

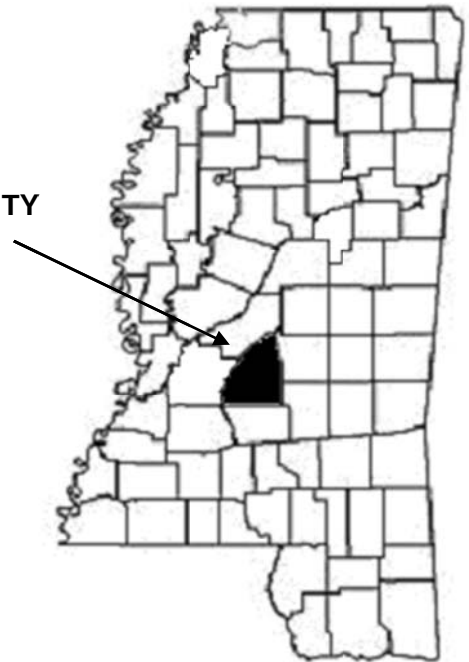
FLOOD INSURANCE STUDY



RANKIN COUNTY, MISSISSIPPI AND INCORPORATED AREAS

VOLUME 1 OF 2

RANKIN COUNTY



COMMUNITY NAME	COMMUNITY NUMBER
BRANDON, CITY OF	280143
FLORENCE, CITY OF	280144
FLOWOOD, CITY OF	280289
JACKSON, CITY OF	280072
PEARL, CITY OF	280145
PEARL RIVER VALLEY WATER SUPPLY DISTRICT	280338
PELAHATCHIE, TOWN OF	280146
PUCKETT, TOWNSHIP OF	280147
RANKIN COUNTY (UNINCORPORATED AREAS)	280142
RICHLAND, CITY OF	280299

REVISED:



Federal Emergency Management Agency

FLOOD INSURANCE STUDY NUMBER
28121CV001B

NOTICE TO
FLOOD INSURANCE STUDY USERS

Communities participating in the National Flood Insurance Program (NFIP) have established repositories of flood hazard data for floodplain management and flood insurance purposes. This Flood Insurance Study (FIS) may not contain all data available within the repository. It is advisable to contact the community repository for any additional data.

Part or all of this FIS may be revised and republished at any time. In addition, part of this FIS may be revised by the Letter of Map Revision process, which does not involve republication or redistribution of the FIS. It is, therefore, the responsibility of the user to consult with community officials and to check the community repository to obtain the most current FIS components.

Initial Countywide FIS Effective Date: November 3, 2003

Revised Countywide FIS Date:

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FLOOD INSURANCE STUDY

RANKIN COUNTY, MISSISSIPPI AND INCORPORATED AREAS

1.0 INTRODUCTION

1.1 Purpose of Study

This Flood Insurance Study (FIS) revises and supersedes the FIS reports and/or Flood Insurance Rate Maps (FIRMs) in the geographic area of Rankin County, Mississippi, including the Cities of Brandon, Florence, Flowood, Jackson, Pearl, and Richland; the Town of Pelahatchie; the Township of Puckett; the Pearl River Valley Water Supply District; and unincorporated areas of Rankin County (hereinafter referred to collectively as Rankin County).

For the City of Jackson, only the Jackson-Evers International Airport is included in this Flood Insurance Study since it is located within Rankin County. The remainder of the City of Jackson can be located in the Hinds County, MS, FIS. For the Pearl River Valley Water Supply District, only the portion that is located within Rankin County has been included in this study. The other portions of the Pearl River Valley Water Supply District can be located in the Hinds, Madison, Leake, and Scott Counties FIS.

This FIS aids in the administration of the National Flood Insurance Act of 1968 and the Flood Disaster Protection Act of 1973. This study has developed flood risk data for various areas of the community that will be used to establish actuarial flood insurance rates. This information will also be used by Rankin County to update existing floodplain regulations as part of the Regular Phase of the National Flood Insurance Program (NFIP), and by local and regional planners to further promote sound land use and floodplain development. Minimum floodplain management requirements for participation in the NFIP are set forth in the Code of Federal Regulations at 44 CFR, 60.3.

In some States or communities, floodplain management criteria or regulations may exist that are more restrictive or comprehensive than the minimum Federal requirements. In such cases, the more restrictive criteria take precedence and the State (or other jurisdictional agency) will be able to explain them.

1.2 Authority and Acknowledgments

The sources of authority for this FIS report are the National Flood Insurance Act of 1968 and the Flood Disaster Protection Act of 1973.

This FIS was prepared to include all jurisdictions within Rankin County in a countywide FIS. The authority and acknowledgments prior to this countywide FIS have been compiled from the previously printed FIS reports from the previously identified flood-prone jurisdictions within Rankin County and are shown below.

City of Brandon FIS

Initial FIS Effective Date: September 17, 1980

Revised FIS Date: September 18, 1986

The hydrologic and hydraulic analyses from the FIS report dated September 17, 1980, were performed by Michael Baker, Jr., Inc., for the Federal Insurance Administration (FIA), under Contract No. H-4631. That work was completed in February 1979.

For the September 18, 1986, FIS revision, the hydrologic and hydraulic analyses for Terrapin Skin Creek and Terrapin Skin Tributary 2 were performed by Michael Baker Jr., Inc (FEMA, 1986).

City of Florence FIS

Initial FIS Effective Date: February 1980

The hydrologic and hydraulic analyses for the FIS report dated February 1980, were performed by Michael Baker Jr., Inc., for the FIA, under Contract No. H-4631. That work was completed in February 1979 (FEMA, 1980).

City of Flowood FIS

Initial FIS Effective Date: December 15, 1982

Revised FIS Date: July 5, 1994

The hydrologic and hydraulic analyses for the December 15, 1982, FIS report were prepared by Michael Baker Jr., Inc., for the Federal Emergency Management Agency (FEMA), under Contract No. H-4631. That work was completed in March 1980.

For the July 5, 1994, FIS revision, the hydrologic and hydraulic analyses for the Pearl River were prepared by Neel-Schaffer, Inc., for FEMA under Contract No. EMW-88-C-2616 (FEMA, 1994). In addition, the hydrologic and hydraulic for Hog Creek and Hog Creek Tributary were taken from the June 16, 1993, FIS for the unincorporated areas of Rankin County.

City of Jackson FIS

Initial FIS Effective Date: April 1, 1980

Revised FIS Dates: June 25, 1982
September 18, 1986
December 16, 1988
April 4, 1994
June 16, 1999

For the original, April 1, 1980, FIS, the hydrologic and hydraulic analyses were performed by the USACE, Mobile District, for the FIA, under Inter-Agency Agreement

No. IAA-H-16-75, Project Order No. 20. This work, which was completed in February 1977, covered all significant flooding sources affecting the City of Jackson.

For the September 18, 1986 revision, the hydrologic and hydraulic analyses for the City of Jackson were obtained from the FIS for the Town of Flowood, Mississippi.

For the December 16, 1988, revision, the hydraulic analyses were performed by Michael Baker, Jr., and Deas, Eldridge, and Associates.

For the April 4, 1994, revision, the hydrologic and hydraulic analyses were performed by Neel-Schaffer, Inc., for FEMA, under Contract no. EMW-90-C-3100. That work was completed in July 1991.

For the June, 16, 1999, the hydrologic and hydraulic analyses were performed by Neel-Schaffer, Inc., and by the City of Jackson. That work was completed in July 1997.

City of Pearl FIS

Initial FIS Effective Date: June 15, 1982

Revised FIS Date: March 17, 1997

For the FIS dated June 15, 1982, the hydrologic and hydraulic analyses were prepared by Michael Baker, Jr., Inc., for FEMA, under Contract No. H-4631. That work was completed in April 1980.

For the FIS report dated March 17, 1997, the hydrologic and hydraulic analyses for Richland Creek were prepared by Neel-Schaffer, Inc., for FEMA, under Contract No. EMW-90-C-3129. That work was completed in December 1992. The hydrologic and hydraulic analyses for Conway Slough Tributary 2, Neely Creek Tributary 2, and Neely Creek (Left Channel) were prepared by Braswell Engineering, Inc., for FEMA, under Contract No. EMW-93-C-4147. That work was completed in February 1994.

Town of Pelahatchie FIS

Initial FIS Effective Date: July 1980

The hydrologic and hydraulic analyses for the FIS report dated July 1980 were performed by Michael Baker, Jr., Inc., for the FIA, under Contract No. H-4631. That work was completed in December 1978 (FEMA, July 1980).

Pearl River Valley Water Supply District FIS

Initial FIS Effective Date: March 2, 1993

Revised FIS Dates: January 19, 1996

For the FIS effective March 2, 1993, the hydrologic and hydraulic analyses were performed by Neel-Schaffer, Inc. (the Study Contractor), for FEMA, under Contract No. EMW-89-C-2826.

For the FIS revision effective January 19, 1996, the hydrologic and hydraulic analyses were obtained from Flood Characteristics of Mississippi Streams, prepared by the U.S. Geological Survey (U.S. Dept. of Interior, 1991).

Rankin County (Unincorporated Areas) FIS

Initial FIS Effective Date: June 15, 1982

Revised FIS Dates: June 19, 1989
June 16, 1993

The hydrologic and hydraulic analyses for the FIS report dated June 15, 1982, were performed by Michael Baker, Jr., Inc., for FEMA, under Contract No. H-4631. That work was completed in May 1980.

For the FIS report dated June 19, 1989, the hydraulic analyses for Richland Creek were performed by the U.S. Army Corps of Engineers (USACE), Vicksburg District, to reflect the channel work performed as part of the Soil Conservation Service Richland Creek Watershed Project (Dept. of Agriculture, 1987).

For the FIS report dated June 16, 1993, the hydrologic and hydraulic analyses were performed by Neel-Schaffer, Inc., under Contract Nos. EMW-87-C-2457 and EMW-88-C-2616 (FEMA, 1993).

City of Richland FIS

Initial FIS Effective Date: August 2, 1982

Revised FIS Date: March 15, 1994

For the FIS dated August 2, 1982, the hydrologic and hydraulic analyses were performed by Michael Baker, Jr., Inc., for FEMA, under Contract No. H-4631. That work was completed in January 1980.

For the FIS report dated March 15, 1994, the hydrologic and hydraulic analyses for the Pearl River were performed by Neel-Schaffer, Inc., under Contract No. EMW-88-C-2616 (FEMA, 1994).

Township of Puckett

No precountywide data is available for the Township of Puckett. No FIS was published for this community.

November 5, 2003, Countywide FIS

Revised hydrologic and hydraulic analyses were prepared for FEMA by FTN Associates, Ltd., under Contract No. EMA-98-CO-0090. That work was completed in February 2000.

This Revised Countywide FIS

The hydrologic and hydraulic analyses for this countywide FIS were performed by the State of Mississippi for the Federal Emergency Management Agency, under Contract No. EMA-

CA-5932. This study was completed in August 2011.

The following streams were included in the study:

- Brush Branch
- Brush Creek
- Butler Creek
- Conway Slough Tributary 1
- Indian Creek
- Indian Creek Tributary 1
- Mill Creek
- Neely Creek (Right Channel)
- Park Creek
- Pelahatchie Creek (Upper Reach)
- Richland Creek
- Richland Creek Tributary 2
- Richland Creek Tributary 3
- Steen Creek
- Woodrun Creek

Table 1: “Summary of Flooding Sources Presented in Current Study,” provides a summary of the flooding sources within Rankin County included in this current study, the contract number under which they were performed, and the communities affected by each.

Table 1: Summary of Flooding Sources Presented in Current Study

Flooding Source	Completion Date	Study Contractor(s)	Contract or Inter-Agency Agreement No.	Communities Affected
Brush Branch*	August 2011	State of Mississippi	EMA-CA-5932	Rankin County
Brush Creek*	August 2011	State of Mississippi	EMA-CA-5932	Rankin County
Butler Creek*	February 1979	Michael Baker, Jr., Inc.	H-4631	Rankin County
	August 2011	State of Mississippi	EMA-CA-5932	Rankin County
Clark Creek	May 1980	Michael Baker, Jr., Inc.	H-4631	Rankin County
Clark Creek Tributary	May 1980	Michael Baker, Jr., Inc.	H-4631	Rankin County
Conway Slough	January 1989	Michael Baker, Jr., Inc.	H-4631	Pearl, City of Richland, City of
Conway Slough Tributary 1*	April 1980	Michael Baker, Jr., Inc.	H-4631	Pearl, City of
	August 2011	State of Mississippi	EMA-CA-5932	Pearl, City of
Conway Slough Tributary 2	February 1994	Brasswell Engineering, Inc.	EMW-93-C-4147	Pearl, City of
Eutacutachee Creek	February 2000	FTN Associates	EMA-98-CO-0090	Pelahatchie, Town of Rankin County
Eutacutachee Creek Tributary 1	February 2000	FTN Associates	EMA-98-CO-0090	Rankin County
Eutacutachee Creek Tributary 2	February 2000	FTN Associates	EMA-98-CO-0090	Rankin County
Eutacutachee Creek Tributary 3	February 2000	FTN Associates	EMA-98-CO-0090	Rankin County
Eutacutachee Creek Tributary 4	February 2000	FTN Associates	EMA-98-CO-0090	Rankin County
Hog Creek	February 2000	FTN Associates	EMA-98-CO-0090	Flowood, City of Jackson, City of Pearl, City of
Hog Creek Tributary	March 1980	Michael Baker, Jr., Inc.	H-4631	Flowood, City of
*Flooding source restudied as part of the current revision				
** Data Not Available				

Table 1: Summary of Flooding Sources Presented in Current Study

Flooding Source	Completion Date	Study Contractor(s)	Contract or Inter-Agency Agreement No.	Communities Affected
Indian Creek*	February 1980	Michael Baker, Jr., Inc	H-4631	Florence, City of Rankin County
	August 2011	State of Mississippi	EMA-CA-5932	City of Florence
Indian Creek Tributary 1*	January 1980	Michael Baker, Jr., Inc	H-4631	Florence, City of Richland, City of Rankin County
	August 2011	State of Mississippi	EMA-CA-5932	City of Florence
Mill Creek*	March 1980	Michael Baker, Jr., Inc	H-4631	Flowood, City of Pearl River Valley Water Supply District Rankin County
	August 2011	State of Mississippi	EMA-CA-5932	Flowood, City of Pearl River Valley Water Supply District Rankin County
Mill Creek Tributary	March 1980	Michael Baker, Jr., Inc	H-4631	Flowood, City of Rankin County
Neely Creek	March 1980	Michael Baker, Jr., Inc	H-4631	Flowood, City of
Neely Creek (Left Channel)	February 1994	Brasswell Engineering, Inc.	EMW-93-C-4147	Flowood, City of Pearl, City of
Neely Creek (Right Channel)*	March 1980	Michael Baker, Jr., Inc	H-4631	Flowood, City of Pearl, City of
	August 2011	State of Mississippi	EMA-CA-5932	Flowood, City of Pearl, City of
Neely Creek Tributary 2	February 1994	Brasswell Engineering, Inc.	EMW-93-C-4147	Flowood, City of Pearl, City of
Park Creek	August 2011	State of Mississippi	EMA-CA-5932	Town of Pelahatchie Rankin County
*Flooding source restudied as part of the current revision				
** Data Not Available				

Table 1: Summary of Flooding Sources Presented in Current Study

Flooding Source	Completion Date	Study Contractor(s)	Contract or Inter-Agency Agreement No.	Communities Affected
Pearl River	May 1980 June 1993 March 1994	Michael Baker, Jr., Inc Neel-Schaffer, Inc. Neel-Schaffer, Inc.	H-4631 EMW-87-C-2457 EMW-88-C-2616	Flowood, City of Pearl, City of Pearl River Valley Water Supply District Rankin County Richland, City of
Pearl River Tributary 1	January 1980	Michael Baker, Jr., Inc	H-4631	Rankin County Richland, City of
Pearl River Tributary 2	February 2000	FTN Associates	EMA-98-CO-0090	Rankin County Richland, City of
Pearl River Tributary 3	February 2000	FTN Associates	EMA-98-CO-0090	Flowood, City of
Pelahatchie Bay Unnamed Tributary	February 2000	FTN Associates	EMA-98-CO-0090	Pearl River Valley Water Supply District
Pelahatchie Creek (Lower Reach)	May 1980	Michael Baker, Jr., Inc	H-4631	Pearl River Valley Water Supply District Rankin County
Pelahatchie Creek (Upper Reach)*	December 1978	Michael Baker, Jr., Inc.	H-4631	Pelahatchie, Town of Rankin County
	August 2011	State of Mississippi	EMA-CA-5932	Rankin County
Pelahatchie Creek Tributary	May 1980	Michael Baker, Jr., Inc	H-4631	Pearl River Valley Water Supply District Rankin County
Pelahatchie Creek Tributary 1	February 2000	FTN Associates	EMA-98-CO-0090	Pelahatchie, Town of
Pierce Creek	December 1978	Michael Baker, Jr., Inc.	H-4631	Pelahatchie, Town of Rankin County
Plummer Slough	May 1980	Michael Baker, Jr., Inc.	H-4631	Pearl River Valley Water Supply District Rankin County
Prairie Branch Canal	March 1980	Michael Baker, Jr., Inc	H-4631	Flowood, City of Pearl, City of
*Flooding source restudied as part of the current revision				
** Data Not Available				

