included in this study:

- Bucatunna Creek
- Coats Creek
- Long Creek
- Rodgers Creek
- Suqualena Creek Tributary 4

Table 1, “Summary of Flooding Sources Presented in Current Study,” provides a summary of the flooding sources within Lauderdale County included in this current study, the contract number under which they were performed, and the communities affected by each.

Table 1: Summary of Flooding Sources Presented in Current Study

Flooding Source	Completion Date	Study Contractor(s)	Contract or Inter-Agency Agreement No.	Communities Affected
Bailey Branch	March 1987	USACE, Mobile District	EMW-85-E-1822	Lauderdale County
Bucatunna Creek	April 2011	Mississippi Geographic Information, LLC	EMA-2009-CA-5932	Lauderdale County
Coats Creek	April 2011	Mississippi Geographic Information, LLC	EMA-2009-CA-5932	Lauderdale County
Gallagher Creek	February 1998	USACE, Mobile District	EMW-94-E-4371	Lauderdale County City of Meridian

Table 1: Summary of Flooding Sources Presented in Current Study

Flooding Source	Completion Date	Study Contractor(s)	Contract or Inter-Agency Agreement No.	Communities Affected
Gunn Branch	March 1987	USACE, Mobile District	EMW-85-E-1822	Lauderdale County
Harbour Creek	February 1998	USACE, Mobile District	EMW-94-E-4371	Lauderdale County City of Meridian
Harper Creek	March 1987	USACE, Mobile District	EMW-85-E-1822	Lauderdale County
Harper Creek Tributary 1	March 1987	USACE, Mobile District	EMW-85-E-1822	Lauderdale County
Long Creek	April 2011	Mississippi Geographic Information, LLC	EMA-2009-CA-5932	Lauderdale County City of Meridian
Loper Creek	March 1987	USACE, Mobile District	EMW-85-E-1822	Lauderdale County City of Meridian
Magnolia Creek	February 1998	USACE, Mobile District	EMW-94-E-4371	Lauderdale County City of Meridian
McLemore Branch	September 2008	State of Mississippi	EMA-2005-CA-5215	Lauderdale County City of Meridian
Nanabe Creek	March 1987	USACE, Mobile District	EMW-85-E-1822	Lauderdale County
Nanabe Creek	January 1993	USACE, Mobile District	*	Lauderdale County

*Data not available

Table 1: Summary of Flooding Sources Presented in Current Study

Flooding Source	Completion Date	Study Contractor(s)	Contract or Inter-Agency Agreement No.	Communities Affected
Newell Branch	February 1998	USACE, Mobile District	EMW-94-E-4371	Lauderdale County Town of Marion City of Meridian
Okatibbee Creek	March 1987	USACE, Mobile District	EMW-85-E-1822	Lauderdale County
Okatibbee Creek	January 1993	USACE, Mobile District	*	Lauderdale County City of Meridian
Okatibbee Creek Tributary 5	March 1987	USACE, Mobile District	EMW-85-E-1822	Lauderdale County
Robbins Branch	February 1998	USACE, Mobile District	EMW-94-E-4371	Lauderdale County City of Meridian
Rodgers Creek	April 2011	Mississippi Geographic Information, LLC	EMA-2009-CA-5932	Lauderdale County
Shear's Branch	March 1987	USACE, Mobile District	EMW-85-E-1822	Lauderdale County City of Meridian
Shear's Branch	January 1993	USACE, Mobile District	*	Lauderdale County City of Meridian

*Data not available

Table 1: Summary of Flooding Sources Presented in Current Study

Flooding Source	Completion Date	Study Contractor(s)	Contract or Inter-Agency Agreement No.	Communities Affected
Sowashee Creek	January 1993	USACE, Mobile District	*	Lauderdale County Town of Marion City of Meridian
Sowashee Creek Tributary 8	September 2008	State of Mississippi	EMA-2005-CA-5215	Lauderdale County Town of Marion
Sowashee Creek Tributary 10	September 2008	State of Mississippi	EMA-2005-CA-5215	Lauderdale County Town of Marion
Sowashee Creek Tributary 11	September 2008	State of Mississippi	EMA-2005-CA-5215	Lauderdale County
Suqualena Creek	March 1987	USACE, Mobile District	EMW-85-E-1822	Lauderdale County
Suqualena Creek Tributary 4	April 2011	Mississippi Geographic Information, LLC	EMA-2009-CA-5932	Lauderdale County

*Data not available

For the February 3, 2010 FIS for Lauderdale County, the digital base map information was provided by the State of Mississippi. The digital orthoimagery was photogrammetrically compiled at a scale of 1:400 from aerial photography dated March 2006.

The digital FIRM was produced using the State Plane Coordinate System, Mississippi East, FIPS ZONE 2301. The horizontal datum was the North American Datum of 1983, GRS 80 spheroid. Distance units were measured in U.S. feet.

Base map information for the revised panels of this study of Lauderdale County and the City of Meridian was provided in digital format by the State of Mississippi. The

digital orthoimagery was photogrammetrically compiled at a scale of 1:12,000 from aerial photography dated July 2009.

The digital FIRM was produced using the State Plane Coordinate System, Mississippi East, FIPS ZONE 2301. The horizontal datum was the North American Datum of 1983, GRS 80 spheroid. Distance units were measured in U.S. feet. Differences in the datum and spheroid used in the production of FIRMs for adjacent counties may result in slight positional differences in map features at the county boundaries. These differences do not affect the accuracy of information shown on the FIRM.

1.3 Coordination

An initial Consultation Coordination Officer (CCO) meeting (also occasionally referred to as the Scoping meeting) is held with representatives of the communities, FEMA, and the study contractors to explain the nature and purpose of the FIS and to identify the streams to be studied by detailed methods. A final CCO (often referred to as the Preliminary DFIRM Community Coordination, or PDCC, meeting) is held with representatives of the communities, FEMA, and the study contractors to review the results of the study.

For this revision of the countywide FIS, the initial CCO meeting was held on September 16, 2009, and attended by representatives of FEMA, the Mississippi Emergency Management Agency, Lauderdale County, the City of Meridian, AES Limited, and MGI, LLC.

The final CCO meeting was held on _____ to review and accept the results of this FIS. Those who attended this meeting included representatives of Lauderdale County, the Study Contractor, FEMA, and the communities. All problems raised at that meeting have been addressed in this study.

The dates of the historical initial and final CCO meetings held for the communities within the boundaries of Lauderdale County are shown in Table 2, "Historical CCO Meeting Dates."

Table 2: Historical CCO Meeting Dates

Community Name	Initial CCO Date	Final CCO Date
Lauderdale County and Incorporated Areas (September 29, 1989)	June 13, 1985	October 17 and 18, 1988
Lauderdale County and Incorporated Areas (August 16, 1995)	July 21, 1993	August 9, 1994
Lauderdale County and Incorporated Areas (March 21, 2000)	August 13, 1993	*

*Data not available

Table 2: Historical CCO Meeting Dates

Community Name	Initial CCO Date	Final CCO Date
Lauderdale County and Incorporated Areas (February 3, 2010)	October 19, 2005	November 13, 2008

2.0 AREA STUDIED

2.1 Scope of Study

This FIS report covers the geographic area of Lauderdale County, Mississippi, including the incorporated community listed in Section 1.1. The scope and methods of this study were proposed to, and agreed upon, by FEMA and Lauderdale County.

September 29, 1989 Initial Countywide

All or portions of these flooding sources were studied using detailed methods for this countywide FIS: Bailey Branch, Nanabe Creek, Gallagher Creek, Okatibbee Creek, Gunn Branch, Okatibbee Creek Tributary, Harper Creek, Robbins Branch, Harper Creek Tributary, Shear's Branch, Loper Creek, Sowashee Creek, Magnolia Creek, and Suqualena Creek were studied by detailed methods.

August 16, 1995 Revision

All or portions of these flooding sources were studied using detailed methods for this FIS revision: Gallagher Creek, Magnolia Creek, Nanabe Creek, Okatibbee Creek, Robbins Branch, Shear's Branch, and Sowashee Creek. Backwater effects from Sowashee Creek on the other creeks were adjusted based on detailed analyses of Sowashee Creek.

March 21, 2000 Revision

All or portions of these flooding sources were studied using detailed methods for this FIS revision: Bailey Branch, Gallagher Creek, Gunn Branch, Harbour Creek, Harper Creek, Harper Creek Tributary, Loper Creek, Magnolia Creek, Nanabe Creek, Newell Branch, Okatibbee Creek, Okatibbee Creek Tributary, Robbins Branch, Shear's Branch, Sowashee Creek, and Suqualena Creek. These analyses superseded any previous studies on these streams.

This revision also incorporated a Letter of Map Revision (LOMR), dated September 30, 1999. This LOMR reflected more up-to-date hydrologic and hydraulic analyses along Sowashee Creek. The subject area is located from a point approximately 1,000 feet downstream of Hawkins Crossing Road (U.S. Route 45), to a point approximately 2,600 feet upstream of the confluence with Okatibbee Creek.

February 3, 2010 Revision

For the February 3, 2010 FIS revision, some streams had names other than those used in previously printed FIS reports. Details of these name changes are listed in the following tabulation:

<u>Community</u>	<u>Old Name</u>	<u>New Name</u>
Lauderdale County	Harper Creek Tributary	Harper Creek Tributary 1
Lauderdale County	Okatibbee Creek Tributary	Okatibbee Creek Tributary 5

For that countywide FIS revision, limited detailed analyses were performed for McLemore Branch, Sowashee Creek Tributary 8, Sowashee Creek Tributary 10, and Sowashee Creek Tributary 11. Streams that had not been previously studied using detailed methods were studied by approximate methods.

This Countywide Revision Analyses

For this revision, Bucatunna Creek was studied using detailed methods, and Coats Creek, Long Creek, Rodgers Creek, and Suqualena Creek Tributary 4 were studied by limited detailed methods. No new streams were studied by approximate methods.

All or portions of the flooding sources listed in Table 3, "Flooding Sources Studied by Detailed Methods," were studied by detailed methods and incorporated into this FIS report. Limits of detailed study are indicated on the Flood Profiles (Exhibit 1) and on the FIRM (Exhibit 2).

Table 3: Flooding Sources Studied by Detailed Methods

Flooding Source	Reach Length (miles)	Study Limits
Bailey Branch	2.4	From the confluence with Loper Creek and Gun Branch to a point approximately 1,400 feet upstream of Windsor Road.
Bucatunna Creek*	17.1	From the county boundary to a point approximately 1.25 miles upstream of Carl Harper Road.
Gallagher Creek	6	From the confluence with Sowashee Creek to a point approximately 50 feet upstream of State Route 493.

*Flooding source with new or revised analysis incorporated as part of the current study update

Table 3: Flooding Sources Studied by Detailed Methods

Flooding Source	Reach Length (miles)	Study Limits
Harbour Creek	2.6	From the confluence with Sowashee Creek to a point approximately 1,025 feet upstream of Windover Circle.
Harper Creek	4.5	From the confluence with Okatibbee Creek to a point approximately 3,975 feet upstream of Cooksey Road.
Harper Creek Tributary 1	1.1	From the confluence with Harper Creek to a point approximately 115 feet upstream of Woods Road.
Loper Creek	6.1	From the confluence with Okatibbee Creek to the confluence with Gunn Branch and Bailey Branch.
Gunn Branch	2.8	From the confluence with Loper Creek and Bailey Branch to a point approximately 2,500 feet upstream of State Highway 493.
Magnolia Creek	2.7	From the confluence with Sowashee Creek to a point approximately 100 feet upstream of 36th Street.
Nanabe Creek	4.5	From the confluence with Sowashee Creek to the confluence of Brandon Branch.
Newell Branch	3.2	From the confluence with Sowashee Creek to a point approximately 1,150 feet upstream of 61 st Court.
Okatibbee Creek	11.9	From a point approximately 18 miles upstream of the mouth to a point approximately 180 feet upstream of Allen Swamp Road.
Okatibbee Creek Tributary 5	1.0	From the confluence with Okatibbee Creek to a point approximately 145 feet upstream of Old 8 th Street Road.
Robbins Branch	4.0	From the confluence with Sowashee Creek to a point approximately 2,800 feet upstream of 52nd Court
Shear's Branch	1.9	From the confluence with Sowashee Creek to a point approximately 0.2 miles upstream of 20 th Street.