Table 1: Summary of Flooding Sources Presented in Current Study

Flooding Source	Completion Date	Study Contractor(s)	Contract or Inter-Agency Agreement No.	Communities Affected
Prairie Branch Canal Tributary 1	February 2000	FTN Associates	EMA-98-CO-0090	Flowood, City of
Richland Creek*	February 2000	FTN Associates	EMA-98-CO-0090	Brandon, City of Pearl, City of Rankin County Richland, City of
	August 2011	State of Mississippi	EMA-CA-5932	Brandon, City of Rankin County
Richland Creek Tributary 1	April 1980	Michael Baker, Jr., Inc	H-4631	Pearl, City of Rankin County
Richland Creek Tributary 2	August 2011	State of Mississippi	EMA-CA-5932	Brandon, City of
Richland Creek Tributary 3	August 2011	State of Mississippi	EMA-CA-5932	Brandon, City of
Spring Branch	May 1980	Michael Baker, Jr., Inc	H-4631	Pearl River Valley Water Supply District Rankin County
Squirrel Branch	January 1980	Michael Baker, Jr., Inc	H-4631	Rankin County, Richland, City of
Steen Creek*	February 1979	Michael Baker, Jr., Inc	H-4631	Florence, City of Rankin County
	August 2011	State of Mississippi	EMA-CA-5932	Florence, City of Rankin County
Terrapin Skin Creek	February 2000	FTN Associates	EMA-98-CO-0090	Brandon, City of Pearl, City of Rankin County
Terrapin Skin Creek Tributary 1	February 1979	Michael Baker, Jr., Inc	H-4631	Brandon, City of
Terrapin Skin Creek Tributary 2	September 1986	Michael Baker, Jr., Inc.	**	Brandon, City of
Town Branch	February 1979	Michael Baker, Jr., Inc	H-4631	Florence, City of
*Flooding source restudied as part of the current revision				
** Data Not Available				

Flooding Source	Completion Date	Study Contractor(s)	Contract or Inter-Agency Agreement No.	Communities Affected
Turtle Creek	May 1980	Michael Baker, Jr., Inc	H-4631	Pearl River Valley Water Supply District Rankin County
Woodrun Creek*	February 2000	FTN Associates	EMA-98-CO-0090	Pearl, City of
	August 2011	State of Mississippi	EMA-CA-5932	Pearl, City of
All Zone A Streams	February 2000	FTN Associates	EMA-98-CO-0090	All

*Flooding source restudied as part of the current revision

** Data Not Available

The upstream portion of Neely Creek (Right Channel) previously above the divergence of Prairie Branch Canal is now the upstream portion of Prairie Branch Canal.

For the digital base map information files, the digital orthophotography was acquired in September 2010, by the United States Department of Agriculture under the National Agriculture Imagery Program, with the imagery processed to a 2-foot pixel resolution (U.S. Department of Agriculture, 2010).

The digital FIRM was produced using the State Plane Coordinate System, Mississippi West, FIPSZONE 2302. The horizontal datum was the North American Datum of 1983, GRS 80 spheroid. Distance units were measured in U.S. feet.

1.3 Coordination

An initial Consultation Coordination Officer's (CCO) meeting is held with representatives from FEMA, the community, and the study contractor to explain the nature and purpose of a FIS, and to identify the streams to be studied by detailed methods. A final CCO meeting is held with representatives from FEMA, the community, and the study contractor to review the results of the study. The dates of the pre-countywide initial and final CCO meeting held for Rankin County and the incorporated communities are shown in Table 2, "Historical CCO Meeting Dates."

Table 2: Historical CCO Meeting Dates

Community Name	Initial CCO Date	Final CCO Date
Brandon, City of	February 1978	August 29, 1979
Florence, City of	February 1978	August 28, 1979
Flowood, City of	February 1978	December 5, 1980
Jackson, City of	January 8, 1975	September 11, 1980
Pearl, City of	*	November 30, 1993
Pearl River Valley Water Supply District	June 8, 1988	August 27, 1991
Pelahatchie, Town of	February 1978	April 10, 1979
Rankin County (Unincorporated Areas)	**	December 5, 1991
Richland, City of	February 1978	January 20, 1981
* No initial meeting held. Community notified by FEMA in a letter dated December 15, 1992, that a revision to the FIS would be performed.		
**Date not available		

For the initial countywide FIS dated November 5, 2003, all jurisdictions within Rankin County were notified by FEMA letters dated August 28, 2000, that a countywide FIS was being prepared using the analyses prepared by FTN Associates, Ltd. A final CCO meeting was held on September 13, 2002, and was attended by representatives of FEMA and the State.

For this countywide FIS revision, a Project Scoping Meeting was held on November 10, 2009. Attendees for these meetings included representatives from the Mississippi Department of Environmental Quality, Mississippi Emergency Management Agency, FEMA National Service Provider, Rankin County, City of Brandon, City of Florence, City of Flowood, City of Jackson, City of Pearl, City of Richland, Pearl River Valley Water Supply District, and the State Study Contractor. Coordination with county officials and Federal, State, and regional agencies produced a variety of information pertaining to floodplain regulations, available community maps, flood history, and other hydrologic data. A final meeting was held on XXXXXX, XX, 2012 with officials and the public to review the new maps. All problems raised in the meetings have been addressed.

2.0 AREA STUDIED

2.1 Scope of Study

This FIS report covers the geographic area of Rankin County, MS, including the incorporated communities listed in Section 1.1. The scope and methods of this study were proposed to, and agreed upon, by FEMA and the State of Mississippi.

For this countywide FIS, detailed streams were studied by detailed and limited detailed methods. This study type entails collecting basic field measurements of hydraulic structures and channel geometry. Generalized roughness values are estimated from land-use data, aerial photography, and photographs collected during survey. Channel and overbank reach lengths are computed using GIS methods. Model results are calibrated to known stage values, as they are available and deemed reliable

The areas studied by detailed methods were selected with priority given to all known flood hazards and areas of projected development or proposed construction. The flooding sources studied by detailed methods are presented in Table 3, “Flooding Sources Studied by Detailed Methods.”

Table 3: Flooding Sources Studied by Detailed Methods

Flooding Source	Reach Length (miles)	Study Limits
Butler Creek	2.6	From the confluence with Steen Creek to a point approximately 11,318 feet upstream of U.S. Highway 49
Conway Slough Tributary 1	1.7	From just downstream of Childre Road to just downstream of South Pearson Road
Indian Creek	2.8	From the confluence with Steen Creek to a point approximately 4,094 feet upstream of Crosspark Boulevard
Indian Creek Tributary 1	1.8	From the confluence with Indian Creek to a point approximately 12,292 feet upstream of U.S. Highway 49
Mill Creek	3.8	From State Highway 25 to State Highway 471
Neely Creek Right Channel	1.4	From a point approximately 2,640 feet downstream of Flynn Drive to a point 791 feet upstream of Harle Street
Steen Creek	8.2	From a point approximately 1,013 feet downstream of South Church Street to R.T. Braddy Road

Limits of the detailed study are indicated on the Flood Profiles (Exhibit 1) and on the FIRM (Exhibit 2).

The areas studied by limited detailed methods were selected for areas having low to moderate development potential or flood hazards. The scope and methods of study were proposed to and agreed upon by FEMA and Rankin County. The flooding sources studied by limited detailed methods are presented in Table 4, “Flooding Sources Studied by Limited Detailed Methods.”

Table 4: Flooding Sources Studied by Limited Detailed Methods

Flooding Source	Reach Length (miles)	Study Limits
Brush Branch	1.3	From the confluence with Brush Creek to a point approximately 3,770 feet upstream of Lakeview Road
Brush Creek	6.2	From the confluence with Pelahatchie Creek to a point approximately 10,716 feet upstream of Andrew Chapel Road
Park Creek	2.8	From the confluence with Pelahatchie Creek to a point approximately 5,571 feet upstream of Lake Road
Pelahatchie Creek	3.6	From the confluence of Eutacutachee Creek to a point approximately 800 feet downstream of U.S. Highway 80.
Richland Creek	0.6	From a point just downstream of McDonald Lane to a point approximately 2,637 feet upstream of Tiffany Drive
Richland Creek Tributary 2	2.2	From the confluence with Richland Creek to a point approximately 3,565 feet upstream of Kennedy Farm Parkway
Richland Creek Tributary 3	2.0	From the confluence with Richland Creek to a point approximately 4,420 feet upstream of Trickhambridge Road
Woodrun Creek	1.6	From Greenville Road to U.S. Interstate 20

This countywide FIS also incorporates the determination of letters issued by FEMA resulting in Letters of Map change as shown in Table 5, “Letters of Map Revision (LOMRs) Incorporated into Current Study.”

**Table 5: Letters of Map Revision (LOMRs)
Incorporated into Current Study**

Case Number	Flooding Source(s)	Communities Affected	Effective Date
03-04-401P	Pearl River Tributary 3	City of Flowood	11/19/2003
04-04-093P	Pelahatchie Creek Tributary	Rankin County	3/31/2004
05-04-A319P	Terrapin Skin Creek	City of Brandon	2/27/2006
06-04-B146P	Richland Creek	City of Brandon, Rankin County	4/28/2006
06-04-B935P	Neely Creek (Left Channel)	City of Pearl	12/27/2006
06-04-B977P	Terrapin Skin Creek Terrapin Skin Creek Tributaries 1 and 2	City of Brandon	11/22/2006
06-04-BN09P	Mill Creek	Rankin County	2/12/2007
06-04-C397P	Prairie Branch Tributary 1	City of Flowood, Rankin County	3/29/2007
07-04-2653P	Pearl River	City of Jackson, City of Flowood	8/28/2007
07-04-3666P	Terrapin Skin Creek	City of Brandon, City of Pearl, Rankin County	8/24/2007
07-04-5839P	Pearl River	City of Flowood, Rankin County	9/14/2007
08-04-5000P	Conway Slough Tributary	City of Pearl	10/29/2008
08-04-5371P	Richland Creek	City of Brandon	8/13/2009
09-04-2764P	Indian Creek Tributary 1	City of Richland	7/30/2009
09-04-6879P	Richland Creek	City of Brandon, Rankin County	3/25/2010
10-04-5433P	Pearl River	City of Flowood	8/10/2010

Approximate study boundaries within Rankin County were redigitized using updated terrain models listed in Section 4.1.

2.2 Community Description

Rankin County is located in the west-central portion of Mississippi. It is situated approximately 200 miles south of Memphis, Tennessee, 190 miles north of New Orleans, Louisiana, and immediately east of Jackson, Mississippi. According to the U.S. Bureau of the Census figures, the estimated 2009 population is 143,124 (U.S. Census, 2011). Rankin County is contiguous to Hinds County to the west, Madison County to the north, Scott County to the east, Simpson County to the south, and Smith County to the southeast.

Within the floodplains studied, the development is limited, consisting of single-family residences and a few private businesses. The majority of development is adjacent to U.S. Routes 49 and 80, U.S. Interstate Route 20, and State Routes 18, 25, 43, 468, 469, and 471.

The topography of Rankin County varies from moderately high hills with steep slopes and narrow valleys through lower, more rolling hills, wider valleys, and gentler slopes, to rather broad, flat, alluvial plains. The drainage basins have moderately well-drained to poorly drained soils. Vegetation in the drainage basins consists mostly of pine trees and hardwoods, with light to very heavy undergrowth.

Rankin County has a warm, temperate climate, characterized by long summers and short winters. The annual precipitation averages 56 inches. Temperatures range from a January average of 45 degrees Fahrenheit (°F) to a July average of 81 °F (National Oceanic and Atmospheric Administration, 2011).

2.3 Principal Flood Problems

Intense seasonal rains and occasional tropical storms or hurricanes are the cause of periodic flooding in Rankin County.

The principal flood problems arise from the overflow into the relatively flat, developed overbanks along the rivers and streams in the county. Extensive flooding along the reach of the Pearl River occurs periodically. Past severe floods on the river, measured at the Woodrow Wilson gaging station, occurred on April 25, 1874; December 5, 1880; April 21, 1900; March 31, 1902; and December 21, 1961. (FEMA, 1979 and USACE, 1973). The recorded flood height for the March 31, 1902 flood was 272.3 feet North American Vertical Datum of 1988 (NAVD 88). The recorded flood heights for the other floods varied from 271.4 feet NAVD 88 to 272.0 NAVD 88. The most severe flood on record occurred on April 17, 1979. The recorded flood height measured at U.S. Route 80 (new gaging site) on April 17 was 276.97 feet NAVD 88 and was estimated to be a 300-500 year event.

No stream gage information is available in the Brandon or Florence areas for determining the maximum flood of record or the magnitude of the most recent flood of significance.

Factors that may contribute to flood problems are bridges, culverts, and stream reaches, which either have inadequate capacities or are subject to constriction due to debris collection and siltation.

The rapid population growth and economic development that the county is presently experiencing will undoubtedly lead to increased pressure for floodplain use.

2.4 Flood Protection Measures

Portions of western Rankin County are protected from the Pearl River by the East Jackson Levee System. The pumping station is located over Conway Slough within the Richland corporate limits and helps drain local runoff from the adjacent streams in the Cities of Flowood and Pearl when the Pearl River is above flood stage. The levee system is operated and maintained by the Rankin-Hinds Pearl River Flood and Drainage Control District. East Jackson Levee System has been certified by the USACE, Vicksburg District, Mississippi, to meet all the requirements in Section 65.10 of the NFIP.

In 1958, the Mississippi legislature passed enabling legislation to create the PRVWSD, an autonomous State Agency empowered to construct, operate, and maintain a reservoir on the Pearl River. In 1964, construction was completed on Ross Barnett Reservoir, approximately 15 miles upstream from the City of Jackson. The reservoir, with storage approximately 310,000 acre feet, ensures approximately 150 million gallons per day for water supply and provides a 30,000 acre lake for recreation. The relatively shallow reservoir contains no designated storage for flood control and has been operated on an emergency basis to provide limited regulation during major floods at the City of Jackson (USACE, 1985).

3.0 ENGINEERING METHODS

For the flooding sources studied by detailed methods in the communities, standard hydrologic and hydraulic study methods were used to determine the flood hazard data required for this study. Flood events of a magnitude that are expected to be equaled or exceeded once on the average during any 10-, 50-, 100-, or 500-year period (recurrence interval) have been selected as having special significance for floodplain management and for flood insurance rates. These events, commonly termed the 10-, 50-, 100-, and 500-year floods, have a 10-, 2-, 1-, and 0.2-percent chance, respectively, of being equaled or exceeded during any year. Although the recurrence interval represents the long-term, average period between floods of a specific magnitude, rare floods could occur at short intervals or even within the same year. The risk of experiencing a rare flood increases when periods greater than 1 year are considered. For example, the risk of having a flood that equals or exceeds the 1-percent-annual-chance flood in any 50-year period is approximately 40 percent (4 in 10); for any 90-year period, the risk increases to approximately 60 percent (6 in 10). The analyses reported herein reflect flooding potentials based on conditions existing in the community at the time of completion of this study. Maps and flood elevations will be amended periodically to reflect future changes.

3.1 Hydrologic Analyses

3.1.1 Methods for Flooding Sources with New or Revised Analyses in Current Study

Hydrologic analyses were carried out to establish peak discharge-frequency relationships for each flooding source studied by detailed methods affecting Rankin County. Peak discharges for the streams studied by limited detailed and detailed methods were calculated based on USGS regional regression equations (U.S. Dept. of the Interior, 1991). For the discharges calculated based on regional regression equations, the rural regression values were modified to reflect nearby stream gages and/or urbanization as necessary.

3.1.2 Methods for Flooding Sources Incorporated from Previous Studies

Each jurisdiction within Rankin County, with the exception of the Township of Puckett, had a previously printed FIS report. The hydrologic analyses described in those reports have been compiled and are summarized below.

Discharge-frequency relationships for the Pearl River were determined using information from previous reports (USACE, 1985; U.S. Department of the Interior, Floods of April 1979, Mississippi, Alabama, and Georgia; U.S. Department of the Interior, 1976; and U.S. Department of the Interior, 1983). The

relationships were developed through statistical analysis of discharge data from various streamflow gages.

Peak discharge values for Hog Creek and Hog Creek Tributary were determined using USGS regression equations (U.S. Department of the Interior 1976).

Peak discharges for Conway Slough and Neely Creek were developed by the hydrograph method based upon the use of SCS unit hydrograph technique (U.S. Department of Agriculture, 1972) for the drainage basin, in conjunction with storm frequency rainfall developed from the U.S. Weather Service Technical Paper 40 (U.S. Department of Commerce, 1961). The sub-area runoff hydrographs were routed and combined through the basin by using the HEC-1 flood hydrograph computer program developed by the USACE (USACE, 1991).

Peak discharges for Conway Slough Tributary 2 and Neely Creek Tributary 2 were determined using updated regional regression equations published by the USGS (U.S. Department of the Interior, 1991). Adjustments for urbanization were made using methods presented in USGS Water Supply Paper 2207 (U.S. Department of the Interior, 1983). The three-parameter urbanization equations were used. Drainage basin areas were delineated and planimetered from USGS 7.5-minute quadrangles (U.S. Department of the Interior, 1980, et cetera).

A USGS gaging station located on Pelahatchie Creek, 2 miles south of Fannin Road at the State Highway 471 bridge, provided values of peak discharges for a log-Pearson Type III distribution of annual peak flow data from 1938-1965 and 1973-1975 (U.S. Department of the Interior, 1982 and U.S. Department of the Interior, 1938). Peak discharges were adjusted and transferred upstream using the USGS methodology of stream gage data transfer (U.S. Department of the Interior, 1976).

Peak discharge values for Clark Creek, Clark Creek Tributary, Hog Creek, Hog Creek Tributary, Mill Creek, Mill Creek Tributary, Plummer Slough, Spring Branch, Turtle Creek, and Pelahatchie Creek Tributary were determined by regression equations developed by the USGS for urban watersheds in the United States (U.S. Department of the Interior, 1983).

For the remaining detailed studied streams, peak discharge computations were based on a regional flood-frequency report prepared by the USGS (U.S. Department of the Interior, 1976). Techniques for estimating future flood magnitudes were developed in the USGS report using records of annual peaks for 89 basins and observed annual peak-flow data for 221 stream gaging stations. The length of record for 82 of the 221 stations with actual records is 25 years or more. The natural drainage areas for which flood frequency is defined range from 0.14 to 6,630 square miles. Multi-regression analyses were used to average the chance variability of the data and relate flood frequency to basin characteristics, the most significant being drainage area, slope, and length. Because the regional analysis is applicable only to non-urbanized basins, adjustment factors were applied where appropriate to include consideration for urbanization along the streams in the study area. Urbanization factors were determined for Terrapin Skin Creek and Terrapin Skin Creek Tributary 2 using methods contained in a report prepared by the USGS (U.S. Department of the Interior, 1974). The percentage of impervious area within the Terrapin Skin Creek drainage basin as related to a composite

runoff curve number was determined from Urban Hydrology for Small Watersheds (U.S. Department of Agriculture, 1975).

Peak discharges were obtained for approximate study streams by the method described previously (U.S. Dept. of the Interior, 1976).

For the Pearl River Valley Water Supply District, two different hydrologic models were developed by the USACE in their flood control projects. The HEC-1 model was used to simulate the basin's runoff characteristics for project design and to analyze the effect of the project at downstream locations (U.S. Department of the Interior, 1986). The HEC-5 model was used to simulate the long-term operation of the reservoir and to analyze the effect of the proposed Shoccoe Dam Flood Control Project at the reservoir (USACE, 1985).

For the City of Jackson, no hydrologic studies were published in the FIS for any streams located within the Jackson-Evers International Airport.

The HEC-5 model simulates the operation of the reservoir by using the storage curve (elevation-volume) and the spillway rating (elevation-discharge). The model output represents daily outflows and pool elevations. The model was then operated to compute discharges for various frequencies of floods for natural conditions.

Peak discharge-frequency data for several of the tributaries of the Pearl River studied in detail were determined by regression equations (USACE, 1983).

November 5, 2003, Countywide Analyses

There were occasions where the peak flow used was extracted from the effective unincorporated areas of Rankin County, or respective community, FIS.

The peak discharge computations were based on the regional flood frequency report prepared by the USGS in cooperation with the Mississippi State Highway Department (U.S. Dept. of the Interior, 1991). The report includes rural regression equations applicable to watersheds located in one of three Mississippi regions, Delta-3, West-2, and East-1. Rankin County is located in Region East-1. The peak discharge values for other streams located in more urbanized watersheds were also modified to account for the degree of urbanization.

A summary of drainage area-peak discharge relationships for all the streams studied by detailed methods is shown in Table 6, "Summary of Discharges."

Table 6: Summary of Discharges

Flooding Source and Location	Drainage Area (Square miles)	Peak Discharges (Cubic Feet per Second)			
		10-percent	2-percent	1-percent	0.2-percent
BRUSH BRANCH					
At Confluence with Brush Creek	2.00	*	*	1,114	*
BRUSH CREEK					
At confluence with Pelahatchie Creek	7.70	*	*	2943	*
At confluence with Brush Branch	4.03	*	*	2211	*
Approximately 1,119 feet upstream of dam	2.02	*	*	1474	*
At Cross-Section H	1.04	*	*	896	*
BUTLER CREEK					
At confluence with Steen Creek	5.74	2559	2,162	3,574	3,100
Approximately 3,140 feet downstream of Williams Road	4.30	1470	3,326	2,415	4,194
At Williams Road	2.96	1,317	1,995	2,335	3,517
CLARK CREEK					
At confluence with Pelahatchie Creek	12.47	2,750	4,550	5,410	7,700
Just downstream of Clark Creek Road	11.10	2,650	4,260	5,200	7,400
Approximately 2,900 feet upstream of Holly Bush Road	7.22	2,040	3,300	3,940	5,560
Approximately 1.1 miles upstream of Mount Helen Road	5.68	1,950	3,100	3,680	5,200
Approximately 2.0 miles upstream of Mount Helen Road	4.30	1,540	2,470	2,900	4,100
CLARK CREEK TRIBUTARY					
At Holly Bush Road	2.51	860	1340	1610	2500
Approximately 3,800 feet upstream of Holly Bush Road	0.85	630	970	1170	1760

* Data Not Available

Table 6: Summary of Discharges

Flooding Source and Location	Drainage Area (Square miles)	Peak Discharges (Cubic Feet per Second)			
		10-percent	2-percent	1-percent	0.2-percent
CONWAY SLOUGH					
At East Jackson Pumping Station	7.54	3,502	4,435	4,925	6,966
At Railroad	4.16	2,334	2,948	3,265	4,427
CONWAY SLOUGH TRIBUTARY 1					
At Interstate 20	1.64	1,346	1,711	1,790	2,143
Approximately 1,576 feet upstream of White Boulevard	0.42	606	764	813	943
CONWAY SLOUGH TRIBUTARY 2					
At confluence with Conway Slough	1.01	584	833	932	1,159
Upstream of Pearson Road	0.76	495	709	790	980
Downstream of Lloyd Street	0.54	383	547	608	751
Downstream of Pine Park Drive	0.51	366	525	583	721
At Pine Circle Drive	0.32	262	374	413	507
EUTACUTACHEE CREEK					
At confluence of Pelahatchie Creek	27.93	4,128	6,232	7,063	9,254
At confluence with Eutacutachee Creek Tributary 4	19.56	3,984	5,996	6,755	8,809
Just upstream of confluence of Eutacutachee Creek Tributary 4	15.26	3,227	4,856	5,471	7,134
Just upstream of confluence of Eutacutachee Creek Tributary 3	6.84	1,709	2,563	2,883	3,751
At confluence of Eutacutachee Creek Tributary 2	5.03	1,477	2,210	2,480	3,218
Just upstream of confluence of Eutacutachee Creek Tributary 2	2.90	924	1,383	1,552	2,014
EUTACUTACHEE CREEK TRIBUTARY 1					
At confluence with Eutacutachee Creek	1.13	556	817	910	1,165
At U.S. Highway 80	0.24	249	354	388	486

Table 6: Summary of Discharges

Flooding Source and Location	Drainage Area (Square miles)	Peak Discharges (Cubic Feet per Second)			
		10-percent	2-percent	1-percent	0.2-percent
EUTACUTACHEE CREEK TRIBUTARY 2					
At confluence with Eutacutachee Creek	2.13	883	1,305	1,456	1,872
At Rankin Road	0.91	49	697	775	986
EUTACUTACHEE CREEK TRIBUTARY 3					
At confluence with Eutacutachee Creek	7.34	1,937	2,902	3,261	4,237
Approximately 1,500 feet downstream of Rankin Road	6.04	1,939	2,890	3,234	4,184
Approximately 1 mile upstream of Rankin Road	1.76	816	1,206	1,343	1,726
EUTACUTACHEE CREEK TRIBUTARY 4					
At confluence with Eutacutachee Creek	4.30	1,211	1,793	2,013	2,599
At Gulde-Shiloh Road	1.91	911	1,333	1,482	1,894
HOG CREEK					
Approximately 1 mile upstream of its confluence with Pearl River (which is the confluence of a small diversion channel)	12.05	2,377	3,638	4,239	5,204
Just upstream of confluence of small diversion channel	11.81	2,329	3,567	4,157	5,103
At State Highway 468/Flowood Drive	10.84	2,297	3,446	3,885	5,060
Approximately 2.9 miles upstream of Railroad	7.75	2,760	4,420	5,180	7,140
Approximately 3.5 miles upstream of Railroad	4.13	1,580	2,440	2,920	4,250
Approximately 4.6 miles upstream of Railroad	2.62	1,140	1,720	2,070	3,000
Just upstream of Luckney Road	1.42	690	1,020	1,220	1,800
HOG CREEK TRIBUTARY					
At confluence with Hog Creek	1.83	870	1,300	1,530	2,100
Approximately 2,000 feet upstream of confluence with Hog Creek	1.12	650	940	1,090	1,500

Table 6: Summary of Discharges

Flooding Source and Location	Drainage Area (Square miles)	Peak Discharges (Cubic Feet per Second)			
		10-percent	2-percent	1-percent	0.2-percent
INDIAN CREEK					
At confluence with Steen Creek	9.61	2,237	3,334	3,751	4,865
Just upstream of Confluence with Indian Creek	4.98	1,520	2,237	2,504	3,218
Tributary 1					
At Gunter Road	2.65	1,111	1,685	2,013	3,052
INDIAN CREEK TRIBUTARY 1					
At confluence with Indian Creek	4.20	2,021	2,595	2,794	3,287
At Cross Section I	3.16	1,820	2,322	2,493	2,920
At U.S. Route 49	1.01	526	769	908	1,330
MILL CREEK					
At Spillway Road	11.21	3,340	5,520	7,600	8,600
Approximately 2,150 feet downstream of State Highway 25	9.39	3,378	5,014	5,403	6,384
Approximately 1,185 feet upstream of Castlewoods Boulevard	6.05	2,959	3,818	4,108	4,845
At Cross-Section I	4.57	1,564	2,367	2,652	3,459
Approximately 300 feet downstream of Highway 471	2.42	1,100	1,628	1,813	2,330
MILL CREEK TRIBUTARY					
At confluence with Mill Creek	1.47	800	1,250	1,460	1,980
Approximately 3,100 feet above confluence with Mill Creek	1.22	770	1,180	1,400	1,920
Approximately 1.0 mile above confluence with Mill Creek	0.96	690	1,060	1,240	1,670
NEELY CREEK					
At State Highway 468	5.46	2,617	3,323	3,678	5,201
At confluence of Neely Creek (Right Channel)	5.46	2,617	3,323	3,678	5,201
At confluence of Neely Creek Tributary 2	1.71	902	1,316	1,529	2,223

Table 6: Summary of Discharges

Flooding Source and Location	Drainage Area (Square miles)	Peak Discharges (Cubic Feet per Second)			
		10-percent	2-percent	1-percent	0.2-percent
NEELY CREEK (LEFT CHANNEL)					
At confluence with Neely Creek and Neely Creek Tributary 2	1.71	902	1,316	1,529	2,223
At U.S. Highway 80	1.29	813	1,161	1,333	1,895
NEELY CREEK (RIGHT CHANNEL)					
Approximately 2,800 feet downstream of Flynn Drive	0.28	292	360	383	440
NEELY CREEK TRIBUTARY 2					
At confluence with Neely Creek	1.86	798	1,184	1,431	2,135
At City of Pearl downstream corporate limits	1.85	756	1,100	1,227	1,565
Upstream of confluence of Neely Creek	1.49	323	467	517	654
Upstream of confluence of Neely Creek Tributary 4	0.15	207	274	350	437
PARK CREEK					
At confluence with Pelahatchie Creek (Upper Reach)	2.31	*	*	1,320	*
PEARL RIVER					
At Old Brandon Road (Jackson gaging station)	3,171	56,800	90,000	106,000	148,000
PEARL RIVER TRIBUTARY 1					
At City of Richland Corporate Limits	1.69	1,118	1,584	1,833	2,594
At Cross Section B	1.44	971	1,373	1,582	2,232
At Cross Section C	0.89	635	892	1,016	1,421
At Old U.S. Route 49	0.67	494	692	782	1,089
At U.S. Route 49	0.26	214	296	327	447
PEARL RIVER TRIBUTARY 2					
At City of Richland corporate limits	0.83	400	579	702	1,025
At Old U.S. Route 49	0.55	400	595	670	814
At Railroad	0.62	305	440	533	776

Table 6: Summary of Discharges

Flooding Source and Location	Drainage Area (Square miles)	Peak Discharges (Cubic Feet per Second)			
		10-percent	2-percent	1-percent	0.2-percent
PEARL RIVER TRIBUTARY 3					
At confluence with Hog Creek	1.07	480	716	814	988
At State Route 468/Flowood Drive	0.78	374	560	635	766
PELAHATCHIE BAY UNNAMED TRIBUTARY					
At confluence with Pelahatchie Bay/Ross Barnett Reservoir	1.64	699	1,021	1,139	1,456
PELAHATCHIE CREEK (LOWER REACH)					
At State Route 471	205	17,900	29,600	36,000	51,400
Approximately 2.4 miles upstream of State Route 25	189	16,900	28,000	33,600	48,900
PELAHATCHIE CREEK (UPPER REACH)					
At confluence with Eutacutachee Creek	72.64	*	*	12,882	*
At Railroad	67.39	6,699	11,198	14,111	23,649
PELAHATCHIE CREEK TRIBUTARY					
At confluence with Pelahatchie Creek (Lower Reach)	1.6	710	1,130	1,350	1,900
At Holly Bush Road	1.14	620	970	1,160	1,500
PELAHATCHIE CREEK TRIBUTARY 1					
At confluence with Pelahatchie Creek (Upper Reach)	1.05	585	868	983	1,206
At Ragan Street	0.38	306	441	486	614
PIERCE CREEK					
At confluence with Pelahatchie Creek	4.85	1,409	2,210	2,679	4,201
At Heslip Street	4.69	1,390	2,177	2,634	4,122
At Railroad	4.30	1,343	2,093	2,522	3,926
At Cross Section K	3.50	1,238	1,908	2,276	3,498
At Lockwood Street	2.33	1,053	1,588	1,857	2,784

* Data Not Available

Table 6: Summary of Discharges

Flooding Source and Location	Drainage Area (Square miles)	Peak Discharges (Cubic Feet per Second)			
		10-percent	2-percent	1-percent	0.2-percent
PLUMMER SLOUGH					
At State Route 25	4.10	1,420	2,300	2,800	3,800
Approximately 0.8 mile upstream of State Route 25	3.00	1,220	1,940	2,380	3,400
Approximately 1.4 miles upstream of State Route 25	2.38	1,080	1,710	2,070	2,900
Just downstream of Oakdale Drive	1.57	850	1,340	1,600	2,200
Approximately 0.6 mile upstream of Oakdale Drive	0.80	540	850	1,000	1,400
PRAIRIE BRANCH CANAL					
At confluence with Pearl River	6.48	1,608	2,549	3,084	4,879
Approximately 1,380 feet upstream of Railroad	5.01	1,351	2,124	2,568	4,031
At divergence of Prairie Branch Canal	3.27	1,580	2,335	2,754	4,067
At Old Brandon Road	1.00	702	995	1,127	1,587
PRAIRIE BRANCH CANAL TRIBUTARY 1					
At Magnum Road	2.24	986	1,482	1,696	2,076
Just upstream of unnamed tributary	1.37	706	1,050	1,195	1,464
At Thompson Field, Jackson Airport Property	0.78	604	889	1,005	1,233

Table 6: Summary of Discharges

Flooding Source and Location	Drainage Area (Square miles)	Peak Discharges (Cubic Feet per Second)			
		10-percent	2-percent	1-percent	0.2-percent
RICHLAND CREEK					
At western corporate limits of the City of Richland	126.16	9,535	15,794	19,075	31,356
At U.S. Route 49	125.42	9,506	15,744	19,017	31,260
At eastern corporate limits of the City of Richland	124.32	9,462	15,670	18,930	31,117
At State Route 469	79.76	7,500	12,369	15,053	24,697
At State Highway 18	20.61	3,996	6,039	7,062	8,606
Approximately 500 feet downstream of Shiloh Road	16.22	3,477	5,276	6,155	7,511
At Interstate Route 20	5.67	1,852	2,821	3,252	4,010
At Railroad	2.64	1,175	1,784	2,040	2,507
At Cross Section AY	2.53	*	*	1,852	*
Approximately 1,000 feet downstream of Tiffany Drive	1.25	*	*	1,066	*
At Tiffany Drive	0.95	*	*	892	*
RICHLAND CREEK TRIBUTARY 1					
At confluence with Richland Creek	2.85	921	1,423	1,715	2,649
At Railroad	2.20	800	1,221	1,464	2,233
At southwest corporate limits of the City of Pearl	1.07	542	797	943	1,388
RICHLAND CREEK TRIBUTARY 2					
At confluence with Richland Creek	3.84	*	*	2,233	*
Approximately 2,200 feet upstream of Kennedy Farm Parkway	1.09	*	*	1,488	*
RICHLAND CREEK TRIBUTARY 3					
At confluence with Richland Creek	1.93	*	*	1,900	*