Table 3: Flooding Sources Studied by Detailed Methods

Flooding Source	Reach Length (miles)	Study Limits
Sowashee Creek	18.5	From a point approximately 2,600 feet above the confluence with Okatibbee Creek to a point approximately 90 feet upstream of Sam Lackey Road.
Suqualena Creek	6.1	From the confluence with Okatibbee Creek to a point approximately 2,300 feet upstream of Wilsondale Road.

The areas studied by limited detailed methods were selected for areas having low to moderate development potential or flood hazards. The flooding sources studied by limited detailed methods are presented in Table 4, “Flooding Sources Studied by Limited Detailed Methods.”

Table 4: Flooding Sources Studied by Limited Detailed Methods

Flooding Source	Reach Length (miles)	Study Limits
Coats Creek*	5.9	From a point approximately 2,925 feet upstream of the confluence with Okatibbee Creek to a point approximately 1.7 miles upstream of Brookins Road.
Long Creek*	9.9	From the southern county boundary to a point approximately 3,200 feet upstream of Causeyville Road.
McLemore Branch	3.4	From the confluence with Newell Branch to a point approximately 3,400 feet upstream of Windmill Drive.
Rodgers Creek*	11.6	From the confluence with Okatibbee Creek to a point approximately 3,400 feet upstream of Haguewood Road.
Sowashee Creek Tributary 8	6.9	From the confluence with Sowashee Creek to a point approximately 2,475 feet upstream of State Highway 39.
Sowashee Creek Tributary 10	3.0	From the confluence with Sowashee Creek Tributary 8 to a point approximately 475 feet upstream of Cotton Gin Road.

*Flooding source with new or revised analysis incorporated as part of the current study update

Table 4: Flooding Sources Studied by Limited Detailed Methods

Flooding Source	Reach Length (miles)	Study Limits
Sowashee Creek Tributary 11	1.1	From the confluence with Sowashee Creek Tributary 8 to a point approximately 575 feet upstream of Ponta Hills Road.
Suqualena Creek Tributary 4*	1.1	From a point approximately 470 upstream of the confluence with Suqualena Creek Tributary 2 to a point approximately 3,550 feet upstream of Collinsville Road.

*Flooding source with new or revised analysis incorporated as part of the current study update

Approximately 491.8 stream miles were studied by approximate methods. None of these were newly studied or revised for this countywide revision. Approximate analyses were used to study those areas having a low development potential or minimal flood hazards.

2.2 Community Description

Lauderdale County is located in east-central Mississippi, and is bordered by Kemper County on the north, Newton County on the west, Clarke County on the south, and Sumter and Choctaw Counties, Alabama on the east. The City of Meridian, the county seat, is approximately 90 miles east of the City of Jackson. Lauderdale County is served by Interstate Routes 20 and 59; U.S. Routes 11, 45, and 80; State Routes 19 and 39; the Illinois Central Railroad; the Norfolk Southern Railway; and the Meridian & Bigbee Railroad. Lauderdale County's 2010 population was 80,261 (Reference 1).

Topography in the area consists of gently rolling hills with elevations ranging from 270 feet National Geodetic Vertical Datum of 1929 (NGVD) to approximately 550 feet NGVD. The hilly terrain to the north, east, and west of the study area has a pronounced effect on the temperature pattern at Meridian.

Lauderdale County's climate is mild, with an average summer temperature of 79.8 degrees Fahrenheit (°F), and an average winter temperature of 48.4°F. Extreme temperatures of -7°F and 105°F have been recorded at Meridian (Reference 2).

Precipitation is evenly distributed throughout the year. March is the wettest month, averaging 6.0 inches of rainfall, and October is the driest, averaging 2.4 inches. Annual precipitation averages 56.7 inches (Reference 3).

The Okatibbee Creek floodplain is relatively undeveloped, with only a few structures in the area. The floodplains of Sowashee Creek, Gallagher Creek, Magnolia Creek, Robbins Branch, Nanabe Branch, and Loper Creek have yielded to urban expansion, including commercial, industrial, and residential development and public utilities. Numerous city streets, state highways, and railway lines cross the floodplains. Economic development

within the study area continues to expand.

2.3 Principal Flood Problems

Significant flooding occurs in the low-lying areas along Okatibbee Creek and its tributaries and along Sowashee Creek and its tributaries. Major floods occurred on Okatibbee Creek and its tributaries in 1961, 1976, and 1979. Major floods occurred on Sowashee Creek and its tributaries in 1951, 1961, 1964, 1972, 1974, and 1979. Parts of the Sowashee Creek floodplain have been filled in and developed in recent years, leading to increased flooding potential along some parts of the stream. Floods can occur in the City of Meridian any time during the year, but the most frequent flooding occurs during late summer or early fall caused by brief intense storms.

2.4 Flood Protection Measures

The Okatibbee Dam, built in 1968 and operated by the USACE, is 37.65 miles upstream of the Okatibbee Creek mouth. In the City of Meridian, the dam provides reductions in the 10-, 2- and 1-percent-annual-chance frequency floods of 2.1 feet, 1.4 feet, and 1.2 feet, respectively.

During World War II, the USACE constructed a levee in southern Meridian that joins high ground and encloses Key Field. The levee would be overtopped by the 0.2-percent-annual-chance flood on Okatibbee Creek and by the 2-percent-annual-chance flood on Sowashee Creek.

The USACE completed the Sowashee Creek Flood Control Project in December 1992. The project consisted of clearing and snagging the channel and overbanks; channel modifications; bridge modifications and relocations; and control structures. The project extends from the mouth of the stream to Hawkins Crossing Road. The Soil Conservation Service (SCS, now known as the Natural Resources Conservation Service). has completed three flood control structures in the Sowashee Creek drainage basin that help to reduce the flood elevations along Sowashee Creek. These existing SCS structures were considered for this FIS. The SCS plans to construct additional flood retarding structures in the upper reaches of the Sowashee Creek drainage basin when funding becomes available; these structures were not considered for this FIS.

Portions of Gallagher Creek, Magnolia Creek, and Robbins have been modified. Channel modifications range from grading to concrete lining. The lower portion of Gallagher Creek was modified by the construction of a project built by the SCS.

3.0 **ENGINEERING METHODS**

For the flooding sources studied by detailed methods in the community, standard hydrologic and hydraulic study methods were used to determine the flood-hazard data required for this study. Flood events of a magnitude that is expected to be equaled or exceeded once on the average during any 10-, 50-, 100-, or 500-year period (recurrence interval) have been selected as having special significance for floodplain management and for flood insurance rates. These events, commonly termed the 10-, 50-, 100-, and 500-year floods, have a 10-, 2-, 1-, and 0.2-

percent chance, respectively, of being equaled or exceeded during any year. Although the recurrence interval represents the long-term, average period between floods of a specific magnitude, rare floods could occur at short intervals or even within the same year. The risk of experiencing a rare flood increases when periods greater than 1 year are considered. For example, the risk of having a flood that equals or exceeds the 1-percent-annual-chance flood in any 50-year period is approximately 40 percent (4 in 10); for any 90-year period, the risk increases to approximately 60 percent (6 in 10). The analyses reported herein reflect flooding potentials based on conditions existing in the community at the time of completion of this study. Maps and flood elevations will be amended periodically to reflect future changes.

3.1 Hydrologic Analyses

3.1.1 Methods for Flooding Sources with New or Revised Analyses in Current Study

For this countywide study, peak discharges for the streams studied by detailed and limited detailed methods were calculated based on USGS regional regression equations (Reference 4).

For the discharges calculated based on regional regression equations, the rural regression values were updated to reflect urbanization as necessary. The seven-parameter urbanization equation in the USGS report 91-4037 (Reference 4) was used to calculate urbanization discharges.

A summary of the drainage area-peak discharge relationships for the detailed and limited detailed study streams is shown in Table 5, "Summary of Discharges for Detailed Study Streams", and for limited detail study streams is shown in Table 6, "Summary of Discharges for Limited Detailed Study Streams."

3.1.2 Methods for Flooding Sources Incorporated from Previous Studies

This section describes the methodology used in previous studies of flooding sources incorporated into this FIS that were not revised for this countywide study.

September 29, 1989 Countywide Analyses

Flow frequencies for uncontrolled drainage areas along the detailed studied streams were determined by one of two methods. For drainage areas larger than four square miles, flows were developed using regional relationships taken from a USACE environmental impact statement (Reference 5). Methodology presented in that report was developed from regional data on drainage areas of 15 square miles or more in the Pascagoula River basin. For drainage areas smaller than four square miles, flows were determined using the regional equations presented in the U.S. Geological Survey's (USGS) *Flood Frequency of Mississippi Streams* (Reference 6).

August 16, 1995 Countywide Analyses

Discharges for locations from the mouth of Sowashee Creek to below the

confluence with Nanabe Creek were determined by using the existing conditions discharges from Table II-3, "Peak Discharges at Selected Locations, Project Conditions," in the USACE report entitled *Sowashee Creek, Meridian, Mississippi: Design Memorandum No. 1*, dated April 1986 (Reference 7). These discharges included adjustments for expected probability. The expected probability adjustments were removed from these estimates by reversing the procedure described in Bulletin 17B (Reference 8). Table II-1 in Bulletin 17B was entered at P_N , the expected probability values from Table II-3, and the number of years of record at the gage, $N=80$. The values of P , the exceedance frequencies without the adjustments, were then interpolated from the table header. The discharges from the table were then plotted at these frequencies and the discharges for the 10-, 2-, 1-, and 0.2-percent-annual-chance events were interpolated from the graphs.

Discharges for locations on Sowashee Creek above the confluence with Nanabe Creek were determined by using regional frequency relationships developed by the Mobile District and described in the USACE report entitled *Sowashee Creek, Meridian, Mississippi, Phase 1 General Design Memorandum and Environmental Impact Statement* (Reference 5).

Discharge data were obtained from the SCS for each of the reservoirs. An analysis was made to determine the effects of these reservoirs on the discharges on Sowashee Creek from the mouth to the county road crossing at Marion. Although these structures create relatively large reductions on the tributaries on which they are located, their location in the drainage basin and the size of the areas they control, compared to the size of the Sowashee Creek drainage area, almost eliminates the effect they have on the discharges on Sowashee Creek. However, combined with the other 10 structures in the total basin plan, the effect could be a significant reduction in the discharges.

The USGS recently completed a regional study in Mississippi and published a report, Water-Resources Investigation Report 91-4037 (Reference 4). The regional equations from this study produced discharges that were considerably lower than those in the USACE report. The USACE conducted a search of the data compiled by the USGS, which revealed that the period of record at the Sowashee Creek gage ran from 1939 to 1988, with peaks from 1946 to 1948 missing. Two floods, February 1936 and April 1938, occurred prior to the systematic record and were higher than any flood in the systematic record. In addition, the 1936 flood was listed as the highest flood since 1900. These two floods only had peak stages listed in the database, and were not included in the USGS study. The USACE study included estimated discharges for these two flood events. By including these events the historic gage record could be extended to 1900. Omitting these two floods, especially the 1936 flood, may underestimate the frequency curve. Therefore, for purposes of this study, the USACE decided not to use the USGS regional study and to use its own study, which included these estimated peaks.

March 21, 2000 Countywide Revision Analyses

Discharges were determined using USGS regional equations from *Flood Characteristics of Mississippi Streams* (Reference 4). Drainage areas were determined from 1:24,000 quadrangle maps (Reference 9).

February 3, 2010 Countywide Revision Analyses

Peak discharges for the streams studied by Limited Detailed methods were calculated based on USGS regional regression equations (Reference 4). For the discharges calculated based on regional regression equations, the rural regression values were updated to reflect urbanization as necessary.

3.2 Hydraulic Analyses

Analyses of the hydraulic characteristics of flooding from the sources studied were carried out to provide estimates of the elevations of floods of the selected recurrence intervals. Users should be aware that flood elevations shown on the FIRM represent rounded whole-foot elevations and may not exactly reflect the elevations shown on the Flood Profiles or in the Floodway Data tables in the FIS report. Flood elevations shown on the FIRM are primarily intended for flood insurance rating purposes. For construction and/or floodplain management purposes, users are cautioned to use the flood elevation data presented in this FIS in conjunction with the data shown on the FIRM.

Flood profiles were drawn showing the computed water-surface elevations for floods of the selected recurrence intervals. Locations of selected cross sections used in the hydraulic analyses are shown on the Flood Profiles (Exhibit 1).

Roughness coefficients (Manning's "n") were chosen by engineering judgment and based on field observation of the channel and floodplain areas. Table 7, "Summary of Roughness Coefficients," contains the channel and overbank "n" values for the streams studied by detailed methods.

The hydraulic analyses for this study were based on unobstructed flow. The flood elevations shown on the Flood Profiles (Exhibit 1) are thus considered valid only if hydraulic structures remain unobstructed, operate properly, and do not fail.

All elevations are referenced for NAVD88.

Table 5: Summary of Discharges for Detailed Study Streams

Flooding Source and Location	Drainage Area (Square miles)	Peak Discharges (Cubic Feet per Second)			
		10-percent	2-percent	1-percent	0.2-percent
BAILEY BRANCH					
At mouth	6.24	1,410	3,040	4,010	7,170
GALLAGHER CREEK					
At mouth	5.65	2,720	3,990	4,600	5,690
Just downstream of State Boulevard	4.23	2,230	3,300	3,790	4,700
Just downstream of Royal Road	3.22	1,880	2,790	3,190	3,970
Just downstream of 40 th Street	2.2	1,250	1,900	2,160	2,700
At a point approximately 1,000 feet upstream of 52 nd Street	0.61	570	1,100	1,320	2,100
GUNN BRANCH					
At mouth	9.88	2,640	5,610	7,390	13,080
HARBOUR CREEK					
At mouth	1.34	785	1,207	1,369	1,727
Just downstream of Newell Road	0.98	698	1,061	1,199	1,503
Just upstream of Newell Road	0.5	404	612	688	863
Just downstream of 9 th Avenue	0.07	113	178	196	247
HARPER CREEK					
At mouth	10.86	2,780	5,790	7,570	13,200
Just downstream of confluence of Harper Creek Tributary 1	8.26	2,340	4,960	6,520	11,500

Table 5: Summary of Discharges for Detailed Study Streams

Flooding Source and Location	Drainage Area (Square miles)	Peak Discharges (Cubic Feet per Second)			
		10-percent	2-percent	1-percent	0.2-percent
HARPER CREEK (continued)					
At County Road	5.65	1,700	3,680	4,880	8,750
Just upstream of County Road running south of State Route 494	3.99	1,240	2,710	3,600	6,500
HARPER CREEK TRIBUTARY 1					
At mouth	1.64	750	1,130	1,320	1,770
LOPER CREEK					
At mouth	21.96	3,950	8,050	10,430	17,870
At State Boulevard	20.06	4,330	8,820	11,430	19,580
At Kings Road	16.49	3,810	7,840	10,190	17,590
MAGNOLIA CREEK					
At mouth	1.44	1,070	1,560	1,770	2,170
Just downstream of 23 rd Street	0.74	780	1,140	1,290	1,580
Just downstream of 36 th Street	0.2	390	560	620	760
NANABE CREEK					
At mouth	13.70	3,360	6,980	9,120	15,900
Just downstream of the confluence of Brandon Branch	9.29	2,730	5,790	7,610	13,400

Table 5: Summary of Discharges for Detailed Study Streams

Flooding Source and Location	Drainage Area (Square miles)	Peak Discharges (Cubic Feet per Second)				
		10-percent	2-percent	1-percent	0.2-percent	
NEWELL BRANCH						
At mouth	3.54	1,334	2,065	2,366	2,961	
Just downstream of unnamed tributary	3.43	1,356	2,099	2,404	3,005	
Just upstream of unnamed tributary	1.26	698	1,082	1,227	1,548	
Just downstream of 23 rd Street	0.11	175	268	296	370	
OKATIBBEE CREEK						
At Meridian gage	235.64	10,590	19,580	24,680	41,250	
Just downstream of mouth of Leper Creek	230.60	10,700	19,800	24,680	41,700	
Just upstream of mouth of Leper Creek	208.64	8,360	15,390	19,410	32,740	
Just downstream of mouth of Harper Creek	201.84	8,800	16,510	20,890	35,380	
Just downstream of mouth of Suqualena Creek	189.58	7,450	13,680	17,260	29,300	
At Okatibbee Dam	153.29	1,200	1,200	1,200	2,200	
OKATIBBEE CREEK TRIBUTARY 5						
At mouth	0.74	380	560	650	900	
ROBBINS BRANCH						
At mouth	2.02	1,060	1,570	1,790	2,210	

Table 5: Summary of Discharges for Detailed Study Streams

Flooding Source and Location	Drainage Area (Square miles)	Peak Discharges (Cubic Feet per Second)			
		10-percent	2-percent	1-percent	0.2-percent
ROBBINS BRANCH (continued)					
Just downstream of 34 th Street	1.27	1,010	1,500	1,700	2,110
Just downstream of North Hills Street	0.34	480	720	800	1,000
SHEAR'S BRANCH					
At mouth	1.8	2,600	3,300	3,700	4,400
At 21 st Street	0.1	500	620	700	850
SOWASHEE CREEK					
At mouth	84.21	21,185	17,567	19,911	23,440
At Substation	77.20	14,776	19,423	21,551	24,917
At 49 th Avenue	75.14	14,925	19,717	21,805	25,368
Just downstream of Gallagher Creek	74.00	14,900	19,775	21,799	25,202
Just upstream of Gallagher Creek	68.35	11,836	16,072	18,349	21,582
At Shears Avenue	67.94	11,813	16,131	18,555	21,765
At Grand Avenue	64.93	10,693	15,357	17,656	20,999
At 22 nd Avenue	62.83	10,138	14,814	17,057	20,617
At Magnolia Creek	59.25	10,003	14,636	16,919	20,646
At gage	52.07	8,578	12,544	14,600	18,211

Table 5: Summary of Discharges for Detailed Study Streams

Flooding Source and Location	Drainage Area (Square miles)	Peak Discharges (Cubic Feet per Second)			
		10-percent	2-percent	1-percent	0.2-percent
SOWASHEE CREEK (continued)					
At Robbins Branch	52.07	8,355	12,227	14,272	17,862
At Hawkins Road Crossing	48.97	8,161	11,986	14,054	17,612
At Clear Branch	46.23	7,677	15,200	18,767	30,200
Just downstream of Nanabe Creek	41.71	7,087	13,803	17,392	29,913
Just upstream of Nanabe Creek	28.01	5,048	7,755	8,813	11,653
At Alamutcha Street	20.28	4,478	6,356	6,968	8,698
At County Road	7.59	2,171	3,242	3,635	4,711
SUQUALENA CREEK					
At mouth	16.78	3,190	6,570	8,550	14,750
At County Road	15.39	3,380	6,950	9,030	15,590
At County Road	13.20	3,290	6,830	8,910	15,500
Just upstream of County Road	7.87	2,290	4,860	6,390	11,280

Table 6: Summary of Discharges for Limited Detailed Study Streams

Flooding Source and Location	Drainage Area (Square miles)	Peak Discharges (Cubic Feet per Second)			
		10-percent	2-percent	1-percent	0.2-percent
COATS CREEK					
At confluence with Okatibbee Creek	11.8	*	*	6,495	*
At T M Jones Road	2.1	*	*	1,435	*
LONG CREEK					
At county boundary	30.6	*	*	9,575	*
At Purvis Road	19.1	*	*	7,007	*
At Railroad	6.6	*	*	3,035	*
MCLEMORE BRANCH					
At mouth	2.14	*	*	1,829	*
RODGERS CREEK					
At confluence with Okatibbee Creek	14.1	*	*	5,016	*
At State Highway 495	8.3	*	*	4,286	*
SOWASHEE CREEK TRIBUTARY 8					
At Willow Lake Road	3.53	*	*	2,350	*
SOWASHEE CREEK TRIBUTARY 10					
Approximate 2,900 feet upstream of mouth	3.06	*	*	1,943	*
SOWASHEE CREEK TRIBUTARY 11					
At mouth	0.41	*	*	532	*

*Data Not Available

Table 6: Summary of Discharges for Limited Detailed Study Streams

Flooding Source and Location	Drainage Area (Square miles)	Peak Discharges (Cubic Feet per Second)			
		10-percent	2-percent	1-percent	0.2-percent
SUQUALENA CREEK TRIBUTARY 4					
Approximately 470 feet upstream of the confluence with Suqualena Creek Tributary 2	2.6	*	*	2,053	*

* Data Not Available

Table 7: Summary of Roughness Coefficients

Flooding Source	Channel	Overbanks
Bailey Branch	0.015-0.07	0.080-0.15
Bucatunna Creek	0.045-0.05	0.08-0.15
Coats Creek	0.038-0.05	0.12-0.14
Gallagher Creek	0.015-0.07	0.080-0.15
Gunn Creek	0.015-0.07	0.080-0.15
Harbour Creek	0.015-0.07	0.080-0.15
Harper Creek	0.015-0.07	0.080-0.15
Harper Creek Tributary 1	0.015-0.07	0.080-0.15
Long Creek	0.045	0.12-0.14
Loper Creek	0.015-0.07	0.080-0.15
Magnolia Creek	0.015-0.07	0.080-0.15
McLemore Branch	0.048	0.08-0.15
Nanabe Creek	0.015-0.07	0.080-0.15
Newell Branch	0.015-0.07	0.080-0.15
Okatibbee Creek	0.015-0.07	0.080-0.15
Okatibbee Creek Tributary 5	0.015-0.07	0.080-0.15
Robbins Branch	0.015-0.07	0.080-0.15
Rodgers Creek	0.045	0.10-0.14
Shear's Branch	0.015-0.07	0.080-0.15
Sowashee Creek	0.015-0.07	0.080-0.15
Sowashee Creek Tributary 8	0.046-0.051	0.05-0.15
Sowashee Creek Tributary 10	0.05-0.055	0.09-0.15
Sowashee Creek Tributary 11	0.035-0.05	0.07-0.15
Suqualena Creek	0.015-0.07	0.080-0.15
Suqualena Creek Tributary 4	0.04-0.05	0.12-0.14

3.2.1 Methods for Flooding Sources with New or Revised Analyses in Current Study

Cross section geometries were obtained from a combination of terrain data and

field surveys. The computer program WISE was used as a preprocessor to extract cross section topographic data from the WISE terrain project (Reference 10). Structure data is based on Mississippi Department of Transportation (MDOT) as-built data and field surveys. Standard limited detailed survey method was used to collect elevation data and structural geometry for bridges and culverts located within the limited detail study limits where as-built data is not available.

Downstream boundary conditions for the hydraulics models were set to normal depth using a starting slope calculated from values taken from topographic data, or where applicable, derived from the water surface elevations of existing effective flood elevations or recalculated flood elevations. Water surface profiles were computed through the use of USACE HEC-RAS version 4.1 computer program (Reference 11). The model was run for the 1 and 0.2-percent-annual-chance storm for the detail study stream and for the 1-percent-annual-chance storm for the limited detail study streams.

3.2.2 Methods for Flooding Sources Incorporated from Previous Studies

September 29, 1989, Countywide Analyses

Cross sections were obtained from field surveys supplemented by sections from previously published topographic maps of the City of Meridian (Reference 12).

Water-surface elevations of floods of the selected recurrence intervals were computed using the USACE HEC-2 step-backwater computer program (Reference 13).

Roughness factors (Manning's "n") were chosen by engineering judgment and based on field observations of the stream and floodplain areas, and comparing water-surface profiles with rating curves at gages on Okatibbee Creek. The channel and "n" values for Sowashee Creek ranged from 0.020 to 0.045, and the overbank "n" values ranged from 0.060 to 20.0; the overbank "n" value of 20.0 was used to represent ineffective flow in the HEC-2 model for Sowashee Creek. The channel "n" values for all other flooding sources studied by detailed methods ranged from 0.038 to 0.060 and the overbank "n" values ranged from 0.070 to 0.150.

August 16, 1995 Revision Analyses

Cross sections for Sowashee Creek from the mouth to Hawkins Crossing Road were obtained from field surveys. Right and left overbank cross-section geometry was obtained from topographic maps at a scale of 1"=200' with a contour interval of 2 feet (Reference 14). Cross sections for Sowashee Creek from Hawkins Crossing Road to the second county road above Highway 45 By-Pass were taken from the previously published September 29, 1989, FIS for Lauderdale County (Reference 15). All bridges, dams, and culverts were field surveyed to obtain elevation data and structural geometry.