* Data Not Available

Table 6: Summary of Discharges

Flooding Source and Location	Drainage Area (Square miles)	Peak Discharges (Cubic Feet per Second)			
		10-percent	2-percent	1-percent	0.2-percent
SPRING BRANCH					
At mouth	2.80	980	1,560	1,900	2,900
Approximately 0.9 mile upstream of Fannin Landing Road	1.98	780	1,230	1,470	2,270
Approximately 1.2 miles upstream of Fannin Landing Road	1.30	540	830	1,000	1,550
Just upstream of Church Road	1.11	480	730	880	1,360
SQUIRREL BRANCH					
At U.S. Highway 49	1.93	906	1,337	1,548	2,272
At Lowe Circle	1.35	668	994	1,145	1,690
STEEN CREEK					
At downstream limit of study	38.11	5,239	8,636	10,477	17,169
Approximately 950 feet downstream of South Church Street	37.25	5177	7765	8799	11489
At Confluence of Indian Creek	25.18	3901	5850	6621	8640
Approximately 3,280 feet upstream of U.S. Highway 49	17.44	3130	4676	5277	6862
Approximately 12,363 feet upstream of U.S. Interstate 20	13.17	2548	3801	4288	5571
At Cross-Section O	11.41	2323	3434	3869	5000
TERRAPIN SKIN CREEK					
At confluence with Richland Creek	21.33	3,683	5,992	7,221	11,676
At State Route 468	18.90	3,483	5,653	6,821	11,010
At southern corporate limits of the City of Brandon	6.05	1,869	2,931	3,552	5,568
At confluence of Terrapin Skin Creek Tributary 2	5.77	1,814	2,838	3,436	5,375
At State Highway 471	2.64	1,105	1,672	1,989	3,007
At northern corporate limits of the City of Brandon	0.98	541	796	885	1,133

Table 6: Summary of Discharges

Flooding Source and Location	Drainage Area (Square miles)	Peak Discharges (Cubic Feet per Second)			
		10-percent	2-percent	1-percent	0.2-percent
TERRAPIN SKIN CREEK TRIBUTARY 1					
At confluence with Terrapin Skin Creek	1.60	635	958	1,167	1,770
At Cross Section D	0.59	174	254	286	406
At Cross Section F	0.13	97	132	151	206
TERRAPIN SKIN CREEK TRIBUTARY 2					
At confluence with Terrapin Skin Creek	0.83	367	542	652	968
At Cross Section H	0.46	278	396	463	659
TOWN BRANCH					
At confluence with Steen Creek	1.31	519	781	934	1,405
At White Street	1.11	475	708	842	1,256
At West Main Street	1.06	463	689	818	1,217
At Cross Section K	0.82	404	592	696	1,020
At City of Florence corporate limits	0.37	264	369	423	590
TURTLE CREEK					
At Spillway Road	1.48	800	1,270	1,520	2,130
Approximately 0.5 mile upstream of Spillway Road	1.10	600	880	1,050	1,550
WOODRUN CREEK					
At Confluence with Terrapin Skin Creek	2.92	*	*	2,569	*
Approximately 1,800 feet downstream of U.S. Interstate 20	1.79	*	*	1860	*
Approximately 900 feet downstream of U.S. Interstate 20	0.68	*	*	994	*
At Interstate Route 20	0.63	420	662	704	850
Approximately 2,000 feet upstream of Interstate Route 20	0.52	364	540	610	732
At Oak Ridge Drive	0.07	134	197	218	264
* Data Not Available					

The Stillwater elevations have been determined for the 10-, 2.0-, 1.0-, and 0.2-percent annual chance floods for the flooding sources studied by detailed methods and are summarized in Table 7, “Summary of Stillwater Elevations.”

TABLE 7. SUMMARY OF STILLWATER ELEVATIONS

<u>FLOODING SOURCE AND LOCATION</u>	<u>ELEVATION (Feet)</u>			
	<u>10-percent</u>	<u>2-percent</u>	<u>1-percent</u>	<u>0.2-percent</u>
OLD PEARL RIVER				
At U.S. Highway 80	259.4	261.7	262.5	266.2
ROSS BARNETT RESEVOIR				
From Ross Barnett Reservoir to State Route 43	*	*	299.8	*
Upstream of State Route 43	*	*	301.3	*

* Data Not Available

3.2 Hydraulic Analyses

Analyses of the hydraulic characteristics of flooding from the sources studied were carried out to provide estimates of the elevations of floods of the selected recurrence intervals. Users should be aware that flood elevations shown on the Flood Insurance Rate Map (FIRM) represent rounded whole-foot elevations and may not exactly reflect the elevations shown on the Flood Profiles or in the Floodway Data table in the FIS report. Flood elevations shown on the FIRM are primarily intended for flood insurance rating purposes. For construction and/or floodplain management purposes, users are cautioned to use the flood elevation data presented in this FIS report in conjunction with the data shown on the FIRM.

The hydraulic analyses for this countywide FIS were based on unobstructed flow. The flood elevations shown on the Flood Profiles (Exhibit 1) are thus considered valid only if hydraulic structures remain unobstructed, operate properly, and do not fail.

Qualifying bench marks within a given jurisdiction that are cataloged by the National Geodetic Survey (NGS) and entered into the National Spatial Reference System (NSRS) as First or Second Order Vertical and have a vertical stability classification of A, B, or C are shown and labeled on the FIRM with their 6-character NSRS Permanent Identifier.

Benchmarks cataloged by the NGS and entered into the NSRS vary widely in vertical stability classification. NSRS vertical stability classifications are as follows:

Stability A: Monuments of the most reliable nature, expected to hold position/elevation well (e.g., mounted in bedrock)

Stability B: Monuments which generally hold their position/elevation well (e.g., concrete bridge abutment)

Stability C: Monuments which may be affected by surface ground movements (e.g., concrete monuments below frost line)

Stability D: Mark of questionable or unknown vertical stability (e.g., concrete monument above frost line, or steel witness post)

In addition to NSRS bench marks, the FIRM may also show vertical control monument established by a local jurisdiction; these monuments will be shown on the FIRM with the appropriate designations. Local monuments will only be placed on the FIRM if the community has requested that they be included, and if the monuments meet the aforementioned NSRS inclusion criteria.

To obtain current elevation, description, and/or location information for benchmarks shown on the FIRM for this jurisdiction, please contact the Information Services Branch of the NGS at (301) 713-3242, or visit its website at <http://www.ngs.noaa.gov>.

Temporary vertical monuments are often established during the preparation of a flood hazard analysis for the purpose of establishing local vertical control. Although these monuments are not shown on the FIRM, they may be found in the Technical Support Data Notebook associated with the FIS report and FIRM. Interested individuals may contact FEMA to access this data.

3.2.1 Methods for Flooding Sources with New or Revised Analyses in Current Study

Flood profiles were drawn showing the computed water-surface elevations for floods of the selected recurrence intervals. Locations of selected cross sections used in the hydraulic analyses are shown on the Flood Profiles (Exhibit 1). For stream segments for which a floodway was computed (Section 4.2), selected cross section locations are also shown on the FIRM (Exhibit 2).

Cross section geometries were obtained from a combination of digital terrain data developed from LiDAR products, 5-foot topography developed from the 2006 statewide 2-foot resolution orthophotography, and field surveys. Bridges and culverts located within the detailed study limits were field surveyed to obtain elevation data and structural geometry.

Downstream boundary conditions for the hydraulic models were set to normal depth using a starting slope calculated from values taken from topographic data, or where applicable, derived from the water-surface elevations. Water-surface profiles were computed through the use of the U.S. Army Corps of Engineers (USACE) HEC-RAS version 4.1.0 computer program (USACE, 2010). The model was run for the 1.0-percent annual chance storm for the limited detailed studies and for the 0.2-, 1-, 2.0-, and 10-percent annual chance storm for the detailed studies.

Roughness coefficients (Manning's "n") were chosen by engineering judgment and based on field observation of the channel and floodplain areas Table 8, "Summary of Roughness Coefficients," contains the channel and overbank "n" values for all streams studied by detailed methods.

Table 8: Summary of Roughness Coefficients

Flooding Source	Channel	Overbanks
Brush Branch	0.05	0.15
Brush Creek	0.05	0.15
Butler Creek	0.05-0.06	0.08-0.15
Clark Creek	*	*
Clark Creek Tributary	0.013-0.070	0.055-0.023
Conway Slough	0.013-0.070	0.100-0.150
Conway Slough Tributary 1	0.05	0.15
Conway Slough Tributary 2	0.013-0.070	0.100-0.150
Eutacutachee Creek	0.03-0.055	0.04-0.085
Eutacutachee Creek Tributary 1	0.045	0.04-0.075
Eutacutachee Creek Tributary 2	0.045	0.055
Eutacutachee Creek Tributary 3	0.030-0.055	0.040-0.085
Eutacutachee Creek Tributary 4	0.030-0.045	0.040-0.095
Hog Creek	0.025-0.060	0.050-0.12
Hog Creek Tributary	0.028-0.060	0.055-0.020
Indian Creek	0.05	0.15-0.20
Indian Creek Tributary 1	0.05	0.12-0.15
Mill Creek	0.03-0.04	0.1
Mill Creek Tributary	0.013-0.07	0.055-0.023
Neely Creek	0.013-0.070	0.100-0.150
Neely Creek (Left Channel)	*	*
Neely Creek (Right Channel)	0.03-0.05	0.12-0.15
Neely Creek Tributary 2	0.013-0.060	0.110-0.150
Park Creek	0.05	0.15
Pearl River	0.013-0.065	0.055-0.020
Pearl River Tributary 1	0.030-0.045	0.050-0.06
Pearl River Tributary 2	0.035-0.045	0.055-0.14
Pearl River Tributary 3	0.030-0.045	0.050-0.06
Pelahatchie Bay Unnamed Tributary	0.045	0.060
Pelahatchie Creek (Lower Reach)	0.013-0.065	0.050-0.02
* Data Not Available		

Table 8: Summary of Roughness Coefficients

Flooding Source	Channel	Overbanks
Pelahatchie Creek (Upper Reach)	0.013-0.065	0.050-0.15
Pelahatchie Creek Tributary	*	*
Pelahatchie Creek Tributary 1	0.028-0.050	0.050-0.085
Pierce Creek	0.013-0.065	0.050-0.170
Plummer Slough	0.013-0.07	0.055-0.023
Prairie Branch Canal	0.013-0.070	0.100-0.150
Prairie Branch Canal Tributary 1	0.03-0.15	0.045-0.3
Richland Creek	0.025-0.045	0.050-0.080
Richland Creek Tributary 1	0.013-0.070	0.045-0.3
Richland Creek Tributary 2	0.05	0.15
Richland Creek Tributary 3	0.05	0.15
Spring Branch	0.013-0.07	0.055-0.023
Squirrel Branch	0.013-0.065	0.10-0.15
Steen Creek	0.05	0.12
Terrapin Skin Creek	0.05	0.09-0.15
Terrapin Skin Creek Tributary 1	0.013-0.045	0.12-0.15
Terrapin Skin Creek Tributary 2	0.013-0.045	0.12-0.15
Town Branch	0.013-0.07	0.08-0.150
Turtle Creek	0.013-0.07	0.055-0.023
Woodrun Creek	0.027-0.05	0.045-0.15
* Data Not Available		

3.2.2 Methods for Flooding Sources Incorporated from Previous Studies

Pre Countywide Analyses

Each jurisdiction within Rankin County, with the exception of the Township of Puckett, had a previously printed FIS report. The hydraulic analyses described in those reports have been compiled and are summarized below.

Cross-sections of stream channels were field surveyed along with bridge and culvert waterway openings following reconnaissance of the study areas by engineers. Cross-sections of stream overbanks and road profiles were obtained by photogrammetric methods using maps at a scale of 1:2,400, with a contour interval of 2 feet, developed specifically for this study with the following references: (U.S. Dept of Agriculture, 1975, for the City of Brandon; Michael Baker, Jr., Inc., 1978, for the Town of Florence; Michael Baker, Jr., Inc., 1979, for the Cities of Flowood, Pearl, Richland, and the unincorporated areas of Rankin County; and Michael Baker, Jr., Inc., 1980, for the Town of Pelahatchie.)

For the Cities of Brandon, Flowood, Pearl, Richland, the Towns of Florence and Pelahatchie; and the unincorporated areas of Rankin County, stream characteristics were determined by field observation, and flood profiles were computed using the HEC-2 step-backwater program computer model developed by the USACE (USACE, 1991). Starting water-surface elevations for all streams were developed by the slope/area method.

For approximate study areas in the Town of Florence, calculated peak discharges, stream characteristics based on field observations, and floodplain cross sections determined from maps developed for this study (Michael Baker, Jr., Inc., 1979) were used in Manning's equation to determine approximate flood elevations. For the approximately studied areas in the City of Flowood, calculated peak discharges were based on field observations. Cross sections were determined from 2-foot contour interval maps. For Pierce Creek, the approximate study area, in the Town of Pelahatchie, flood elevations were determined from the elevations computed in the detailed analysis immediately downstream.

In the City of Flowood, the interior drainage of the East Jackson and Fairground levees was evaluated using information from the USACE (USACE, April 22, 1963; USACE, July 1963; USACE, Design Memorandum No. 6...; and USACE, Design Memorandum No. 7..., July 1963)

The limits of flooding along Richland Creek in the vicinity of the Pearson Levee are revised due to the construction of the levee; however, water-surface elevations remain essentially the same.

For the Ross Barnett Reservoir, mathematical models developed in USACE studies were primarily developed for natural conditions with the reservoir operating under a constant pool elevation. The gated spillway at Ross Barnett Reservoir has a sufficient capacity to discharge up to 130,000 cubic feet per second at a minimum lake level elevation of 295 feet NGVD (Pearl River Valley Water Supply District, 1983).

The 1.0-percent annual chance discharge of the inflow hydrograph at Ross Barnett Reservoir is estimated at 106,000 cfs. Present operation of the reservoir utilizes any available storage to reduce downstream flooding, with the pool level at the dam reaching a peak elevation of 299.7 feet NAVD 88 during 1979. Information on the 1979 and 1983 inflow hydrographs were 500-year and 110-year, respectively. The flood frequencies at the Jackson gage 15 miles downstream were 200-year for the 1979 flood and 38-year for the 1983 flood. The elevations of the 1979 and 1983 floods were used to develop the 1-percent annual chance water surface profile upstream of Ross Barnett Dam. Under present conditions, the reservoir pool will not be allowed to exceed elevation 299.9 feet NAVD 88 at the emergency spillway.

The downstream portion of Pierce Creek flows parallel to Pelahatchie Creek just outside the corporate limits, south of the Town of Pelahatchie, and shares common cross-sections, although lettered separately. Flood elevations on Pierce Creek below Heslip Street are controlled by flooding from Pelahatchie Creek in this area.

The interior drainage of the East Jackson and Fairground levees was evaluated using information from the USACE (USACE, April 22, 1963; USACE, July 1963; USACE, Design Memorandum No. 6...; and USACE, Design Memorandum No. 7..., July 1963).

The ponding effects of the East Jackson Levee System on south connecting canal were determined by a reservoir routing analysis, using the Modified-Puls Method included in the HEC-1 computer program (USACE, 1990).

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The overbanks of the cross-section data used in the hydraulic analyses for all streams were obtained from recently developed topographic mapping with contour intervals of 4 feet. This mapping was completed specifically for this study. The detailed channel cross-sections, and structural survey data were further supported by field survey. In this revision, several of the study stream reaches are portions within existing detailed studies. In the case of an existing hydraulic model, the hydraulic model type was maintained. The following discussion indicates the hydraulic model used and other special cases applicable to the hydraulic analyses.

For Eutacutachee Creek, Eutacutachee Creek Tributaries 1-4, Pearl River Tributary 3, Pelahatchie Bay Unnamed Tributary, Pelahatchie Creek Tributary 1, Prairie Branch Canal Tributary 1, and Woodrun Creek, water-surface elevations of floods of the selected recurrence intervals were computed using HEC-RAS model (USACE, 1998). Starting water-surface elevations for these streams were developed by the slope/area method.

For Hog Creek, Pearl River Tributary 2, Richland Creek, and Terrapin Skin Creek, water-surface elevations of floods of the selected recurrence intervals were computed using the USACE HEC-2 model (USACE, 1991). Known water-surface elevations downstream of the restudied reaches were used as the starting water-surface elevations in the computations.

Roughness factors (Manning's "n") used in the hydraulic computations were chosen by engineering judgment and based on field observations of the streams and floodplain areas. See Table 8, "Summary of Roughness Coefficients" for more information.

3.3 Vertical Datum

All FIS reports and FIRMs are referenced to a specific vertical datum. The vertical datum provides a starting point against which flood, ground, and structure elevations can be referenced and compared. Until recently, the standard vertical datum in use for newly created or revised FIS reports and FIRMs was the National Geodetic Vertical Datum of 1929 (NGVD29). With the finalization of the North American Vertical Datum of 1988 (NAVD88), many FIS reports and FIRMs are being prepared using NAVD88 as the referenced vertical datum.

All flood elevations shown in this FIS report and on the FIRM are referenced to NAVD88. Structure and ground elevations in the community must, therefore, be referenced to NAVD88. It is important to note that adjacent communities may be referenced to NGVD29. This may result in differences in Base Flood Elevations (BFEs) across the corporate limits between the communities.