Starting water-surface elevations for Sowashee Creek were obtained from a USACE design memorandum (Reference 5). Flood profiles were drawn showing computed water-surface elevations for floods of the selected recurrence intervals.

Roughness factors (Manning's "n") were chosen by engineering judgment and based on field observations of the stream and floodplain areas, and comparing water-surface profiles with rating curves at gages on Okatibbee Creek. The channel and "n" values for Sowashee Creek ranged from 0.020 to 0.045, and the overbank "n" values ranged from 0.060 to 20.0; the overbank "n" value of 20.0 was used to represent ineffective flow in the HEC-2 model for Sowashee Creek. The channel "n" values for all other flooding sources studied by detailed methods ranged from 0.038 to 0.060 and the overbank "n" values ranged from 0.070 to 0.150.

Areas of the community protected by levees were subject to potential risk due to possible failure or overtopping of the levee. Those areas were delineated by applying the 1-percent-annual-chance elevation determined from the "levee in place" analysis.

March 21, 2000 Revision Analyses

Cross section geometry utilized in the model was obtained from contour mapping field surveys, and construction drawings. Structural geometry and elevation data for bridges within the study reaches were obtained from field surveys and construction drawings.

The profiles of the selected recurrence interval floods were computed with standard step backwater math models using the Corps of Engineers HEC-2 computer program (Reference 13). Starting water surface elevations for streams studied were developed by the slope-area option of the HEC-2 model. Channel and overbank roughness factors (Manning's "n") used in the hydraulic computations were chosen by engineering judgment from aerial photos and field inspection of the channels and floodplain areas. The channel "n" values used for this study ranged from 0.015 to 0.07. The overbank "n" values ranged from 0.080 to 0.15 except in areas containing noneffective flow.

Maps of the study area, at a scale of 1 inch equals 200 feet with a contour interval of five feet, were used for the topographic data. The maps are based on City of Meridian topographic maps (Reference 12).

February 3, 2010 Revision Analyses

Cross section geometries were obtained from a combination of terrain data and field surveys. Bridges and culverts located within the limited detailed study limits were field surveyed to obtain elevation data and structural geometry.

Downstream boundary conditions for the hydraulics models were set to normal depth using a starting slope calculated from values taken from topographic data, or where applicable, derived from the water-surface elevations of existing

effective flood elevations or recalculated flood elevations. Water-surface profiles were computed through the use of USACE HEC-RAS version 3.1.2 computer program (Reference 16). The model was run for the 1-percent-annual-chance storm for the limited detailed and approximate studies.

Manning's "n" values used in the hydraulic computations for both channel and overbank areas were based on recent digital orthophotography and field investigations. Table 7, "Summary of Roughness Coefficients," shows the ranges of the channel and overbank roughness factors used in the computations for the streams studied by detailed methods and streams studied by limited detailed methods for that revision.

3.3 Vertical Datum

All FIS reports and FIRMs are referenced to a specific vertical datum. The vertical datum provides a starting point against which flood, ground, and structure elevations can be referenced and compared. Until recently, the standard vertical datum used for newly created or revised FIS reports and FIRMs was the National Geodetic Vertical Datum of 1929 (NGVD). With the completion of the North American Vertical Datum of 1988 (NAVD), many FIS reports and FIRMs are now prepared using NAVD as the referenced vertical datum.

Qualifying bench marks within a given jurisdiction that are catalogued by the National Geodetic Survey (NGS) and entered into the National Spatial Reference System (NSRS) as First or Second Order Vertical and have a vertical stability classification of A, B, or C are shown and labeled on the FIRM with their 6-character NSRS permanent Identifier.

Bench marks catalogued by the NGS and entered into the NSRS vary widely in vertical stability classification. NSRS vertical stability classifications are as follows:

Stability A: Monuments of the most reliable nature, expected to hold position/elevation well (e.g., mounted in bedrock)

Stability B: Monuments which generally hold their position/elevation well (e.g., concrete bridge abutment)

Stability C: Monuments which may be affected by surface ground movements (e.g., concrete monuments below frost line)

Stability D: Mark of questionable or unknown vertical stability (e.g., concrete monument above frost line, or steel witness post)

All flood elevations shown in this FIS report and on the FIRM are referenced to NAVD88. These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum.

Ground, structure, and flood elevations may be compared and/or referenced to NGVD by subtracting 0.04 feet from the NAVD elevation. The 0.04 foot value is an average for the entire county. The BFEs shown on the FIRM represent whole-foot rounded values. For

example, a BFE of 12.4 feet will appear as 12 feet on the FIRM and 12.6 feet as 13 feet. Users who wish to convert the elevations in this FIS report to NGVD should apply the stated conversion factor to elevations shown on the Flood Profiles and supporting data tables in the FIS report, which are shown at a minimum to the nearest 0.1 foot.

For more information regarding conversion between the NGVD and NAVD, see the FEMA publication entitled *Converting the National Flood Insurance Program to the North American Vertical Datum of 1988* (Reference 17), visit the National Geodetic Survey website at www.ngs.noaa.gov, or contact the National Geodetic Survey at the following address:

NGS Information Services
NOAA, N/NGS12
National Geodetic Survey
SSMC-3, #9202
1315 East-West Highway
Silver Spring, Maryland 20910-3282
(301) 713-3242

Temporary vertical monuments are often established during the preparation of a flood hazard analysis for the purpose of establishing local vertical control. Although these monuments are not shown on the FIRM, they may be found in the Technical Support Data Notebook associated with the FIS report and FIRM for this community. Interested individuals may contact FEMA to access these data.

To obtain current elevation, description, and/or location information for benchmarks shown on this map, please contact the Information Services Branch of the NGS at (301) 713-3242, or visit their website at www.ngs.noaa.gov.

4.0 FLOODPLAIN MANAGEMENT APPLICATIONS

The NFIP encourages State and local governments to adopt sound floodplain management programs. To assist in this endeavor, each FIS report provides 1-percent-annual-chance floodplain data, which may include a combination of the following: 10-, 2-, 1-, and 0.2-percent-annual-chance flood elevations; delineations of the 1- and 0.2-percent-annual-chance floodplains; and a 1-percent-annual-chance floodway. This information is presented on the FIRM and in many components of the FIS report, including Flood Profiles, Floodway Data tables, and Summary of Stillwater Elevation tables. Users should reference the data presented in the FIS report as well as additional information that may be available at the local community map repository before making flood elevation and/or floodplain boundary determinations.

4.1 Floodplain Boundaries

To provide a national standard without regional discrimination, the 1-percent-annual-chance flood has been adopted by FEMA as the base flood for floodplain management purposes. The 0.2-percent-annual-chance flood is employed to indicate additional areas of flood risk in the community. For each stream studied by detailed or limited detailed

methods, the 1-percent-annual-chance floodplain boundaries have been delineated using the flood elevations determined at each cross section.

For the February 3, 2010 countywide revision, 10 meter Digital Elevation Model (DEM) data from the USGS were used to delineate the floodplain boundaries.

For the revised panels of this update, between cross sections, the boundaries were interpolated using a contour interval of 2 feet.

Some areas of the county that are protected from the 1-percent-annual-chance flood by a levee have been delineated as having potential risk due to possible failure or overtopping of the levee during larger floods.

The 1-percent-annual-chance floodplain boundaries for streams studied by detailed methods are shown on the FIRM. On this map, the 1-percent-annual-chance floodplain boundary corresponds to the boundary of the areas of special flood hazards (Zones A and AE). Small areas within the floodplain boundaries may lie above the flood elevations, but cannot be shown due to limitations of the map scale and/or lack of detailed topographic data.

For streams studied by limited detailed and approximate methods, only the 1-percent-annual-chance floodplain boundary is shown on the FIRM (Exhibit 2).

4.2 Floodways

Encroachment on floodplains, such as structures and fill, reduces flood-carrying capacity, increases flood heights and velocities, and increases flood hazards in areas beyond the encroachment itself. One aspect of floodplain management involves balancing the economic gain from floodplain development against the resulting increase in flood hazard. For purposes of the NFIP, a floodway is used as a tool to assist local communities in this aspect of floodplain management. Under this concept, the area of the 1-percent-annual-chance floodplain is divided into a floodway and a floodway fringe. The floodway is the channel of a stream, plus any adjacent floodplain areas, that must be kept free of encroachment so that the base flood can be carried without substantial increases in flood heights. Minimum Federal standards limit such increases to 1 foot, provided that hazardous velocities are not produced. The floodways in this study are presented to local agencies as minimum standards that can be adopted directly or that can be used as a basis for additional floodway studies.

The floodways presented in this FIS were computed for certain stream segments on the basis of equal conveyance reduction from each side of the floodplain. Floodway widths were computed at cross sections. Between cross sections, the floodway boundaries were interpolated. The results of the floodway computations are tabulated for selected cross sections (Table 8). The computed floodways are shown on the FIRM (Exhibit 2). In cases where the floodway and 1-percent-annual-chance floodplain boundaries are either close together or collinear, only the floodway boundary is shown.

Encroachment into areas subject to inundation by floodwaters having hazardous velocities aggravates the risk of flood damage, and heightens potential flood hazards by further

increasing velocities. A listing of stream velocities at selected cross sections is provided in Table 8, "Floodway Data." In order to reduce the risk of property damage in areas where the stream velocities are high, the community may wish to restrict development in areas outside the floodway.

Near the mouths of streams studied in detail, floodway computations are made without regard to flood elevations on the receiving water body. Therefore, "Without Floodway" elevations presented in Table 8, "Floodway Data" for certain downstream cross sections of Gallagher Creek, Loper Creek, Magnolia Creek, Okatibbee Creek, Robbins Branch, Shear's Branch, Harbour Creek, and Newell Branch are lower than the regulatory flood elevations in that area, which must take into account the 1-percent-annual-chance flooding due to backwater from other sources.

The area between the floodway and 1-percent-annual-chance floodplain boundaries is termed the floodway fringe. The floodway fringe encompasses the portion of the floodplain that could be completely obstructed without increasing the water-surface elevation of the 1-percent-annual-chance flood by more than 1.0 foot at any point. Typical relationships between the floodway and the floodway fringe and their significance to floodplain development are shown in Figure 1.

No floodways were computed for Okatibbee Creek Tributary 5, Nanabe Creek, Suqualena Creek, Harper Creek, Harper Creek Tributary 1, portions of Loper Creek, Gunn Branch, and Bailey Branch. No floodways were computed for Coats Creek, Long Creek, Rodgers Creek, McLemore Branch, Sowashee Creek Tributary 8, Sowashee Creek Tributary 10, Sowashee Creek Tributary 11 and Suqualena Creek Tributary 4 because of limitations in the limited detailed study methodology.

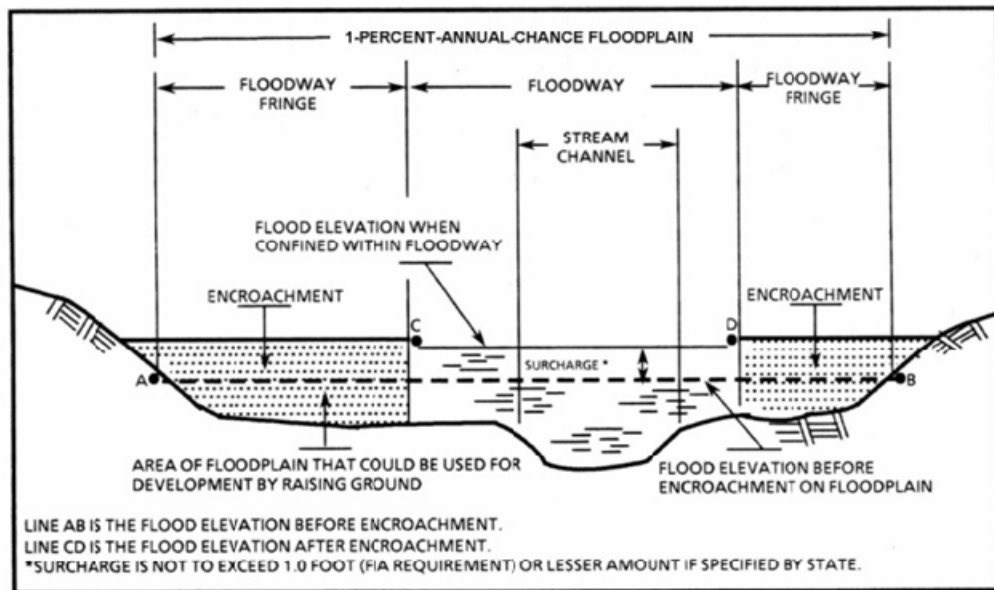


Figure 1: Floodway Schematic

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
Bucantunna Creek								
A	2,000	1378	6,953	1.5	354.3	354.3	355.3	1.0
B	7,678	1670	8,684	1.2	359.3	359.3	359.9	0.6
C	12,000	1706	9,160	1.2	360.9	360.9	361.7	0.8
D	17,000	829	4,471	2.4	363.5	363.5	364.5	1.0
E	23,500	1110	8,526	1.3	368.0	368.0	369.0	1.0
F	27,172	1637	7,361	1.3	371.7	371.7	372.5	0.8
G	34,000	664	4,214	1.9	378.1	378.1	379.0	0.9
H	40,180	992	6,081	1.3	383.3	383.3	384.2	0.9
I	45,634	1445	3,932	2.1	386.8	386.8	387.8	1.0
J	50,840	1008	4,562	1.6	392.3	392.3	393.2	0.9
K	54,185	617	3,439	2.0	396.3	396.3	397.2	0.9
L	58,000	870	3,800	1.7	400.3	400.3	401.3	1.0
M	63,000	1106	4,343	1.4	405.5	405.5	406.5	1.0
N	66,819	233	1,418	3.1	408.7	408.7	409.4	0.7
O	71,500	746	4,855	0.8	411.9	411.9	412.7	0.8
P	76,000	293	1,463	2.5	417.6	417.6	418.4	0.8
Q	79,000	425	1,434	2.2	423.9	423.9	424.5	0.6
R	82,000	309	1,139	2.6	429.6	429.6	430.5	0.9
S	88,028	169	924	2.4	442.4	442.4	443.0	0.6

¹ Feet above Limit of Detailed Study (a point approximately 860 feet upstream of County Rd 432 in Clarke County, MS)

TABLE 8

FEDERAL EMERGENCY MANAGEMENT AGENCY
LAUDERDALE COUNTY, MS
 AND INCORPORATED AREAS

FLOODWAY DATA

Bucatunna Creek

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
Gallagher Creek								
A	1,460	88	813	5.7	306.0	304.2 ²	304.8	0.6
B	2,566	77	650	7.1	306.0	305.3 ²	305.7	0.4
C	3,456	86	725	6.3	306.6	306.6	307.5	0.9
D	4,082	83	696	6.6	307.8	307.8	308.0	0.2
E	5,590	82	601	7.7	312.9	312.9	312.9	0.0
F	6,198	79	541	8.5	313.1	313.1	313.1	0.0
G	7,280	68	355	12.9	316.4	316.4	316.4	0.0
H	8,430	68	350	10.8	319.2	319.2	319.2	0.0
I	8,960	94	678	5.6	328.2	328.2	329.2	1.0
J	10,425	89	831	4.6	333.4	333.4	333.9	0.5
K	11,503	260	955	3.3	334.6	334.6	335.3	0.7
L	12,980	125	899	3.5	337.2	337.2	338.1	0.9
M	14,480	83	732	4.4	344.3	344.3	344.8	0.5
N	15,850	78	631	5.1	347.7	347.7	348.2	0.5
O	17,011	58	436	7.3	351.7	351.7	352.6	0.9
P	18,574	108	512	6.2	358.1	358.1	358.7	0.6
Q	19,797	104	457	4.7	362.9	362.9	363.6	0.7
R	19,922	76	331	6.5	363.1	363.1	363.8	0.7

¹ Feet above confluence with Sowashee Creek

² Elevation computed without consideration of backwater effects from Sowashee Creek

TABLE 8

FEDERAL EMERGENCY MANAGEMENT AGENCY
LAUDERDALE COUNTY, MS
 AND INCORPORATED AREAS

FLOODWAY DATA

Gallagher Creek

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
Gallagher Creek (continued)								
S	20,980	29	219	9.8	364.6	364.6	365.0	0.4
T	22,010	49	277	7.8	371.2	371.2	371.2	0.0
U	23,227	69	397	5.4	376.9	376.9	377.9	1.0
V	24,180	125	708	3.1	380.7	380.7	381.4	0.7
W	25,530	64	247	8.8	386.2	386.2	386.4	0.2
X	26,165	184	806	2.7	391.4	391.4	391.6	0.2
Y	29,069	185	515	1.9	403.8	403.8	404.4	0.6
Z	31,784	29	134	7.3	422.9	422.9	423.0	0.1

¹ Feet above confluence with Sowashee Creek

TABLE 8

FEDERAL EMERGENCY MANAGEMENT AGENCY
LAUDERDALE COUNTY, MS
 AND INCORPORATED AREAS

FLOODWAY DATA

Gallagher Creek

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
Harbour Creek								
A	732	1,220 ²	429	3.2	339.8	332.0 ³	332.0	0.0
B	2,200	155	421	3.1	341.0	336.9 ³	337.1	0.2
C	3,300	115	433	3.0	341.4	340.2 ³	341.2	1.0
D	5,000	103	585	2.1	350.5	350.5	351.2	0.7
E	6,100	219	1,029	1.2	353.0	353.0	353.8	0.8
F	7,100	19	146	4.4	362.5	362.5	362.5	0.0
G	8,650	22	63	9.1	370.4	370.4	370.4	0.0
H	9,700	20	85	5.9	382.8	382.8	383.6	0.8
I	10,704	14	82	6.1	388.9	388.9	389.0	0.1
J	11,850	30	205	1.8	394.9	394.9	395.9	1.0
K	13,025	10	40	9.2	406.4	406.4	406.5	0.1

¹ Feet above confluence with Sowashee Creek

² Combined Harbour Creek/Sowashee Creek floodway

³ Elevation computed without consideration of backwater effects from Sowashee Creek

TABLE 8

FEDERAL EMERGENCY MANAGEMENT AGENCY
LAUDERDALE COUNTY, MS
 AND INCORPORATED AREAS

FLOODWAY DATA

Harbour Creek

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
Loper Creek								
A	3,800	886	8,425	1.4	310.0	306.4 ²	307.2	0.8
B	5,000	823	6,375	1.8	310.0	306.9 ²	307.7	0.8
C-F*								

¹ Feet above confluence with Okatibbee Creek

² Elevation computed without consideration of backwater effects from Okatibbee Creek

*No floodway data computed

TABLE 8

FEDERAL EMERGENCY MANAGEMENT AGENCY
LAUDERDALE COUNTY, MS
 AND INCORPORATED AREAS

FLOODWAY DATA

Loper Creek

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
Magnolia Creek								
A	985	54	357	5.0	319.2	315.3 ²	315.5	0.2
B	2,005	28	265	6.7	322.9	322.9	323.5	0.6
C	3,005	109	299	5.9	328.3	328.3	328.7	0.4
D	4,000	41	258	6.9	332.3	332.3	333.0	0.7
E	5,025	68	432	4.1	336.7	336.7	337.4	0.7
F	6,000	59	547	3.2	339.7	339.7	340.5	0.8
G	7,030	50	334	5.3	345.9	345.9	346.1	0.2
H	8,345	55	319	5.5	348.8	348.8	349.7	0.9
I	9,330	39	187	6.9	351.5	351.5	352.1	0.6
J	10,345	50	228	5.7	355.3	355.3	355.4	0.1
K	11,585	71	415	3.1	360.5	360.5	361.0	0.5
L	12,510	40	289	4.5	369.0	369.0	369.3	0.3
M	13,350	81	529	2.4	372.5	372.5	373.3	0.8
N	14,330	125	647	1.0	379.8	379.8	380.8	1.0

¹ Feet above confluence with Sowashee Creek

² Elevation computed without consideration of backwater effects from Sowashee Creek

TABLE 8

FEDERAL EMERGENCY MANAGEMENT AGENCY
LAUDERDALE COUNTY, MS
 AND INCORPORATED AREAS

FLOODWAY DATA

Magnolia Creek

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
Newell Branch								
A	1,290	221	636	3.7	342.8	341.1 ²	341.6	0.5
B	2,090	60	419	5.6	344.9	344.9	345.5	0.6
C	5,750	329	1,119	1.1	356.3	356.3	356.9	0.6
D	6,900	41	260	4.7	359.7	359.7	360.3	0.6
E	7,600	84	447	2.7	363.5	363.5	364.2	0.7
F	9,000	36	226	4.7	365.4	365.4	366.4	1.0
G	10,500	30	178	5.1	370.1	370.1	370.1	0.0
H	11,500	35	150	6.1	377.2	377.2	377.2	0.0
I	12,500	40	173	5.3	382.7	382.7	382.8	0.1
J	13,600	71	224	4.1	392.1	392.1	392.6	0.5
K	15,450	20	118	3.8	405.2	405.2	406.2	1.0
L	17,040	18	66	4.5	411.7	411.7	412.3	0.6

¹ Feet above confluence with Sowashee Creek

² Elevation computed without consideration of backwater effects from Sowashee Creek

TABLE 8

FEDERAL EMERGENCY MANAGEMENT AGENCY
LAUDERDALE COUNTY, MS
 AND INCORPORATED AREAS

FLOODWAY DATA

Newell Branch

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
Okatibbee Creek								
A	98,600	488	5,170	5.2	287.0	286.5 ²	287.1	0.6
B	101,400	767	8,691	3.1	290.9	290.9	291.8	0.9
C	105,300	1,092	11,214	2.4	293.8	293.8	294.7	0.9
D	107,600	4,100	26,300	1.0	294.7	294.7	295.6	0.9
E	108,250	3,782	24,545	1.1	295.5	295.5	296.2	0.7
F	110,600	2,152	13,228	2.0	296.0	296.0	296.6	0.6
G	119,150	1,573	11,997	2.3	299.7	299.7	300.5	0.8
H-N*								

¹ Feet above mouth

² Elevation computed without consideration of backwater effects from Sowashee Creek

*No floodway data computed

TABLE 8

FEDERAL EMERGENCY MANAGEMENT AGENCY
LAUDERDALE COUNTY, MS
 AND INCORPORATED AREAS

FLOODWAY DATA

Okatibbee Creek

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
Robbins Branch								
A	1,441	57	351	5.1	327.8	326.9 ²	327.5	0.6
B	2,375	20	210	8.5	332.7	332.7	332.7	0.0
C	3,435	66	446	4.0	337.6	337.6	338.0	0.4
D	4,484	44	366	4.9	339.0	339.0	339.2	0.2
E	5,540	91	194	9.2	343.8	343.8	343.8	0.0
F	6,590	55	370	4.8	349.7	349.7	350.7	1.0
G	8,470	262	862	2.1	355.9	355.9	356.8	0.9
H	9,490	33	201	8.5	360.6	360.6	360.9	0.3
I	10,740	35	297	5.7	366.6	366.6	367.1	0.5
J	12,700	50	263	6.5	375.0	375.0	375.4	0.4
K	13,890	84	352	4.8	380.1	380.1	380.4	0.3
L	14,795	251	636	2.7	384.3	384.3	384.5	0.2
M	15,570	239	537	3.2	387.9	387.9	387.9	0.0
N	17,190	52	206	3.9	395.2	395.2	395.3	0.1
O	18,200	36	196	4.1	400.7	400.7	401.1	0.4
P	19,097	36	133	6.0	406.0	406.0	406.0	0.0
Q	21,100	22	89	9.0	427.9	427.9	428.1	0.2

¹ Feet above confluence with Sowashee Creek

² Elevation computed without consideration of backwater effects from Sowashee Creek