The elevations shown in the FIS report and on the FIRM for Rankin County are referenced to NAVD88. Ground, structure, and flood elevations may be compared and/or referenced to NGVD29 by applying a conversion factor. To convert elevations from NAVD88 to NGVD29, add 0.05 feet to the NAVD88 elevation. The 0.05 feet value is an average for the entire county. The BFEs shown on the FIRM represent whole-foot

rounded values. For example, a BFE of 12.4 feet will appear as 12 feet on the FIRM, and 12.6 feet as 13 feet. Users who wish to convert the elevations in this FIS report to NGVD29 should apply the stated conversion factor to elevations shown on the Flood Profiles and supporting data tables in the FIS report, which are shown at a minimum to the nearest 0.1 foot.

For more information regarding conversion between the NGVD and NAVD, see the FEMA publication entitled *Converting the National Flood Insurance Program to the North American Vertical Datum of 1988* (FEMA, June 1992), or contact the Vertical Network Branch, National Geodetic Survey, Coast and Geodetic Survey, National Oceanic and Atmospheric Administration, Rockville, Maryland 20910 (Internet address <http://www.ngs.noaa.gov>).

Temporary vertical monuments are often established during the preparation of a flood hazard analysis for the purpose of establishing local vertical control. Although these monuments are not shown on the FIRM, they may be found in the Technical Support Data Notebook associated with the FIS report and FIRM. Interested individuals may contact FEMA to access this data.

4.0 FLOODPLAIN MANAGEMENT APPLICATIONS

The NFIP encourages State and local governments to adopt sound floodplain management programs. Therefore, each FIS provides 1-percent annual chance flood elevations and delineation of the 1-percent annual chance floodplain boundary to assist communities in developing floodplain management measures. This information is presented on the FIRM and in many components of the FIS report, including Flood Profiles, Floodway Data Table and Summary of Stillwater Elevations Table. Users should reference the data presented in the FIS report as well as additional information that may be available at the local map repository before making flood elevation and/or floodplain boundary determinations.

4.1 Floodplain Boundaries

To provide a national standard without regional discrimination, the 1-percent annual chance flood has been adopted by FEMA as the base flood for floodplain management purposes. The 1-percent annual chance floodplain boundary is shown on the FIRM (Exhibit 2). On this map, the 1-percent annual chance floodplain boundary corresponds to the boundary of the areas of special flood hazards (Zones A and AE). Small areas within the floodplain boundaries may lie above the flood elevations but cannot be shown due to limitations of the map scale and/or lack of detailed topographic data.

For this study, a digital terrain model produced from LiDAR bare-earth points data developed in 2001 for the western portion of Rankin County (West Rankin Municipal Water and Sewer Authority, 2001) and 5 foot contours developed in 2010 from March 2006 digital orthophotography from the State of Mississippi were used to delineate the floodplain boundaries for the detailed and approximate studies (State of Mississippi, 2010).

4.2 Floodways

Encroachment on floodplains, such as structures and fill, reduces flood-carrying capacity, increases flood heights and velocities, and increases flood hazards in areas beyond the

encroachment itself. One aspect of floodplain management involves balancing the economic gain from floodplain development against the resulting increase in flood hazard. For purposes of the NFIP, a floodway is used as a tool to assist local communities in this aspect of floodplain management. Under this concept, the area of the 1-percent annual chance floodplain is divided into a floodway and a floodway fringe. The floodway is the channel of a stream, plus any adjacent floodplain areas, that must be kept free of encroachment so that the 1-percent annual chance flood can be carried without substantial increases in flood heights. Minimum Federal standards limit such increases to 1.0 foot, provided that hazardous velocities are not produced.

The results of the floodway computations have been tabulated for selected cross sections of detailed study streams (Table 9). In cases where the floodway and 1-percent annual chance floodplain boundaries are either close together or collinear, only the floodway boundary is shown.

Near the mouth of streams studied in detail, floodway computations are made without regard to flood elevations on the receiving water body. Therefore, "Without floodway" elevations presented in Table 9, "Floodway Data," for certain downstream cross sections are lower than regulatory flood elevations in that area, which must take into account the 1-percent annual chance flooding due to backwater from other sources.

Encroachment into areas subject to inundation by floodwaters having hazardous velocities aggravates the risk of flood damage, and heightens potential flood hazards by further increasing velocities. In order to reduce the risk of property damage in areas where the stream velocities are high, the county may wish to restrict development in areas outside the floodway.

The area between the floodway and 1-percent annual chance floodplain boundaries is termed the floodway fringe. The floodway fringe encompasses the portion of the floodplain that could be completely obstructed without increasing the water-surface elevation of the 1-percent annual chance flood more than 1.0 foot at any point. Typical relationships between the floodway and the floodway fringe and their significance to floodplain development are shown in Figure 1--Floodway Schematic.

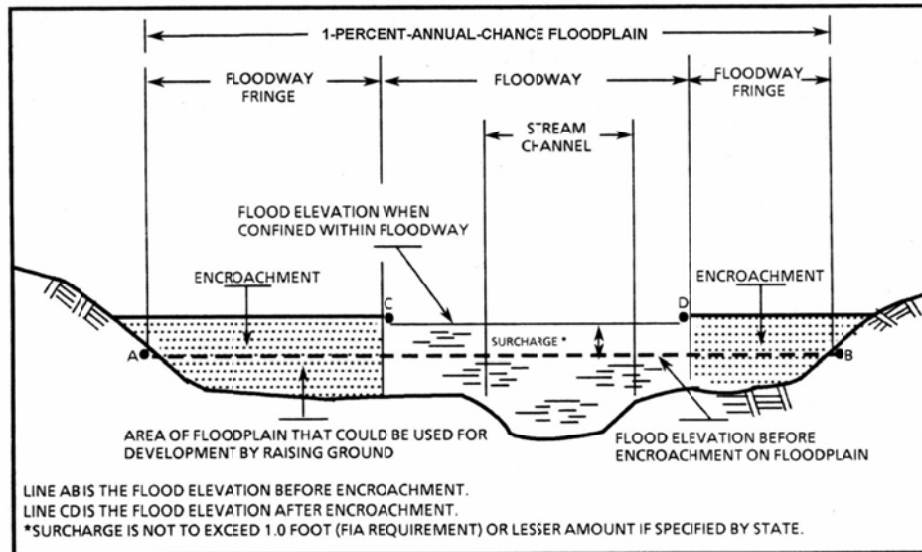


Figure 1 -- Floodway Schematic

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
BUTLER CREEK								
A	658	250	1,898	3.6	296.2	292.7 ²	292.7	0.0
B	1,617	243	1,292	4.8	298.7	296.4 ²	296.4	0.0
C	2,188	300	2,528	3.4	298.7	296.8 ²	297.0	0.2
D	3,679	825	5,064	2.1	298.7	297.4 ²	298.3	0.9
E	4,767	400	2,341	3.7	298.7	297.6 ²	298.5	0.9
F	5,559	446	1,439	6.1	299.6	299.6	300.1	0.5
G	6,284	707	2,719	2.9	302.0	302.0	302.9	0.9
H	6,952	535	2,319	1.9	302.4	302.4	303.3	0.9
I	8,391	375	1,251	4.5	303.2	303.2	304.2	1.0
J	9,915	174	740	5.0	308.9	308.9	309.5	0.6
K	12,758	319	1,829	1.5	312.2	312.2	313.1	0.9
L	17,094	258	1,432	1.9	315.1	315.1	316.1	1.0
M	20,647	152	720	3.2	319.2	319.2	319.8	0.6
N	24,451	110	504	4.2	328.1	328.1	328.8	0.7

¹ STREAM DISTANCE IN FEET ABOVE CONFLUENCE WITH STEEN CREEK

² ELEVATION COMPUTED WITHOUT CONSIDERATION OF BACKWATER EFFECTS FROM STEEN CREEK

TABLE 9

FEDERAL EMERGENCY MANAGEMENT AGENCY

**RANKIN COUNTY, MS
AND INCORPORATED AREAS**

FLOODWAY DATA

BUTLER CREEK

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
CLARK CREEK								
A	450 ¹	3,799 ³	3,919	1.4	309.3	303.5 ⁴	304.5	1.0
B	5,950 ¹	988	4,862	1.1	311.4	306.8 ⁴	307.8	1.0
C	10,800 ¹	982	4,821	1.1	312.7	309.5 ⁴	310.5	1.0
D	15,995 ¹	850	4,346	1.2	314.0	312.6 ⁴	313.6	1.0
E	17,790 ¹	525	2,660	1.5	315.0	315.0	315.2	0.2
F	20,490 ¹	567	2,102	1.9	316.4	316.4	317.4	1.0
G	25,650 ¹	483	2,933	1.3	324.7	324.7	325.5	0.8
H	31,400 ¹	623	3,599	1.0	326.6	326.6	327.6	1.0
I	36,150 ¹	500	1,320	2.2	332.4	332.4	332.8	0.4
J	40,100 ¹	500	1,807	1.6	335.2	335.2	336.2	1.0
K	42,100 ¹	400	729	3.8	339.0	339.0	339.0	0.0
CLARK CREEK TRIBUTARY								
A	1,050 ²	320	889	1.8	314.0	310.5 ⁵	311.4	0.9
B	2,120 ²	320	1,503	1.1	314.8	314.8	315.6	0.8
C	5,800 ²	187	439	2.7	320.5	320.5	320.5	0.0
D	9,880 ²	267	1,199	1.0	327.6	327.6	328.6	1.0
E	13,500 ²	111	461	2.2	332.3	332.3	333.0	0.7

¹ FEET ABOVE CONFLUENCE WITH PELAHATCHIE CREEK (LOWER REACH)

² FEET ABOVE CONFLUENCE WITH CLARK CREEK

³ COMBINED FLOODWAY WIDTH OF PELAHATCHIE CREEK (LOWER REACH) AND CLARK CREEK

⁴ ELEVATION COMPUTED WITHOUT CONSIDERATION OF OVERFLOW EFFECTS FROM PELAHATCHIE CREEK (LOWER REACH)

⁵ ELEVATION COMPUTED WITHOUT CONSIDERATION OF BACKWATER EFFECTS FROM CLARK CREEK

TABLE 9

FEDERAL EMERGENCY MANAGEMENT AGENCY

**RANKIN COUNTY, MS
AND INCORPORATED AREAS**

FLOODWAY DATA

**CLARK CREEK – CLARK CREEK
TRIBUTARY**

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
CONWAY SLOUGH								
A	3,564 ¹	70	461	10.7	262.5	254.4 ³	254.4	0.0
B	5,669 ¹	140	1,003	5.1	262.5	261.1 ³	262.0	0.9
C	5,804 ¹	95	732	6.7	262.5	261.5 ³	262.3	0.8
D	6,762 ¹	182	1,324	5.0	263.6	263.6	264.1	0.5
E	8,147 ¹	156	979	3.7	264.1	264.1	265.1	1.0
F	9,270 ¹	236	1,502	2.2	266.3	266.3	267.0	0.7
G	12,310 ¹	199	1,647	1.5	269.0	269.0	270.0	1.0
CONWAY SLOUGH TRIBUTARY 1								
A	954 ²	60	259	3.3	269.0	265.1 ⁴	265.1	0.0
B	2,899 ²	400	1,713	2.2	270.0	270.0	270.7	0.7
C	5,488 ²	273	908	0.3	273.4	273.4	274.2	0.8
D	6,600 ²	75	149	1.6	274.5	274.5	274.9	0.4
E	7,707 ²	50	151	7.2	282.5	282.5	283.1	0.6
F	8,893 ²	31	140	5.8	291.2	291.2	291.5	0.3

¹ FEET ABOVE CONFLUENCE WITH PEARL RIVER

² FEET ABOVE CONFLUENCE WITH CONWAY SLOUGH

³ ELEVATION COMPUTED WITHOUT CONSIDERATION OF BACKWATER EFFECTS FROM PEARL RIVER

⁴ ELEVATION COMPUTED WITHOUT CONSIDERATION OF BACKWATER EFFECTS FROM CONWAY SLOUGH

TABLE 9

FEDERAL EMERGENCY MANAGEMENT AGENCY

**RANKIN COUNTY, MS
AND INCORPORATED AREAS**

FLOODWAY DATA

**CONWAY SLOUGH – CONWAY SLOUGH
TRIBUTARY 1**

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
CONWAY SLOUGH TRIBUTARY 2								
A	1,230	27	89	10.5	269.6	266.2 ³	266.2	0.0
B	1,965	26	106	8.8	269.6	267.9 ³	267.9	0.0
C	2,610	61	236	4.0	269.6	269.2 ³	269.2	0.0
D	3,940	18	59	10.2	271.5	271.5	271.5	0.0
E	4,650	350	483	1.3	275.1	275.1	275.7	0.6
F	4,982	600 ²	412	1.5	276.4	276.4	277.3	0.9
G	5,302	600 ²	362	1.6	276.8	276.8	277.7	0.9
H	5,653	420 ²	562	1.0	277.4	277.4	278.2	0.8
I	5,950	300	447	1.3	278.5	278.5	279.1	0.6
J	6,210	200	312	1.9	281.3	281.3	282.1	0.8
K	6,850	21	48	8.7	284.4	284.4	284.4	0.0

¹ FEET ABOVE CONFLUENCE WITH CONWAY SLOUGH

² WIDTH INCLUDES HIGH GROUND NOT INCLUDED IN FLOODWAY CONVEYANCE CALCULATIONS

³ ELEVATION COMPUTED WITHOUT CONSIDERATION OF BACKWATER EFFECTS FROM CONWAY SLOUGH

TABLE 9

FEDERAL EMERGENCY MANAGEMENT AGENCY

**RANKIN COUNTY, MS
AND INCORPORATED AREAS**

FLOODWAY DATA

CONWAY SLOUGH TRIBUTARY 2

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
EUTACUTACHEE CREEK								
A	4,590	4,970	1,837	3.9	337.7	337.7	338.4	0.7
B	9,453	1,265	7,193	1.0	339.3	339.3	340.3	1.0
C	12,181	1,500	3,489	2.0	339.9	339.9	340.8	0.9
D	14,886	807	2,695	2.6	343.8	343.8	344.1	0.3
E	17,155	118	830	8.5	344.9	344.9	345.9	1.0
F	17,290	560	6,047	1.2	347.5	347.5	347.8	0.3
G	19,355	1,000	6,784	1.0	347.6	347.6	348.0	0.4
H	22,173	1,794	7,390	0.9	347.7	347.7	348.3	0.6
I	22,289	1,150	11,612	0.6	349.9	349.9	349.9	0.0
J	26,679	950	3,267	2.0	349.9	349.9	350.1	0.2
K	29,989	984	3,652	1.9	351.9	351.9	352.9	1.0
L	30,782	900	3,199	2.1	352.5	352.5	353.5	1.0
M	31,291	900	3,250	2.1	352.9	352.9	353.8	0.9
N	31,631	1,070	6,160	1.1	354.2	354.2	354.7	0.5
O	33,566	990	3,693	1.8	354.5	354.5	355.0	0.5
P	36,084	621	4,180	1.3	355.1	355.1	355.9	0.8
Q	38,207	112	484	11.3	356.8	356.8	356.8	0.0
R	38,567	680	4,195	1.3	360.2	360.2	360.2	0.0
S	40,832	410	4,426	0.7	360.2	360.2	360.3	0.1
T	43,184	254	2,664	1.1	360.3	360.3	360.3	0.0
U	47,119	107	543	4.6	368.7	368.7	369.4	0.7
V	47,564	975	2,783	0.9	371.3	371.3	371.7	0.4
W	48,881	175	925	1.7	371.8	371.8	372.3	0.5
X	50,404	64	264	5.9	372.8	372.8	373.7	0.9
Y	50,596	270	1,201	1.3	380.0	380.0	380.0	0.0
Z	51,848	400	894	1.7	380.2	380.2	380.9	0.7

¹ FEET ABOVE CONFLUENCE WITH PELAHATCHIE CREEK

² COMBINED

TABLE 9

FEDERAL EMERGENCY MANAGEMENT AGENCY

**RANKIN COUNTY, MS
AND INCORPORATED AREAS**

FLOODWAY DATA

EUTACUTACHEE CREEK

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
EUTACUTACHEE CREEK (Continued)								
AA	52,776 ¹	370	1,644	0.9	380.7	380.7	381.5	0.8
AB	53,084 ¹	385	1,633	1.0	384.1	384.1	384.2	0.1
AC	54,382 ¹	263	667	2.3	384.3	384.3	384.5	0.2
AD	54,563 ¹	360	1,948	0.8	387.1	387.1	388.1	1.0
AE	55,314 ¹	33	135	11.5	387.0	387.0	387.0	0.0
EUTACUTACHEE CREEK TRIBUTARY 1								
A	2,828 ²	250	354	2.6	366.2	366.2	366.9	0.7
B	5,062 ²	291	589	1.6	374.4	374.4	375.2	0.8
C	6,504 ²	26	50	7.8	381.1	381.1	381.4	0.3
D	7,070 ²	60	210	1.9	382.8	382.8	383.6	0.8
EUTACUTACHEE CREEK TRIBUTARY 2								
A	2,073 ²	261	587	2.5	373.5	373.5	374.5	1.0
B	3,072 ²	157	309	2.5	376.0	376.0	376.9	0.9
C	4,763 ²	177	251	3.1	380.3	380.3	381.3	1.0