TABLE 8

FEDERAL EMERGENCY MANAGEMENT AGENCY
LAUDERDALE COUNTY, MS
 AND INCORPORATED AREAS

FLOODWAY DATA

Robbins Branch

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
Shear's Branch								
A	211	110	764	4.7	308.3	303.1 ²	304.0	0.9
B	2,270	519	4,836	0.7	321.2	321.2	321.6	0.4
C	2,587	378	2,361	1.5	321.3	321.3	321.7	0.4
D	3,168	373	2,735	1.3	322.7	322.7	323.1	0.4
E	3,749	310	1,839	1.6	323.1	323.1	323.6	0.5
F	4,646	139	655	4.1	326.4	326.4	327.2	0.8
G	5,333	239	1,304	2.0	329.6	329.6	330.4	0.8
H	5,597	359	1,442	1.8	330.2	330.2	330.9	0.7
I	5,966	267	772	3.0	331.3	331.3	331.9	0.6
J	6,389	420	2,249	1.0	334.9	334.9	335.6	0.7
K	6,758	179	745	2.8	335.4	335.4	336.0	0.6
L	7,181	75	423	4.9	337.5	337.5	338.0	0.5
M	7,867	140	609	2.9	343.4	343.4	344.0	0.6
N	8,026	130	787	2.2	344.4	344.4	344.8	0.4
O	8,976	50	398	4.4	348.3	348.3	349.1	0.8

¹ Feet above confluence with Sowashee Creek

² Elevation computed without consideration of backwater effects from Sowashee Creek

TABLE 8

FEDERAL EMERGENCY MANAGEMENT AGENCY
LAUDERDALE COUNTY, MS
 AND INCORPORATED AREAS

FLOODWAY DATA

Shear's Branch

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
Sowashee Creek								
A	2,600	4,017	27,641	0.7	284.1	284.1	285.1	1.0
B	4,450	3,604	22,195	0.9	284.6	284.6	285.6	1.0
C	6,160	3,039	19,996	1.0	285.2	285.2	286.2	1.0
D	6,995	3,004	17,015	1.2	285.6	285.6	286.6	1.0
E	7,920	2,275	13,976	1.4	286.3	286.3	287.2	0.9
F	9,130	2,662	16,770	1.2	287.0	287.0	287.9	0.9
G	10,590	3,116	15,897	1.3	287.9	287.9	288.8	0.9
H	11,970	2,951	19,169	1.0	288.7	288.7	289.6	0.9
I	13,650	1,598	10,895	1.8	289.6	289.6	290.5	0.9
J	14,895	248	3,739	5.3	290.2	290.2	291.1	0.9
K	16,325	575	5,089	3.9	291.1	291.1	292.0	0.9
L	17,240	850	5,533	3.6	291.6	291.6	292.4	0.8
M	18,670	900	7,273	2.7	292.5	292.5	293.2	0.7
N	20,080	1,000	4,180	5.2	293.4	293.4	294.0	0.6
O	20,970	1,006	4,311	5.0	295.4	295.4	296.1	0.7
P	22,300	1,151	7,397	2.9	297.7	297.7	298.6	0.9
Q	23,400	1,059	7,469	2.9	298.4	298.4	299.3	0.9
R	24,425	847	4,935	4.4	298.9	298.9	299.7	0.8
S	25,344	252	3,255	6.7	302.0	302.0	302.4	0.4

¹ Feet above confluence with Okatibbee Creek

TABLE 8

FEDERAL EMERGENCY MANAGEMENT AGENCY
LAUDERDALE COUNTY, MS
 AND INCORPORATED AREAS

FLOODWAY DATA

Sowashee Creek

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
Sowashee Creek (continued)								
T	25,487	373	4,524	4.8	302.6	302.6	303.0	0.4
U	26,700	832	7,893	2.8	303.3	303.3	304.0	0.7
V	28,680	385	4,229	4.3	305.8	305.8	306.0	0.2
W	29,447	805 ²	5,471	3.4	306.3	306.3	306.9	0.6
X	31,090	490	4,632	4.0	307.0	307.0	307.6	0.6
Y	32,620	474	4,252	4.3	308.1	308.1	308.8	0.7
Z	34,032	465	4,725	3.9	310.4	310.4	311.1	0.7
AA	34,875	178	2,858	6.5	312.0	312.0	312.4	0.4
AB	36,130	817	7,536	2.5	313.0	313.0	313.6	0.6
AC	37,651	240	3,505	5.0	313.9	313.9	314.3	0.4
AD	38,500	970	3,496	5.0	314.4	314.4	314.8	0.4
AE	39,780	176	2,820	6.3	315.1	315.1	315.6	0.5
AF	41,413	255	2,870	5.9	316.8	316.8	317.3	0.5
AG	41,860	680	3,792	4.5	317.4	317.4	317.8	0.4
AH	42,688	181	2,928	5.8	318.0	318.0	318.4	0.4
AI	43,191	266	3,168	5.4	318.6	318.6	319.0	0.4
AJ	43,720	735 ³	4,192	4.1	319.2	319.2	319.5	0.3
AK	44,550	200	3,096	5.5	319.5	319.5	319.9	0.4

¹ Feet above confluence with Okatibbee Creek

² Combined Gallagher Creek/Sowashee Creek floodway

³ Combined Magnolia Creek/Sowashee Creek floodway

TABLE 8

FEDERAL EMERGENCY MANAGEMENT AGENCY

LAUDERDALE COUNTY, MS
AND INCORPORATED AREAS

FLOODWAY DATA

Sowashee Creek

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
Sowashee Creek (continued)								
AL	45,530	194	3,043	5.6	320.1	320.1	320.5	0.4
AM	46,350	187	2,928	5.8	320.6	320.6	320.9	0.3
AN	47,930	254	3,091	5.5	321.7	321.7	321.9	0.2
AO	49,281	170	2,468	6.9	323.5	323.5	323.8	0.3
AP	49,705	304	4,119	3.5	324.6	324.6	324.9	0.3
AQ	51,378	300	2,677	5.3	327.1	327.1	327.2	0.1
AR	52,338	260	2,950	4.8	329.9	329.9	330.2	0.3
AS	54,000	725	4,677	4.0	334.7	334.7	335.4	0.7
AT	55,429	654	6,743	2.6	337.8	337.8	338.5	0.7
AU	56,450	684	8,021	2.2	338.1	338.1	339.1	1.0
AV	64,600	1,555	6,949	2.7	340.3	340.3	341.2	0.9
AW	71,580	2,159	10,870	0.8	342.9	342.9	343.8	0.9
AX	78,230	1,695	6,659	1.3	348.6	348.6	349.5	0.9
AY	80,780	300	1,403	6.3	352.6	352.6	353.6	1.0
AZ	82,561	1,049	7,047	1.0	356.9	356.9	357.4	0.5
BA	85,178	1,676	5,668	1.2	357.7	357.7	358.5	0.8
BB	88,143	1,227	10,034	0.7	365.8	365.8	366.2	0.4
BC	91,243	848	4,956	1.4	367.7	367.7	368.5	0.8

¹ Feet above confluence with Okatibbee Creek

TABLE 8

FEDERAL EMERGENCY MANAGEMENT AGENCY
LAUDERDALE COUNTY, MS
 AND INCORPORATED AREAS

FLOODWAY DATA

Sowashee Creek

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
Sowashee Creek (continued)								
BD	92,743	431	2,540	1.4	369.5	369.5	370.5	1.0
BE	95,571	848	3,769	1.0	374.7	374.7	375.6	0.9
BF	100,171	500	1,904	1.9	382.8	382.8	383.3	0.5

¹ Feet above confluence with Okatibbee Creek

TABLE 8

FEDERAL EMERGENCY MANAGEMENT AGENCY
LAUDERDALE COUNTY, MS
 AND INCORPORATED AREAS

FLOODWAY DATA

Sowashee Creek

5.0 INSURANCE APPLICATIONS

For flood insurance rating purposes, flood insurance zone designations are assigned to a community based on the results of the engineering analyses. These zones are as follows:

Zone A

Zone A is the flood insurance rate zone that corresponds to the 1-percent-annual-chance floodplains that are determined in the FIS report by approximate methods. Because detailed hydraulic analyses are not performed for such areas, no base (1-percent-annual-chance) flood elevations (BFEs) or depths are shown within this zone.

Zone AE

Zone AE is the flood insurance rate zone that corresponds to the 1-percent-annual-chance floodplains that are determined in the FIS report by detailed methods. Whole-foot BFEs derived from the detailed hydraulic analyses are shown at selected intervals within this zone.

Zone X

Zone X is the flood insurance rate zone that corresponds to areas outside the 0.2-percent-annual-chance floodplain, areas within the 0.2-percent-annual-chance floodplain, areas of 1-percent-annual-chance flooding where average depths are less than 1 foot, areas of 1-percent-annual-chance flooding where the contributing drainage area is less than 1 square mile (sq. mi.), and areas protected from the base flood by levees. No BFEs or depths are shown within this zone.

6.0 FLOOD INSURANCE RATE MAP

The FIRM is designed for flood insurance and floodplain management applications.

For flood insurance applications, the map designates flood insurance rate zones as described in Section 5.0 and, in the 1-percent-annual-chance floodplains that were studied by detailed methods, shows selected whole-foot BFEs or average depths. Insurance agents use zones and BFEs in conjunction with information on structures and their contents to assign premium rates for flood insurance policies.

For floodplain management applications, the map shows by tints, screens, and symbols, the 1- and 0.2-percent-annual-chance floodplains, floodways, and the locations of selected cross sections used in the hydraulic analyses and floodway computations.

The countywide FIRM presents flooding information for the entire geographic area of Lauderdale County. Historical data relating to the maps prepared for each community are presented in Table 9, "Community Map History."

7.0 OTHER STUDIES

Information pertaining to revised and unrevised flood hazards for each jurisdiction within

Lauderdale County has been compiled into this FIS. Therefore, this FIS supersedes all previously printed FIS reports, FIRMs, and/or FBFMs for all of the incorporated and unincorporated jurisdictions within Lauderdale County.

This FIS report supersedes or is compatible with all previous studies published on streams studied in this report and should be considered authoritative for the purposes of the NFIP.

8.0 LOCATION OF DATA

Information concerning the pertinent data used in the preparation of this study can be obtained by contacting Federal Insurance and Mitigation Division, FEMA Region IV, Koger-Center — Rutgers Building, 3003 Chamblee Tucker Road, Atlanta, GA 30341.

COMMUNITY NAME	INITIAL IDENTIFICATION	FLOOD HAZARD BOUNDARY MAP REVISIONS DATE	FIRM EFFECTIVE DATE	FIRM REVISIONS DATE
Marion, Town of	January 4, 1974	January 16, 1976 September 5, 1980	September 29, 1989	August 16, 1995 March 21, 2000 February 3, 2010
Meridian, City of	June 28, 1974	January 30, 1976	December 15, 1977	September 10, 1982 September 29, 1989 August 16, 1995 March 21, 2000 February 3, 2010
Lauderdale County (Unincorporated Areas)	January 13, 1978	--	September 29, 1989	August 16, 1995 March 21, 2000 February 3, 2010

TABLE 9

FEDERAL EMERGENCY MANAGEMENT AGENCY
LAUDERDALE COUNTY, MS
 AND INCORPORATED AREAS

COMMUNITY MAP HISTORY

9.0 **BIBLIOGRAPHY AND REFERENCES**

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17. Federal Emergency Management Agency. *Converting the National Flood Insurance Program to the North American Vertical Datum of 1988 - Guidelines for Community Officials, Engineers, and Surveyors.* 6/1/1992. 3-0170.
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10.0 REVISION DESCRIPTIONS

This section has been added to provide information regarding significant revisions made since the original FIS was printed. Future revisions may be made that do not result in the republishing of the FIS report. To assure that the user is aware of all revisions, it is advisable to contact the community repository of flood-hazard data located at:

- Lauderdale County Courthouse, Tax Assessor's Office
500 Constitution Avenue
Meridian, Mississippi 30301
- Marion Town Hall
6021 Dale Drive
Marion, Mississippi 39342
- Meridian City Hall
601 24th Avenue
Meridian, Mississippi 39302

10.1 First Revision (August 16, 1995)

The August 16, 1995, revisions to the countywide FIS updated hydrologic and hydraulic analyses for Sowashee Creek, Gallagher Creek, Magnolia Creek, Nanabe Creek, Robbins Branch, and Shear's Branch.

10.2 Second Revision (March 21, 2000)

The March 21, 2000, revisions to the countywide FIS added hydrologic and hydraulic analyses for sections of Gallagher Creek, Magnolia Creek, Robbins Branch, Harbour Creek, and Newell Branch.

10.3 Third Revision (February 3, 2010)

The February 3, 2010 countywide revision was initiated in support of the FEMA MapMod program and converted all data to NAVD, updated the map format, and created the digital data. This updated study resulted in some stream name changes (listed in Section 2.1). New limited detailed analyses were performed for Mclemore Branch, Sowashee Creek Tributary 8, Sowashee Creek Tributary 10, and Sowashee Creek Tributary 11. Floodplain boundaries of streams that had been previously studied by detailed methods were redelineated based on updated topographic information. All floodplain boundaries were refined based on the updated topographic data.

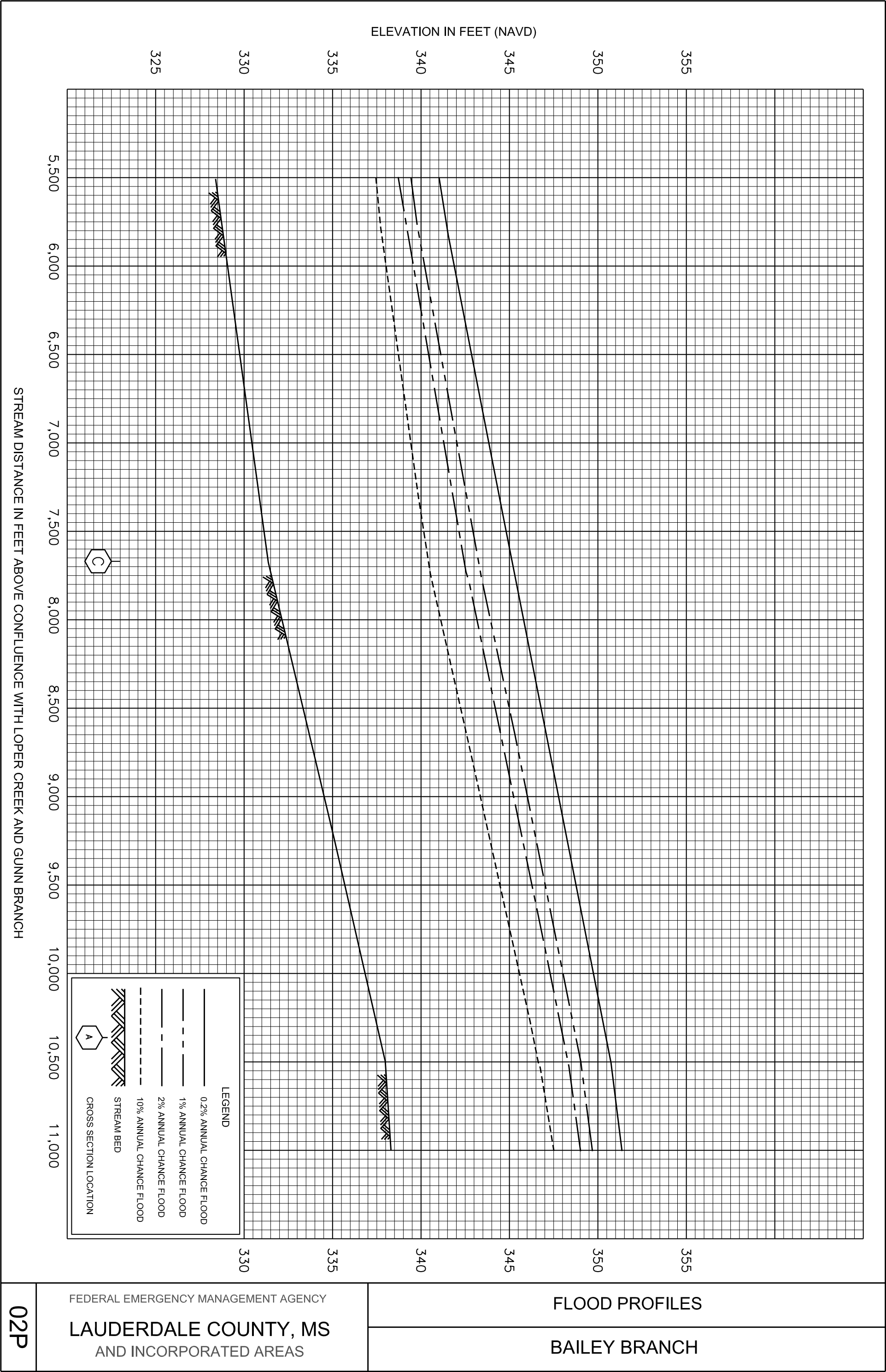
10.4 Fourth Revision (Month Day, 2011)

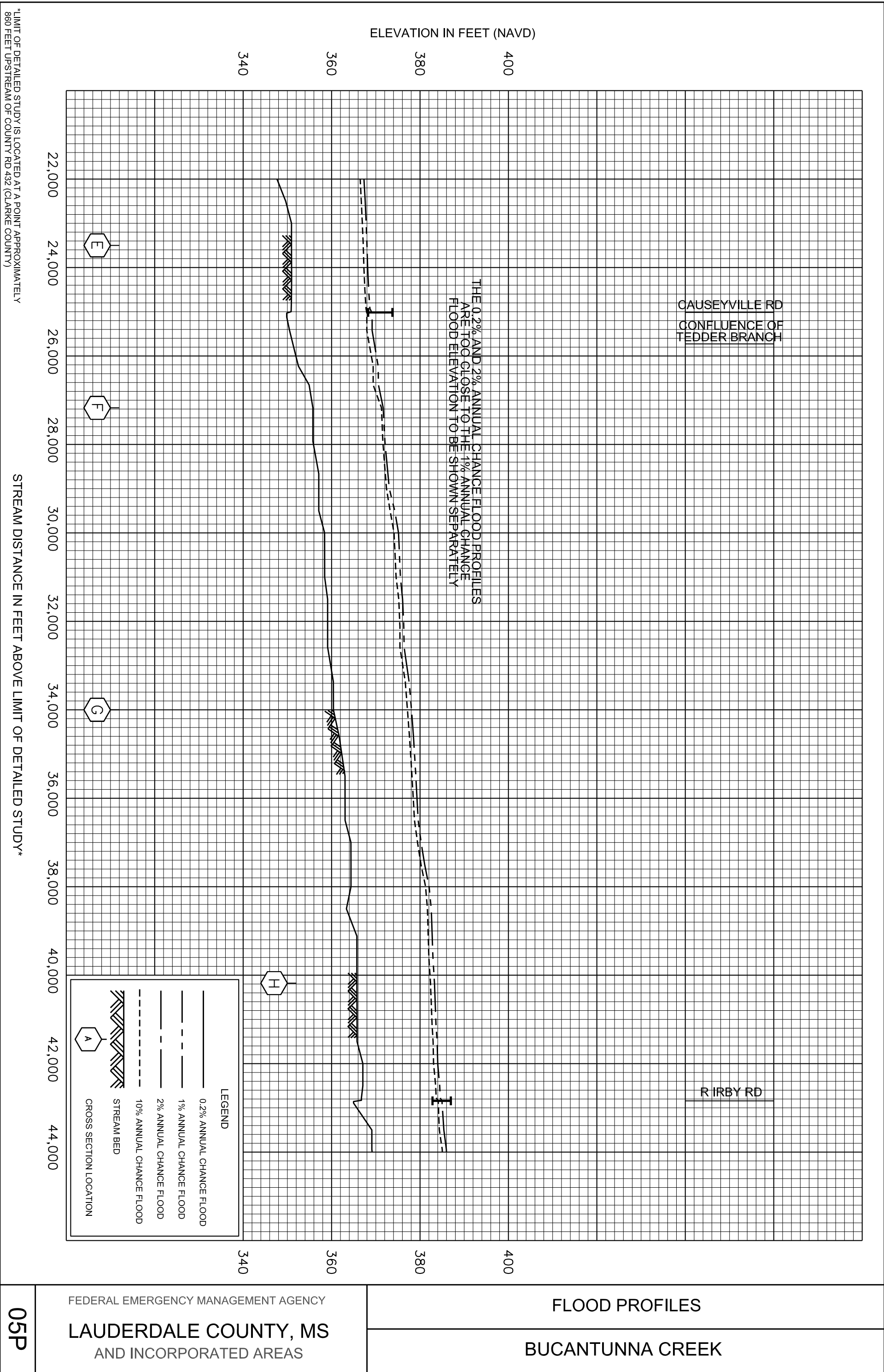
This xx/xx/xxxx revision was initiated in support of the FEMA Risk MAP Program.

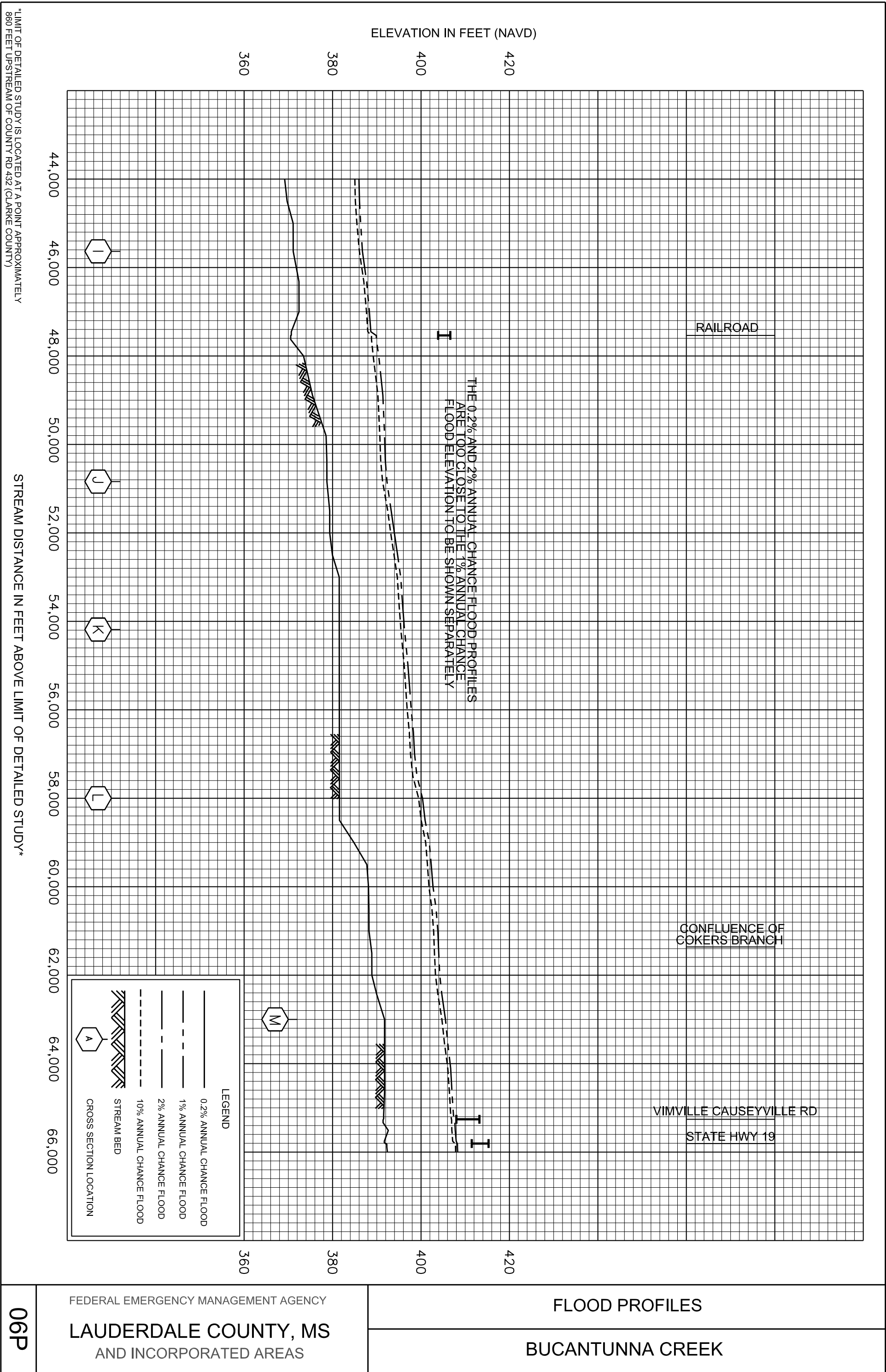
This revision involved updating the mapping for portions of Lauderdale County, Mississippi. The revision includes a new detailed study on Bucatunna Creek and new limited detailed studies on Coats Creek, Long Creek, Rodgers Creek, and Suqualena Creek Tributary 4. These revisions resulted in refined floodplain boundaries.