¹ FEET ABOVE CONFLUENCE WITH PELAHATCHIE CREEK

² FEET ABOVE CONFLUENCE WITH EUTACUTACHEE CREEK

TABLE 9

FEDERAL EMERGENCY MANAGEMENT AGENCY

**RANKIN COUNTY, MS
AND INCORPORATED AREAS**

FLOODWAY DATA

**EUTACUTACHEE CREEK – EUTACUTACHEE
CREEK TRIBUTARY 1 - EUTACUTACHEE
CREEK TRIBUTARY 2**

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
EUTACUTACHEE CREEK TRIBUTARY 3								
A	2,175	189	749	4.4	360.1	357.1 ²	357.3	0.1
B	3,027	98	855	3.8	364.7	364.7	364.7	0.0
C	3,194	40	540	6.8	364.7	364.7	364.7	0.0
D	3,260	40	587	6.4	365.7	365.7	365.7	0.0
E	6,313	635	1,646	2.0	366.7	366.7	367.7	1.0
F	7,576	418	924	3.5	370.4	370.4	370.8	0.4
G	9,226	345	1,161	2.8	372.9	372.9	373.8	0.9
H	10,447	32	212	6.3	374.9	374.9	375.1	0.2
I	10,534	34	197	6.8	375.5	375.5	375.8	0.3
J	12,103	408	461	2.9	381.2	381.2	381.7	0.5
K	13,972	187	601	2.2	387.4	387.4	388.3	0.9
EUTACUTACHEE CREEK TRIBUTARY 4								
A	2,150	436	886	2.3	355.6	355.6	355.8	0.2
B	2,388	178	877	2.3	360.9	360.9	360.9	0.0
C	2,578	440	3,068	0.7	363.0	363.0	363.0	0.0
D	4,084	200	713	2.8	363.0	363.0	363.0	0.0
E	5,307	305	1,141	1.8	363.2	363.2	364.1	0.9
F	6,458	239	1,649	1.9	364.2	364.2	365.0	0.8
G	9,998	200	345	4.3	367.8	367.8	368.7	0.9
H	11,508	600	1,529	1.0	372.1	372.1	372.8	0.7
I	14,203	280	541	2.7	377.2	377.2	378.2	1.0
J	15,628	125	456	3.3	384.1	384.1	384.1	0.0
K	16,825	82	256	5.8	388.4	388.4	389.0	0.6
L	16,943	245	1,139	1.3	390.3	390.3	390.3	0.0

¹ FEET ABOVE CONFLUENCE WITH EUTACUTACHEE CREEK

² ELEVATION COMPUTED WITHOUT CONSIDERATION OF BACKWATER EFFECTS FROM EUTACUTACHEE CREEK

TABLE 9

FEDERAL EMERGENCY MANAGEMENT AGENCY

**RANKIN COUNTY, MS
AND INCORPORATED AREAS**

FLOODWAY DATA

**EUTACUTACHEE CREEK TRIBUTARY 3 –
EUTACUTACHEE CREEK TRIBUTARY 4**

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
HOG CREEK								
A	8,770 ¹	1,992	13,606	0.3	281.9	278.8 ³	279.8	1.0
B	11,234 ¹	151	1,089	3.8	281.9	278.8 ³	279.8	1.0
C	11,406 ¹	152	1,105	3.8	281.9	278.9 ³	279.9	1.0
D	12,743 ¹	174	1,389	3.0	281.9	280.1 ³	280.7	0.6
E	13,131 ¹	432	4,172	1.0	281.9	280.5 ³	281.0	0.5
F	14,740 ¹	348	1,611	2.6	281.9	280.5 ³	281.1	0.6
G	15,846 ¹	93	736	5.3	281.9	281.3 ³	282.1	0.8
H	15,981 ¹	93	754	5.1	281.9	281.5 ³	282.2	0.7
I	17,350 ¹	196	1,086	5.0	286.1	286.1	287.1	1.0
J	20,700 ¹	387	1,913	2.9	291.6	291.6	292.6	1.0
K	25,900 ¹	393	1,834	3.0	298.5	298.5	299.5	1.0
L	29,900 ¹	510	2,425	2.4	303.0	303.0	304.0	1.0
M	32,600 ¹	200	1,281	4.0	307.4	307.4	308.4	1.0
N	35,900 ¹	406	1,914	1.5	312.2	312.2	313.2	1.0
O	38,500 ¹	589	4,662	0.6	312.6	312.6	313.6	1.0
P	41,800 ¹	313	1,277	1.6	316.6	316.6	317.6	1.0
Q	43,900 ¹	100	680	1.8	326.5	326.5	327.5	1.0
R	45,900 ¹	100	275	4.4	336.3	336.3	337.3	1.0
S	47,650 ¹	170	682	1.8	341.0	341.0	342.0	1.0
HOG CREEK TRIBUTARY								
A	500 ²	41	302	5.1	311.7	309.2 ⁴	310.2	1.0
B	2,350 ²	40	271	4.0	315.6	315.6	316.3	0.7
C	4,100 ²	40	258	4.2	319.8	319.8	320.8	1.0
D	6,300 ²	61	245	4.4	330.8	330.8	330.8	0.0

¹ FEET ABOVE CONFLUENCE WITH PEARL RIVER

² FEET ABOVE CONFLUENCE WITH HOG CREEK

³ ELEVATION COMPUTED WITHOUT CONSIDERATION OF BACKWATER EFFECTS FROM PEARL RIVER

⁴ ELEVATION COMPUTED WITHOUT CONSIDERATION OF BACKWATER EFFECTS FROM HOG CREEK

TABLE 9	FEDERAL EMERGENCY MANAGEMENT AGENCY	FLOODWAY DATA
	RANKIN COUNTY, MS AND INCORPORATED AREAS	
		HOG CREEK – HOG CREEK TRIBUTARY

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
INDIAN CREEK								
A	1,923 ¹	285	2,289	2.8	294.9	293.5 ³	294.2	0.7
B	2,789 ¹	130	980	4.4	295.4	295.4	295.9	0.5
C	3,601 ¹	100	912	4.7	296.2	296.2	296.8	0.6
D	4,093 ¹	201	1,226	3.6	298.5	298.5	299.0	0.5
E	5,298 ¹	150	1,410	2.4	298.8	298.8	299.6	0.8
F	6,351 ¹	124	627	5.2	299.4	299.4	300.2	0.8
G	7,262 ¹	138	576	4.6	301.7	301.7	302.1	0.4
H	8,305 ¹	267	1,125	5.5	303.8	303.8	304.4	0.6
I	9,447 ¹	583	2,272	1.2	306.8	306.8	307.8	1.0
J	14,170 ¹	412	1,879	1.5	315.7	315.7	316.4	0.7
K	17,648 ¹	190	675	3.0	320.5	320.5	321.2	0.7
L	20,189 ¹	213	784	2.4	327.8	327.8	328.4	0.6
INDIAN CREEK TRIBUTARY 1								
A	1,548 ²	82	535	5.6	299.8	299.8	299.8	0.0
B	2,477 ²	80	914	3.1	301.8	301.8	301.9	0.1
C	3,523 ²	150	682	6.5	303.8	303.8	304.2	0.4
D	4,340 ²	425	1345	3.7	305.0	305.0	305.9	0.9
E	5,431 ²	240	1160	3.2	306.0	306.0	306.7	0.7
F	6,291 ²	210	624	7.9	306.4	306.4	307.3	0.9
G	7,274 ²	375	1286	3.4	312.3	312.3	312.9	0.6
H	8,731 ²	175	1093	2.7	317.0	317.0	317.5	0.5
I	10,066 ²	400	2473	2.1	317.4	317.4	318.0	0.6
J	14,620 ²	219	599	3.1	321.2	321.2	321.9	0.7
K	17,738 ²	131	364	3.1	328.3	328.3	328.8	0.5
L	21,473 ²	86	261	4.3	335.0	335.0	335.6	0.6
M	22,159 ²	261	821	1.1	339.1	339.1	340.0	0.9
N	23,250 ²	46	205	4.4	341.0	341.0	341.8	0.8

¹ FEET ABOVE CONFLUENCE WITH STEEN CREEK

² FEET ABOVE CONFLUENCE WITH INDIAN CREEK

³ ELEVATION COMPUTED WITHOUT CONSIDERATION OF BACKWATER
EFFECTS FROM STEEN CREEK

9	TABLE	FEDERAL EMERGENCY MANAGEMENT AGENCY RANKIN COUNTY, MS AND INCORPORATED AREAS	FLOODWAY DATA
			INDIAN CREEK – INDIAN CREEK TRIBUTARY 1

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
MILL CREEK								
A	300 ¹	353	1922	4.0	302.4	302.4	303.4	1.0
B	2,700 ¹	199	1672	4.6	305.2	305.2	306.2	1.0
C	4,857 ¹	101	1106	6.9	310.8	310.8	311.5	0.7
D	6,400 ¹	40	565	10.0	315.9	315.9	315.9	0.0
E	8,345 ¹	400	2209	3.7	317.4	317.4	317.7	0.3
F	10,627 ¹	150	838	6.7	321.2	321.2	321.2	0.0
G	12,600 ¹	298	1680	4.2	325.0	325.0	325.5	0.5
H	14,279 ¹	364	2499	2.2	328.1	328.1	328.6	0.5
I	18,234 ¹	145	517	6.2	332.8	332.8	333.1	0.3
J	21,954 ¹	385	1454	3.3	340.0	340.0	340.2	0.2
K	24,523 ¹	431	1229	2.5	344.5	344.5	344.8	0.3
L	26,691 ¹	51	269	7.7	350.3	350.3	351.1	0.8
MILL CREEK TRIBUTARY								
A	1,150 ²	162	644	2.3	316.1	315.6 ⁴	316.4	0.8
B	3,467 ²	156	738	1.9	322.3	322.3	323.2	0.9
C	5,837 ²	108	605	2.0	327.5	327.5	328.5	1.0
NEELY CREEK								
A	2,324 ¹	99	703	5.2	264.2	264.2	264.5	0.3
B	4,572 ¹	152	1519	2.4	270.5	270.5	271.1	0.6
C	5,514 ¹	974	3710	0.8	270.9	270.9	271.5	0.6
D	6,837 ¹	1,213	4873	0.6	271.2	271.2	271.9	0.7

¹ FEET ABOVE MOUTH

² FEET ABOVE CONFLUENCE WITH MILL CREEK

³ COMBINED FLOODWAY WIDTH OF NEELY CREEK AND NEELY CREEK TRIBUTARY

⁴ ELEVATION COMPUTED WITHOUT CONSIDERATION OF BACKWATER EFFECTS FROM MILL CREEK

9 TABLE	FEDERAL EMERGENCY MANAGEMENT AGENCY		FLOODWAY DATA	
	RANKIN COUNTY, MS AND INCORPORATED AREAS		MILL CREEK – MILL CREEK TRIBUTARY - NEELY CREEK	

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
NEELY CREEK (LEFT CHANNEL)								
A	2,550	1,300	2,105	1.7	271.3	268.4 ³	269.2	0.8
B	4,050	79	354	4.3	271.3	269.1 ³	269.6	0.5
C	4,750	67	396	4.1	271.3	269.9 ³	270.2	0.3
D	5,040	53	274	5.6	271.3	270.5 ³	270.8	0.3
E	5,780	51	295	5.2	271.9	271.9	272.1	0.2
F	6,530	56	324	4.7	272.9	272.9	273.0	0.1
G	6,915	41	263	5.8	273.2	273.2	273.3	0.1
H	7,997	850	684	2.2	275.8	275.8	276.4	0.6
I	9,093	482 ²	411	4.0	278.6	278.6	279.3	0.7
J	9,617	41	162	4.2	283.3	283.3	283.7	0.4
K	10,067	22	69	10.0	287.0	287.0	287.0	0.0
L	10,567	28	109	6.3	292.3	292.3	292.3	0.1
M	11,169	30	137	5.0	295.4	295.4	295.5	0.1
N	11,567	28	108	6.4	297.2	297.2	297.3	0.1
O	12,095	25	134	5.1	301.9	301.9	301.9	0.0
P	12,567	25	77	9.0	304.1	304.1	304.1	0.0
Q	13,067	37	120	5.7	310.3	310.3	310.3	0.0
R	13,632	28	104	6.6	315.1	315.1	315.1	0.0
S	14,067	34	145	4.7	317.8	317.8	318.6	0.8
T	14,542	36	124	5.6	320.1	320.1	320.3	0.2
U	15,067	30	105	6.6	323.1	323.1	323.1	0.0
V	15,567	31	92	7.4	327.1	327.1	327.1	0.0
W	16,067	26	72	9.5	333.7	333.7	333.7	0.0
X	16,471	23	96	7.1	339.6	339.6	339.6	0.0

¹ FEET ABOVE CONFLUENCE WITH NEELY CREEK

² WIDTH INCLUDES HIGH GROUND NOT INCLUDED IN FLOODWAY CONVEYANCE CALCULATION

³ ELEVATION COMPUTED WITHOUT CONSIDERATION OF BACKWATER EFFECTS FROM NEELY CREEK

TABLE 9

FEDERAL EMERGENCY MANAGEMENT AGENCY

**RANKIN COUNTY, MS
AND INCORPORATED AREAS**

FLOODWAY DATA

NEELY CREEK (LEFT CHANNEL)

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
NEELY CREEK (RIGHT CHANNEL)								
A	1,630	53	318	2.5	270.6	269.5 ²	270.4	0.9
B	3,627	794	1,517	0.5	271.2	270.4 ²	271.2	0.8
C	4,473	914	1,030	0.8	271.3	270.7 ²	271.6	0.9
D	6,013	300	713	1.0	271.7	270.8 ²	271.8	1.0
E	7,158	516	1,505	0.5	272.1	272.1	273.1	1.0
F	11,140	484	400	2.2	275.7	275.7	276.6	0.9
G	12,934	555	1162	1.1	277.7	277.7	278.2	0.5
H	14,419	470	762	1.7	279.1	279.1	279.7	0.6
I	15,810	248	533	1.8	281.5	281.5	282.3	0.8
J	16,795	36	97	4.0	282.2	282.2	282.8	0.6
K	17,456	32	95	4.0	284.1	284.1	284.2	0.1
NEELY CREEK TRIBUTARY 2								
A	1,358	3,120 ³	10,776	0.3	271.3	271.3 ²	272.2	0.9
B	2,528	51	225	5.4	271.3	269.1 ²	269.5	0.4
C	4,004	67	334	3.7	271.3	271.1 ²	271.3	0.2
D	4,694	85	453	2.7	271.4	271.4	271.6	0.2
E	7,249	37	123	4.2	272.2	272.2	272.3	0.1
F	8,109	23	78	6.7	273.5	273.5	273.5	0.0
G	9,109	34	99	3.5	275.7	275.7	275.7	0.0
H	10,269	35	62	5.6	279.7	279.7	279.7	0.0
I	10,514	35	108	3.3	281.4	281.4	282.1	0.7
J	11,075	96	77	4.6	282.9	282.9	282.9	0.0

¹ FEET ABOVE CONFLUENCE WITH NEELY CREEK

² ELEVATION COMPUTED WITHOUT CONSIDERATION OF BACKWATER EFFECTS FROM NEELY CREEK

³ THIS WIDTH EXTENDS BEYOND COUNTY BOUNDARY

TABLE 9

FEDERAL EMERGENCY MANAGEMENT AGENCY

**RANKIN COUNTY, MS
AND INCORPORATED AREAS**

FLOODWAY DATA

**NEELY CREEK (RIGHT CHANNEL) – NEELY
CREEK TRIBUTARY 2**

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
PEARL RIVER								
A	254.82	6,566 ²	63,412	1.7	251.2	251.2	252.1	0.9
B	257.05	8,164 ²	80,999	1.3	253.4	253.4	254.3	0.9
C	259.26	6,754 ²	75,272	1.4	255.3	255.3	256.0	0.7
D	262.26	6,251 ²	79,341	1.3	257.3	257.3	258.1	0.8
E	264.45	6,282 ²	79,037	1.3	258.8	258.8	259.7	0.9
F	266.22	10,780 ²	122,046	0.9	260.2	260.2	261.1	0.9
G	268.86	9,067 ²	98,436	1.1	261.6	261.6	262.6	1.0
H	270.53	1,581 ²	24,136	4.4	263.1	263.1	264.0	0.9
I	273.33	7,156 ²	78,706	1.3	265.5	265.5	266.3	0.8
J	277.13	9,492 ²	120,010	0.9	267.3	267.3	268.3	1.0
K	278.99	9,848 ²	84,375	1.3	268.2	268.2	269.2	1.0
L	282.10	8,948 ²	93,079	1.1	270.4	270.4	271.2	0.8
M	284.51	6,950 ²	100,969	1.0	271.7	271.7	272.5	0.8
N	285.10	4,214 ²	41,309	2.6	271.8	271.8	272.5	0.7
O	285.35	2,568 ²	27,505	3.9	272.5	272.5	273.4	0.9
P	286.25	3,200 ²	68,685	1.5	273.7	273.7	274.5	0.8
Q	286.64	2,087 ²	43,697	2.4	273.9	273.9	274.8	0.9
R	286.97	1,840 ²	38,293	2.8	274.2	274.2	275.1	0.9
S	287.37	2,043 ²	38,477	2.8	274.8	274.8	275.8	1.0
T	288.99	2,006 ²	39,350	2.7	276.5	276.5	277.3	0.8
U	289.46	3,349 ²	52,495	2.0	276.9	276.9	277.6	0.7
V	290.08	3,918 ²	58,446	1.8	277.3	277.3	277.9	0.6
W	290.46	3,513 ²	54,311	2.0	277.6	277.6	278.2	0.6
X	291.81	5,800 ²	90,446	1.2	279.9	279.9	280.7	0.8
Y	292.34	2,980 ²	50,167	2.1	280.4	280.4	281.0	0.6