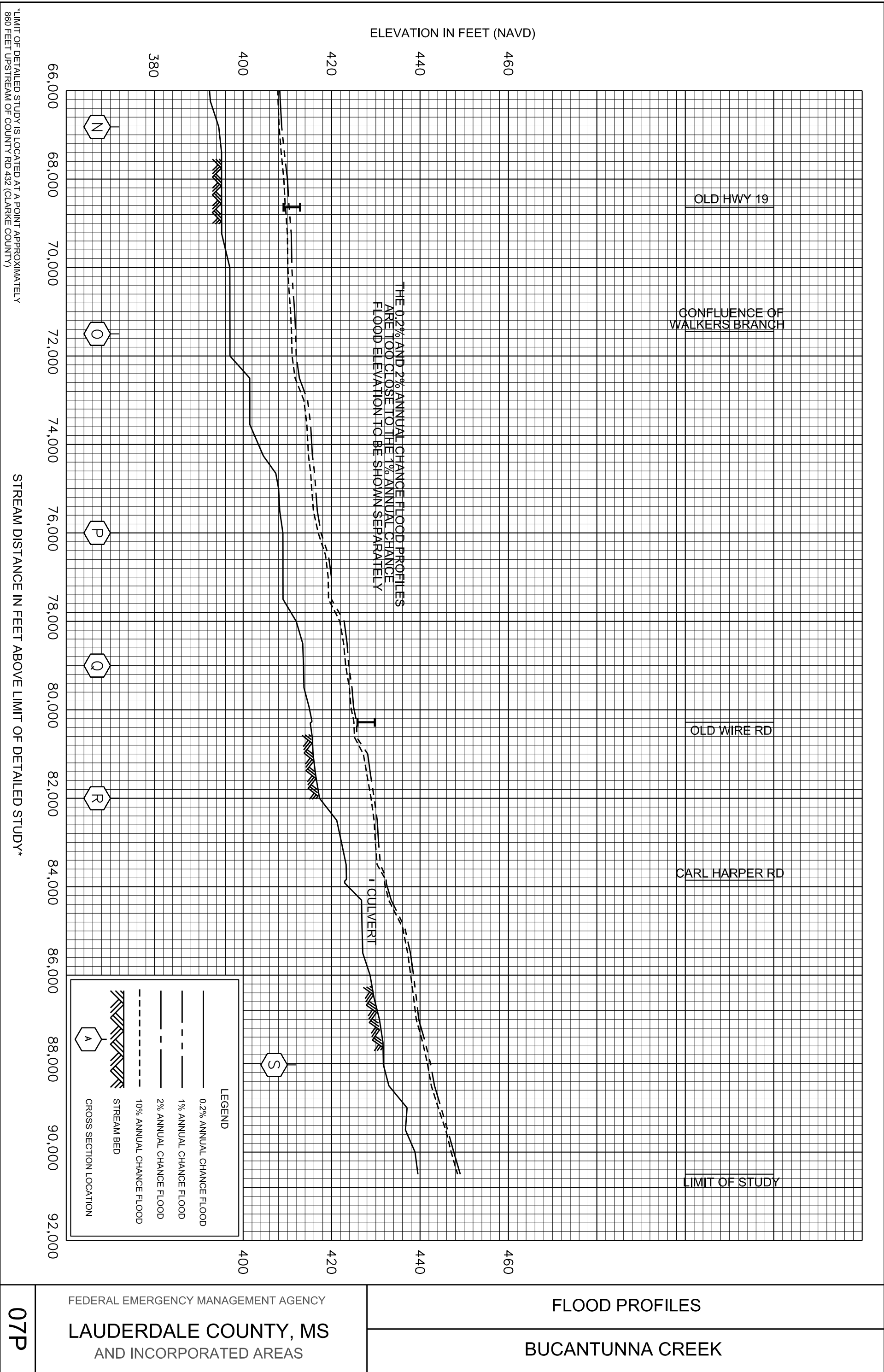
Floodplain boundaries for only the previously mentioned streams were updated. Therefore, only the panels affected by these floodplain boundaries have been updated. The following panels were updated in support of the Risk MAP Program:

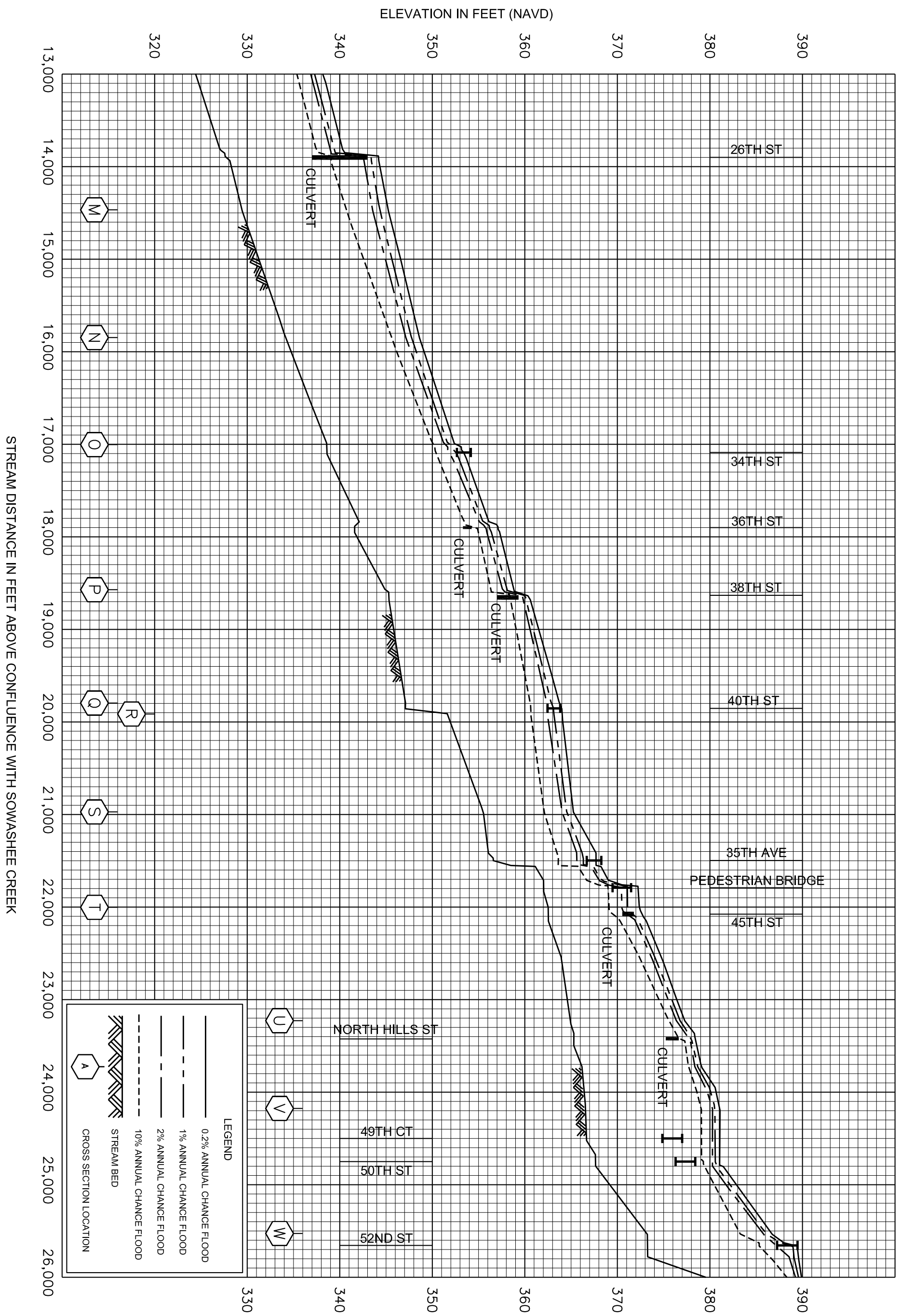
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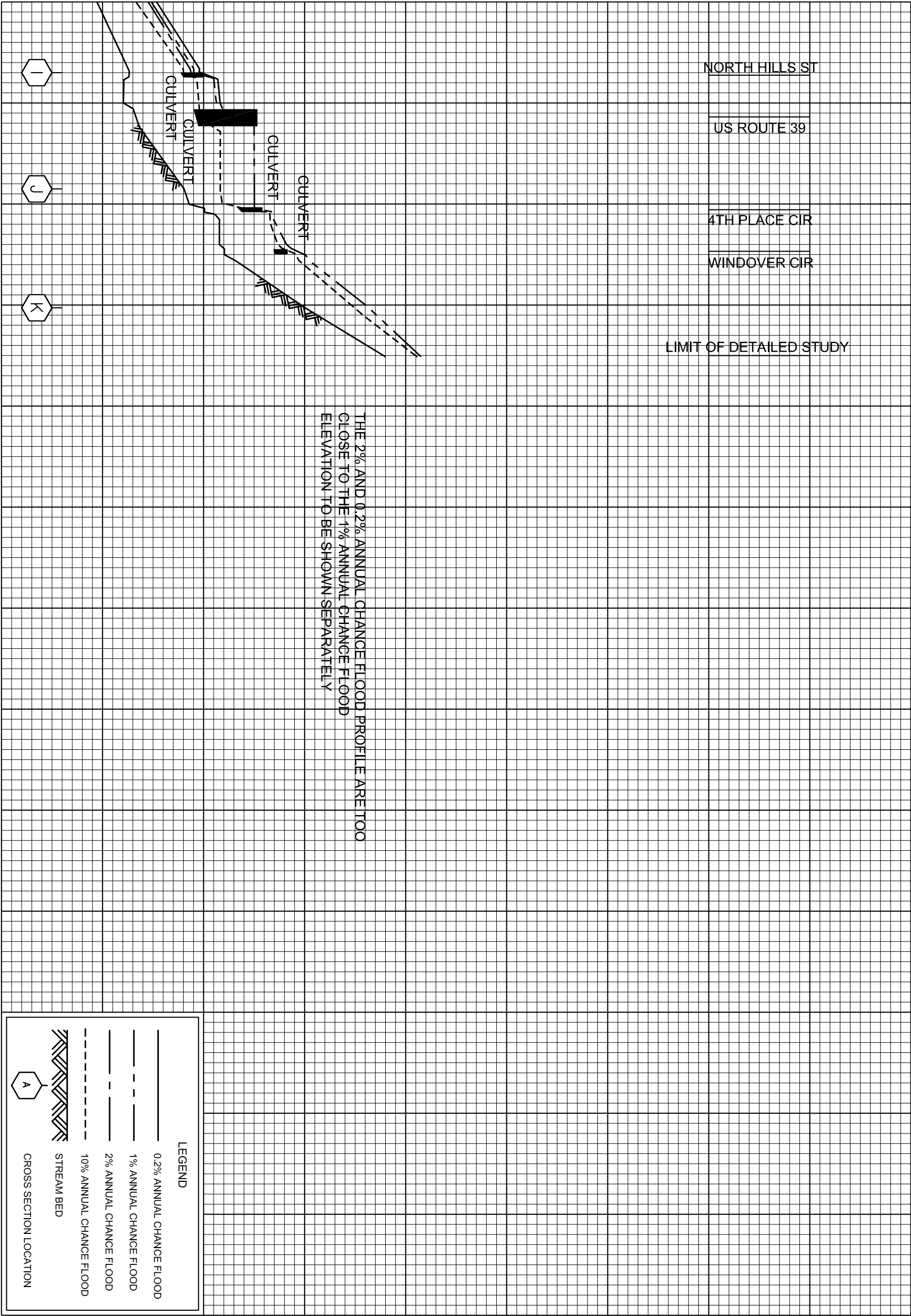












ELEVATION IN FEET (NAVD)

NORTH HILLS ST

US ROUTE 39

4TH PLACE CIR

WINDOVER CIR

LIMIT OF DETAILED STUDY

CULVERT

CULVERT

CULVERT

CULVERT

STREAM DISTANCE IN FEET ABOVE CONFLUENCE WITH SOWASHEE CREEK

FEDERAL EMERGENCY MANAGEMENT AGENCY

LAUDERDALE COUNTY, MS
AND INCORPORATED AREAS

FLOOD PROFILES

HARBOUR CREEK