¹ MILES ABOVE MOUTH

² THIS WIDTH EXTENDS BEYOND THE COUNTY BOUNDARY

TABLE 9

FEDERAL EMERGENCY MANAGEMENT AGENCY

**RANKIN COUNTY, MS
AND INCORPORATED AREAS**

FLOODWAY DATA

PEARL RIVER

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
PEARL RIVER (CONTINUED)								
Z	292.50	2,234 ²	33,439	3.2	280.4	280.4	280.9	0.5
AA	293.81	6,512 ²	99,103	1.1	282.0	282.0	282.6	0.6
AB	295.21	9,490 ²	117,709	0.9	282.7	282.7	283.4	0.7
AC	295.48	10,973 ²	109,150	1.0	283.1	283.1	283.9	0.8
AD	296.71	13,890 ²	168,378	0.6	283.8	283.8	284.7	0.9
AE	298.33	10,413 ²	111,962	0.9	284.3	284.3	285.2	0.9
AF	299.82	10,203 ²	119,038	0.9	284.9	284.9	285.8	0.9
AG	301.31	10,877 ²	123,258	0.9	286.0	286.0	286.9	0.9

¹ FEET ABOVE MOUTH

² THIS WIDTH EXTENDS BEYOND COUNTY BOUNDARY

TABLE 9

FEDERAL EMERGENCY MANAGEMENT AGENCY

**RANKIN COUNTY, MS
AND INCORPORATED AREAS**

FLOODWAY DATA

PEARL RIVER

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
PEARL RIVER TRIBUTARY 1								
A	1,205	76	563	3.3	268.2	263.3 ²	264.3	1.0
B	2,355	41	181	8.7	268.2	264.0 ²	265.0	1.0
C	3,240	39	173	5.9	270.3	270.3	270.3	0.0
D	3,868	34	118	8.6	272.2	272.2	272.3	0.1
E	4,778	41	161	6.3	277.9	277.9	277.9	0.0
F	4,930	43	183	5.5	279.1	279.1	279.1	0.0
G	5,890	28	81	9.6	280.6	280.6	280.6	0.0
H	7,210	35	135	5.8	289.1	289.1	289.1	0.0
I	7,570	29	99	7.9	291.5	291.5	291.5	0.0
J	8,090	36	140	5.6	294.4	294.4	294.4	0.0
K	8,630	37	145	5.4	296.1	296.1	296.1	0.0
PEARL RIVER TRIBUTARY 2								
A	1,295	58	311	2.3	270.4	264.8 ²	265.4	0.6
B	1,889	23	98	7.2	270.4	265.6 ²	266.0	0.4
C	2,746	51	278	2.0	270.4	267.3 ²	267.4	0.1
D	3,574	34	100	6.7	270.4	268.4 ²	268.6	0.2
E	3,686	39	228	2.9	270.4	270.4	271.1	0.7
F	4,907	100	227	3.0	270.4	270.4	271.2	0.8
G	5,620	28	104	6.4	270.4	270.4	271.1	0.7

¹ FEET ABOVE CITY OF RICHLAND CORPORATE LIMITS

² ELEVATION COMPUTED WITHOUT CONSIDERATION OF BACKWATER EFFECTS FROM PEARL RIVER

TABLE 9

FEDERAL EMERGENCY MANAGEMENT AGENCY

**RANKIN COUNTY, MS
AND INCORPORATED AREAS**

FLOODWAY DATA

**PEARL RIVER TRIBUTARY 1 – PEARL
RIVER TRIBUTARY 2**

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
PEARL RIVER TRIBUTARY 3								
A	2,215 ¹	81	197	4.1	281.9	271.4 ⁵	271.5	0.1
B	2,898 ¹	48	166	4.9	281.9	273.8 ⁵	273.8	0.0
C	3,144 ¹	62	415	2.1	281.9	278.2 ⁵	278.2	0.0
D	4,835 ¹	97	384	2.1	281.9	278.5 ⁵	279.1	0.6
E	5,051 ¹	36	306	2.1	281.9	280.2 ⁵	281.0	0.8
F	6,081 ¹	400	2,039	0.3	281.9	280.2 ⁵	281.1	0.9
G	6,151 ¹	500	2,591	0.3	281.9	280.2 ⁵	281.1	0.9
H	6,765 ¹	300	1,381	0.5	281.9	280.2 ⁵	281.1	0.9
I	6,938 ¹	400	2,068	0.3	281.9	280.2 ⁵	281.1	0.9
PELAHATCHIE BAY UNNAMED TRIBUTARY								
A	512 ²	151	841	1.4	306.5	306.5	306.5	0.0
PELAHATCHIE CREEK (LOWER REACH)								
A	1,800 ³	3,147	22,497	1.6	301.6	301.6	302.0	0.4
B	4,700 ³	4,362	29,519	1.2	303.5	303.5	304.5	1.0
C	14,895 ³	3,799 ⁴	29,543	1.2	309.5	309.5	310.4	0.9
D	22,345 ³	2,837	23,311	1.4	313.3	313.3	314.2	0.9
E	29,395 ³	3,987	35,788	0.9	315.3	315.3	316.3	1.0

¹ FEET ABOVE CONFLUENCE WITH HOG CREEK

⁵ ELEVATION COMPUTED WITHOUT CONSIDERATION OF
BACKWATER EFFECTS FROM PEARL RIVER

² FEET ABOVE PEARL RIVER VALLEY WATER SUPPLY DISTRICT CORPORATE LIMITS

³ FEET ABOVE MOUTH

⁴ COMBINED FLOWWAY WIDTH OF PELAHATCHIE CREEK (LOWER REACH) AND CLARK CREEK

TABLE 9

FEDERAL EMERGENCY MANAGEMENT AGENCY

**RANKIN COUNTY, MS
AND INCORPORATED AREAS**

FLOODWAY DATA

**PEARL RIVER TRIBUTARY 3 -
PELAHATCHIE BAY UNNAMED TRIBUTARY
- PELAHATCHIE CREEK (LOWER REACH)**

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
PELAHATCHIE CREEK (UPPER REACH)								
D	165,868 ¹	3,063	26,431	0.5	349.1	349.1	350.1	1.0
E	167,498 ¹	2,955	4,854	2.9	349.4	349.4	350.3	0.9
F	169,343 ¹	2,354	21,816	0.6	351.6	351.6	351.7	0.1
G	171,305 ¹	1,743	2,453	5.8	352.0	352.0	352.5	0.5
H	173,500 ¹	1,780	14,092	1.0	353.2	353.2	353.6	0.4
I	176,995 ¹	1,910	14,340	1.0	353.5	353.5	354.0	0.5
J	178,385 ¹	2,150	12,897	1.1	353.7	353.7	354.3	0.6
K	180,485 ¹	2,980	12,600	1.1	353.9	353.9	354.6	0.7
L	183,295 ¹	2,042	11,739	1.2	354.3	354.3	355.3	1.0
PELAHATCHIE CREEK TRIBUTARY								
A	1,885 ²	127	457	3.0	306.6	306.6	306.9	0.3
B	3,080 ²	564	1,245	1.1	307.3	307.3	308.3	1.0
C	5,380 ²	100	810	1.4	317.1	317.1	317.1	0.0
D	7,100 ²	217	638	1.8	321.7	321.7	321.7	0.0
E	8,500 ²	179	595	2.0	324.8	324.8	325.8	1.0

¹ FEET ABOVE MOUTH

² FEET ABOVE CONFLUENCE WITH PELAHATCHIE CREEK (LOWER REACH)

NOTE: FLOODWAY DATA FOR CROSS-SECTIONS A THROUGH C FOR PELAHATCHIE CREEK (UPPER REACH) NOT AVAILABLE

TABLE 9

FEDERAL EMERGENCY MANAGEMENT AGENCY

**RANKIN COUNTY, MS
AND INCORPORATED AREAS**

FLOODWAY DATA

**PELAHATCHIE CREEK (UPPER REACH) –
PELAHATCHIE CREEK TRIBUTARY**

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
PELAHATCHIE CREEK TRIBUTARY 1								
A	684	36	207	4.8	351.4	344.6 ²	345.1	0.5
B	816	29	250	3.9	351.4	345.3 ²	345.7	0.4
C	2,196	43	189	5.2	351.4	347.6 ²	348.0	0.4
D	2,385	60	366	2.7	351.4	348.1 ²	348.6	0.4
E	2,740	28	198	5.0	351.4	348.4 ²	349.1	0.7
F	2,878	50	266	3.7	351.4	349.3 ²	349.7	0.5
G	4,071	48	134	7.4	351.4	351.1 ²	351.4	0.3
H	4,155	90	849	1.2	359.0	359.0	359.0	0.0
I	4,452	90	658	1.5	359.0	359.0	359.0	0.0
J	4,623	65	627	1.6	359.0	359.0	359.2	0.2
K	5,592	61	317	1.5	359.0	359.0	359.4	0.4
L	5,755	65	477	1.0	359.1	359.1	359.6	0.5
M	7,457	68	100	4.8	362.1	362.1	362.8	0.7
N	8,477	28	89	5.5	365.8	365.8	366.5	0.7
O	8,883	100	340	1.4	370.2	370.2	371.2	1.0

¹ FEET ABOVE CONFLUENCE WITH PELAHATCHIE CREEK (UPPER REACH)

² ELEVATION COMPUTED WITHOUT CONSIDERATION OF BACKWATER EFFECTS FROM PELAHATCHIE CREEK

TABLE 9

FEDERAL EMERGENCY MANAGEMENT AGENCY

**RANKIN COUNTY, MS
AND INCORPORATED AREAS**

FLOODWAY DATA

PELAHATCHIE CREEK TRIBUTARY 1

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
PIERCE CREEK								
A	180 ¹	1,780	1,346	2.0	353.1	345.3 ⁴	346.1	0.8
B	1,615 ¹	1,910	2,379	1.1	353.5	348.2 ⁴	349.1	0.9
C	3,435 ¹	2,150	3,007	0.9	353.6	348.8 ⁴	349.8	1.0
D	6,041 ¹	1,980	3,670	0.7	353.9	350.9 ⁴	351.7	0.8
E	9,446 ¹	232	1,278	2.1	354.8	354.8	355.5	0.7
F	10,766 ¹	303	1,523	1.7	355.8	355.8	356.1	0.3
G	13,036 ¹	505	2,502	1.1	356.7	356.7	357.7	1.0
H	15,610 ¹	54	476	5.5	358.4	358.4	359.3	0.9
I	16,324 ¹	125	206	12.8	359.2	359.2	359.2	0.0
J	17,226 ¹	1,210	8,472	0.3	361.7	361.7	362.7	1.0
K	18,026 ¹	1,116	4,313	0.5	361.7	361.7	362.7	1.0
L	19,042 ¹	489	1,044	2.2	363.5	363.5	363.5	0.0
M	20,041 ¹	414	1,549	1.2	364.7	364.7	365.6	0.9
N	21,192 ¹	571	2,567	0.7	368.3	368.3	368.3	0.0
PLUMMER SLOUGH								
A	6,310 ²	834	3,003	0.9	308.2	304.0 ⁵	304.3	0.3
B	7,900 ²	1,153	4,328	0.5	308.5	305.0 ⁵	305.6	0.6
C	10,960 ²	696	1882	1.1	309.6	306.9	307.9	1.0
D	14,520 ²	270	1,053	1.5	314.9	314.9	315.8	0.9
E	17,860 ²	319	1,380	0.7	324.2	324.2	325.2	1.0

¹ FEET ABOVE CONFLUENCE WITH PELAHATCHIE CREEK (UPPER REACH)

² FEET ABOVE CONFLUENCE WITH PELAHATCHIE CREEK (LOWER REACH)

³ COMBINED PELAHATCHIE CREEK (LOWER REACH) AND PLUMMER SLOUGH FLOODWAY

⁴ ELEVATION COMPUTED WITHOUT CONSIDERATION OF BACKWATER EFFECTS FROM PELAHATCHIE CREEK (UPPER REACH)

⁵ ELEVATION COMPUTED WITHOUT CONSIDERATION OF BACKWATER EFFECTS FROM PELAHATCHIE CREEK (LOWER REACH)

TABLE 9

FEDERAL EMERGENCY MANAGEMENT AGENCY

**RANKIN COUNTY, MS
AND INCORPORATED AREAS**

FLOODWAY DATA

PIERCE CREEK – PLUMMER SLOUGH

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
PRAIRIE BRANCH CANAL								
A	0 ¹	95	933	3.3	279.2	269.3 ³	270.3	1.0
B	667 ¹	114	1,014	3.0	279.2	269.6 ³	270.5	0.9
C	1,439 ¹	95	820	3.8	279.2	270.8 ³	271.5	0.7
D	2,859 ¹	145	988	3.1	279.2	272.1 ³	272.9	0.8
E	6,269 ¹	75	530	4.8	279.2	273.6 ³	274.6	1.0
F	7,169 ¹	65	509	5.0	279.2	274.9 ³	275.6	0.7
G	8,413 ¹	51	319	5.1	279.2	276.5 ³	276.9	0.4
H	10,123 ¹	75	457	3.6	279.2	278.0 ³	278.3	0.3
I	11,583 ¹	79	323	5.0	279.6	279.6	279.7	0.1
J	13,506 ¹	516	1,505	0.5	288.5	288.5	289.5	1.0
K	14,997 ¹	199	610	2.2	292.1	292.1	292.7	0.6
L	16,585 ¹	311	1,137	2.3	297.1	297.1	297.7	0.6
M	17,685 ¹	322	593	2.3	299.2	299.2	300.1	0.9
N	18,740 ¹	304	353	3.2	303.2	303.2	304.1	0.9
PRAIRIE BRANCH CANAL TRIBUTARY 1								
A	1,490 ²	721	2368	0.7	279.4	274.3 ⁴	275.3	1.0
B	4,555 ²	84	247	4.8	279.4	276.1 ⁴	276.6	0.5
C	5,909 ²	480	1541	0.8	279.4	276.4 ⁴	277.4	1.0
D	7,233 ²	791	1377	0.9	279.4	276.6 ⁴	277.6	1.0
E	8,483 ²	26	105	9.5	281.8	281.8	282.3	0.5
F	8,798 ²	120	622	1.6	284.7	284.7	284.9	0.2
G	9,293 ²	120	471	2.1	284.7	284.7	285.4	0.7

¹ FEET ABOVE LIMIT OF DETAILED STUDY (LOCATED APPROXIMATELY 560 FEET DOWNSTREAM OF STATE ROUTE 468)

² FEET ABOVE CONFLUENCE WITH PRAIRIE BRANCH CANAL

³ ELEVATION COMPUTED WITHOUT CONSIDERATION OF BACKWATER EFFECTS FROM PEARL RIVER

⁴ ELEVATION COMPUTED WITHOUT CONSIDERATION OF BACKWATER EFFECTS FROM PRAIRIE BRANCH CANAL

TABLE 9

FEDERAL EMERGENCY MANAGEMENT AGENCY

**RANKIN COUNTY, MS
AND INCORPORATED AREAS**

FLOODWAY DATA

**PRAIRIE BRANCH CANAL – PRAIRIE
BRANCH TRIBUTARY 1**

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
RICHLAND CREEK								
A	21,050	279	4,355	4.1	271.7	268.6 ²	269.3	0.7
B	23,150	1,173	9,797	1.8	271.7	269.9 ²	270.7	0.8
C	24,600	945	8,041	2.2	271.7	270.8 ²	271.6	0.8
D	26,250	322	3,535	5.1	271.7	271.8 ²	272.6	0.8
E	26,461	320	3,328	5.7	272.3	272.3	272.5	0.2
F	28,965	2,939	16,681	1.1	272.9	272.9	273.7	0.8
G	31,836	1,130	7,595	2.5	274.1	274.1	274.9	0.8
H	33,292	2,553	13,559	1.4	275.1	275.1	275.9	0.8
I	34,812	2,178	12,529	1.5	275.8	275.8	276.7	0.9
J	36,287	1,348	9,270	2.0	276.5	276.5	277.4	0.9
K	37,359	1,327	7,935	2.4	277.4	277.4	278.3	0.9
L	39,759	2,054	14,337	1.3	279.7	279.7	280.7	1.0
M	41,495	2,200	14,406	1.3	280.6	280.6	281.5	0.9
N	43,363	2,215	12,257	1.5	281.9	281.9	282.7	0.8
O	46,633	1,557	9,505	2.0	284.6	284.6	285.3	0.7
P	47,043	1,460	13,455	1.4	284.9	284.9	285.7	0.8
Q	53,745	4,020	19,551	1.0	287.5	287.5	288.5	1.0
R	58,525	2,581	19,730	0.9	289.3	289.3	290.3	1.0
S	67,141	3,995	17,897	1.0	292.1	292.1	292.6	0.5
T	74,573	3,730	17,851	0.9	295.5	295.5	296.2	0.7
U	79,013	3,690	16,552	0.9	297.3	297.3	298.3	1.0
V	87,411	2,506	12,063	1.3	302.7	302.7	303.2	0.5
W	92,855	4,001	27,060	0.6	308.5	308.5	308.8	0.3
X	96,465	4,794	18,791	0.8	309.1	309.1	309.7	0.6
Y	107,625	5,137	25,024	0.5	310.8	310.8	311.8	1.0
Z	116,877	2,384	14,317	0.7	312.4	312.4	313.3	0.9

¹ FEET ABOVE CONFLUENCE WITH PEARL RIVER

² ELEVATION COMPUTED WITHOUT CONSIDERATION OF BACKWATER EFFECTS FROM PEARL RIVER

TABLE 9

FEDERAL EMERGENCY MANAGEMENT AGENCY

RANKIN COUNTY, MS

AND INCORPORATED AREAS

FLOODWAY DATA

RICHLAND CREEK

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
RICHLAND CREEK (CONTINUED)								
AA	121,502	1,415	4,729	2.0	316.2	316.2	316.8	0.6
AB	123,353	2,400	7,353	1.3	319.3	319.3	319.7	0.4
AC	123,783	160	1,009	9.4	319.8	319.8	320.0	0.2
AD	123,943	1,100	4,772	2.0	322.7	322.7	322.7	0.0
AE	126,268	1,200	5,900	1.0	325.0	325.0	326.0	1.0
AF	128,668	1,250	6,670	1.4	329.8	329.8	330.7	0.9
AG	131,998	2,348	6,899	1.2	332.0	332.0	332.6	0.6
AH	132,268	107	830	10.1	332.0	332.0	332.6	0.6
AI	134,725	585	2,969	2.6	335.6	335.6	336.6	1.0
AJ	138,475	328	3,989	2.5	339.4	339.4	340.2	0.8
AK	143,691	110	553	12.8	344.3	344.3	344.7	0.4
AL	143,886	151	1,430	7.6	348.0	348.0	348.0	0.0
AM	145,540	656	5,323	1.3	349.3	349.3	349.8	0.5
AN	146,834	804	5,941	1.2	349.6	349.6	350.4	0.8
AO	148,848	81	456	13.5	351.4	351.4	351.4	0.0
AP	149,102	870	3,022	2.0	355.7	355.7	355.7	0.0
AQ	152,052	783	2,159	2.9	357.0	357.0	357.4	0.4
AR	153,382	653	1,889	2.1	358.0	358.0	358.9	0.9
AS	155,382	661	1,632	2.0	359.8	359.8	360.7	0.9
AT	157,132	484	1,175	2.8	361.7	361.7	362.3	0.6
AU	159,332	48	364	8.9	363.2	363.2	364.1	0.9
AV	160,356	500	3,618	0.9	367.8	367.8	367.8	0.0
AW	164,836	411	1,529	2.1	368.3	368.3	369.3	1.0
AX	167,962	775	2,700	0.8	372.2	372.2	372.9	0.7
AY	170,934	235	446	4.6	373.1	373.1	373.9	0.8

¹ FEET ABOVE CONFLUENCE WITH PEARL RIVER

NOTE: CROSS-SECTIONS AZ THROUGH BC DO NOT CONTAIN FLOODWAY DATA

TABLE 9

FEDERAL EMERGENCY MANAGEMENT AGENCY

**RANKIN COUNTY, MS
AND INCORPORATED AREAS**

FLOODWAY DATA

RICHLAND CREEK

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
RICHLAND CREEK TRIBUTARY 1								
A	8,365 ¹	914	3408	0.4	291.7	291.7	292.7	1.0
B	8,962 ¹	401	1711	0.9	293.7	293.7	294.7	1.0
C	12,762 ¹	80	163	5.8	300.9	300.9	301.5	0.6
D	14,621 ¹	40	168	5.6	303.8	303.8	304.7	0.9
SPRING BRANCH								
A	1,380 ²	427	981	1.9	300.9	297.7	298.0	0.3
B	4,210 ²	216	1,181	1.6	301.5	301.5	302.3	0.8
C	7,204 ²	200	805	2.4	306.4	306.4	307.4	1.0
D	5,085 ²	160	555	2.6	309.8	309.8	310.7	0.9
E	8,794 ²	160	234	4.3	315.8	315.8	315.8	0.0
F	11,694 ²	200	725	1.2	321.6	321.6	322.6	1.0
G	13,334 ²	190	212	4.2	327.4	327.4	327.4	0.0
H	14,634 ²	190	554	1.6	331.0	331.0	332.0	1.0
SQUIRREL BRANCH								
A	880 ³	58	291	9.3	270.9	267.8 ⁴	267.9	0.1
B	1,996 ³	720	2814	1.0	270.9	269.6 ⁴	270.0	0.4
C	2,938 ³	250	820	3.3	270.9	270.6 ⁴	270.7	0.1
D	4,463 ³	151	723	2.1	272.4	272.0 ⁵	272.3	0.3
E	7,103 ³	877	1794	0.9	273.4	273.4	274.0	0.6
F	8,603 ³	682	1102	1.0	275.2	275.2	276.1	0.9

¹ FEET ABOVE CONFLUENCE WITH RICHLAND CREEK

² FEET ABOVE CONFLUENCE WITH PELAHATCHIE CREEK

³ FEET ABOVE CITY OF RICHLAND CORPORATE LIMITS

⁴ ELEVATION COMPUTED WITHOUT CONSIDERATION OF BACKWATER EFFECTS FROM PEARL RIVER

⁵ ELEVATION COMPUTED WITHOUT CONSIDERATION OF BACKWATER EFFECTS FROM RICHLAND CREEK

TABLE 9

FEDERAL EMERGENCY MANAGEMENT AGENCY

**RANKIN COUNTY, MS
AND INCORPORATED AREAS**

FLOODWAY DATA

**RICHLAND CREEK TRIBUTARY 1 – SPRING
BRANCH – SQUIRREL BRANCH**

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
STEEN CREEK								
A	500	1,686	9,712	1.1	285.0	285.0	286.0	1.0
B	6,015	1,890	8,605	1.2	287.0	287.0	288.0	1.0
C	7,657	1,242	5,329	2.0	288.2	288.2	289.2	1.0
D	9,417	1,150	5,339	2.0	289.8	289.8	290.7	0.9
E	10,390	126	1,404	6.7	290.8	290.8	291.0	0.2
F	11,603	798	10,215	1.9	292.0	292.0	292.3	0.3
G	12,639	558	3,279	5.5	292.0	292.0	292.3	0.3
H	13,805	800	8,284	2.5	292.8	292.8	295.2	0.4
I	15,902	978	8,284	2.5	294.9	294.9	295.2	0.3
J	17,292	108	1012	3.4	346.1	346.1	346.1	0.0
K	21,445	310	1851	1.9	346.8	346.8	346.8	0.0
L	23,781	613	1495	2.3	347.0	347.0	347.2	0.2
M	28,564	344	3,204	3.2	306.5	306.5	306.9	0.4
N	30,730	628	4,704	2.9	308.1	308.1	308.5	0.4
O	37,752	892	5,647	2.1	308.8	308.8	309.5	0.7
P	41,096	512	1,421	6.1	313.3	313.3	313.6	0.3
Q	47,650	577	1,958	2.6	324.6	324.6	324.9	0.3
R	52,793	458	3,726	2.1	327.6	327.6	328.5	0.9

¹ FEET ABOVE LIMIT OF DETAILED STUDY (LOCATED APPROXIMATELY 1.96 MILES DOWNSTREAM OF SOUTH CHURCH STREET)

TABLE 9

FEDERAL EMERGENCY MANAGEMENT AGENCY

**RANKIN COUNTY, MS
AND INCORPORATED AREAS**

FLOODWAY DATA

STEEN CREEK

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
TERRAPIN SKIN CREEK								
A	17,360	1545	5,814	1.2	300.3	300.3	301.3	1.0
B	19,172	88	763	8.9	301.8	301.8	302.8	1.0
C	22,675	584	2,420	2.7	307.2	307.2	307.8	0.6
D	25,955	629	2,452	2.7	311.7	311.7	312.3	0.6
E	30,733	150	1,482	3.6	316.9	316.9	317.6	0.7
F	32,352	170	1,717	3.1	318.0	318.0	318.8	0.8
G	34,619	182	1,892	2.9	319.3	319.3	319.9	0.6
H	38,649	407	1,553	2.7	331.9	331.9	332.9	1.0
I	42,359	606	1,485	2.4	339.7	339.7	340.5	0.8
J	44,439	108	1,012	3.4	346.1	346.1	346.1	0.0
K	45,009	310	1,851	1.9	346.8	346.8	346.8	0.0
L	45,892	613	1,495	2.3	347.0	347.0	347.2	0.2
M	46,827	682	2,103	1.4	350.1	350.1	350.8	0.7
N	47,747	442	1,638	1.4	351.3	351.3	352.3	1.0
O	48,647	534	1,288	1.8	353.9	353.9	354.7	0.8
P	49,675	534	1,568	1.4	356.5	356.5	357.5	1.0
Q	50,612	275	941	2.4	359.5	359.5	360.5	1.0
R	51,681	347	1,698	1.2	365.5	365.5	366.2	0.7
S	52,711	160	628	2.7	366.3	366.3	367.3	1.0
T	55,186	170	641	1.4	372.4	372.4	373.0	0.6
U	56,497	325	979	0.9	379.2	379.2	379.4	0.2
V	56,625	70	155	5.7	379.3	379.3	379.3	0.0
W	59,347	34	173	3.2	392.1	392.1	392.3	0.2

¹ FEET ABOVE CONFLUENCE WITH RICHLAND CREEK

TABLE 9

FEDERAL EMERGENCY MANAGEMENT AGENCY

**RANKIN COUNTY, MS
AND INCORPORATED AREAS**

FLOODWAY DATA

TERRAPIN SKIN CREEK

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
TERRAPIN SKIN CREEK TRIBUTARY 1								
A	705	333 ²	472	2.5	346.8	345.7 ³	346.4	0.7
B	2,232	295	646	0.6	350.0	348.8 ³	349.7	0.9
C	3,072	49	78	4.9	353.8	353.8	353.8	0.0
D	4,232	51	162	1.8	361.0	361.0	362.0	1.0
E	5,087	20	65	4.4	364.8	364.8	365.7	0.9
F	6,172	25	90	2.3	371.4	371.4	372.4	1.0
TERRAPIN SKIN CREEK TRIBUTARY 2								
A	249	40 ²	113	9.6	346.8	338.0 ³	338.0	0.0
B	1,989	47	177	6.1	347.7	345.9 ³	345.9	0.0
C	2,943	92	194	4.5	352.9	352.9	353.0	0.1
D	4,070	164	669	1.3	357.1	357.1	358.0	0.9
E	5,000	94	384	2.3	361.6	361.6	362.1	0.5
F	5,968	125	385	2.2	364.6	364.6	365.5	0.9
G	7,088	70	231	2.5	369.3	369.3	369.5	0.2
H	7,688	151	471	1.3	370.1	370.1	370.9	0.8

¹ FEET ABOVE CONFLUENCE WITH TERRAPIN SKIN CREEK

² THIS WIDTH COMPUTED WITHOUT CONSIDERATION OF INFLUENCE FROM TERRAPIN SKIN CREEK FLOODWAY

³ ELEVATION COMPUTED WITHOUT CONSIDERATION OF OVERFLOW FROM TERRAPIN SKIN CREEK

TABLE 9

FEDERAL EMERGENCY MANAGEMENT AGENCY

**RANKIN COUNTY, MS
AND INCORPORATED AREAS**

FLOODWAY DATA

**TERRAPIN SKIN CREEK TRIBUTARY 1 –
TERRAPIN SKIN CREEK TRIBUTARY 2**

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
TOWN BRANCH								
A	2,898 ¹	58	279	3.3	292.3	288.8 ³	289.8	1.0
B	4,346 ¹	81	309	3.0	293.5	293.5	294.2	0.7
C	5,480 ¹	65	281	3.3	298.5	298.5	299.4	0.9
D	6,028 ¹	80	253	3.7	301.0	301.0	301.6	0.6
E	6,440 ¹	108	555	1.7	306.2	306.2	306.2	0.0
F	7,029 ¹	40	212	4.0	306.5	306.5	306.8	0.3
G	7,396 ¹	203	823	1.0	308.8	308.8	309.5	0.7
H	8,176 ¹	101	336	2.4	309.2	309.2	310.2	1.0
I	9,796 ¹	71	283	2.9	314.4	314.4	314.5	0.1
J	11,216 ¹	27	159	5.1	318.1	318.1	319.0	0.9
K	12,061 ¹	25	106	6.5	322.9	322.9	323.7	0.8
L	12,991 ¹	54	219	3.2	326.9	326.9	327.8	0.9
M	14,120 ¹	77	144	4.8	331.0	331.0	331.4	0.4
N	15,274 ¹	69	262	2.7	338.9	338.9	339.9	1.0
O	16,289 ¹	18	72	5.9	343.9	343.9	344.5	0.6
TURTLE CREEK								
A	120 ²	100	655	2.3	302.6	303.3	303.7	0.4
B	400 ²	100	869	1.7	305.3	305.6	305.6	0.0
C	2,640 ²	100	325	3.2	308.5	308.5	308.5	0.0
D	5,150 ²	100	421	2.5	314.7	314.7	315.5	0.8

¹ FEET ABOVE CONFLUENCE WITH STEEN CREEK

² FEET ABOVE MOUTH

³ ELEVATION COMPUTED WITHOUT CONSIDERATION OF BACKWATER EFFECTS FROM STEEN CREEK

TABLE 9

FEDERAL EMERGENCY MANAGEMENT AGENCY

**RANKIN COUNTY, MS
AND INCORPORATED AREAS**

FLOODWAY DATA

TOWN BRANCH – TURTLE CREEK

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
WOODRUN CREEK								
D	15,051	23	118	6.0	337.6	337.6	337.6	0.0
E	15,366	54	208	3.9	340.3	340.3	340.3	0.0
F	17,217	119	112	5.5	343.4	343.4	343.4	0.0
G	18,730	115	219	2.8	351.9	351.9	351.9	0.0
H	19,029	84	459	1.5	355.0	355.0	355.8	0.8
I	19,669	12	34	6.4	358.4	358.4	358.4	0.0
J	19,821	90	394	0.6	360.1	360.1	361.1	1.0

¹ FEET ABOVE CONFLUENCE WITH TERRAPIN SKIN CREEK

NOTE: CROSS-SECTIONS A THROUGH C DO NOT HAVE FLOODWAY DATA

TABLE 9

FEDERAL EMERGENCY MANAGEMENT AGENCY

**RANKIN COUNTY, MS
AND INCORPORATED AREAS**

FLOODWAY DATA

WOODRUN CREEK

5.0 **INSURANCE APPLICATION**

For flood insurance rating purposes, flood insurance zone designations are assigned to a community based on the results of the engineering analyses. These zones are as follows:

Zone A

Zone A is the flood insurance risk zone that corresponds to the 1-percent-annual-chance floodplains that are determined in the FIS by approximate methods. Because detailed hydraulic analyses are not performed for such areas, no base (1-percent-annual-chance) flood elevations (BFEs), or base flood depths are shown within this zone.

Zone AE

Zone AE is the flood insurance risk zone that corresponds to the 1-percent-annual-chance floodplains that are determined in the FIS by detailed methods. In most instances, whole-foot BFEs derived from the detailed hydraulic analyses are shown at selected intervals within this zone.

Zone AH

Zone AH is the flood insurance rate zone that corresponds to the areas of 1-percent-annual-chance shallow flooding (usually areas of ponding) where average depths are between 1 and 3 feet. Whole-foot base flood elevations derived from the detailed hydraulic analyses are shown at selected intervals within the zone.

Zone AO

Zone AO is the flood insurance rate zone that corresponds to the areas of 1-percent-annual-chance shallow flooding (usually sheet flow on sloping terrain) where the average depths are between 1 and 3 feet. Average whole-foot depths derived from the detailed hydraulic analyses are shown within the zone.

Zone A99

Zone A99 is the flood insurance rate zone that corresponds to areas of the 1-percent floodplain that will be protected by a Federal flood protection system where construction has reached specified statutory milestones. No base flood elevations or depths are shown within this zone.

Zone V

Zone V is the flood insurance rate zone that corresponds to the 1-percent coastal floodplains that have additional hazards associated with storm waves. Because approximate hydraulic analyses are performed for such areas, no base flood elevations are shown within this zone.

Zone VE

Zone VE is the flood insurance rate zone that corresponds to the 1-percent coastal floodplains that have additional hazards associated with storm waves. Whole-foot base flood elevations derived from the detailed hydraulic analyses are shown at selected intervals within this zone.

Zone X

Zone X is the flood insurance risk zone that corresponds to areas outside the 0.2-percent annual chance floodplain, areas within the 0.2-percent annual chance floodplain, areas of 1-percent-annual-chance flooding where average depths are less than 1 foot, areas of 1-percent-annual-chance flooding where the contributing drainage area is less than 1 square mile, and areas protected from the base flood by levees. No BFEs or depths are shown within this zone.

Zone D

Zone D is the flood insurance rate zone that corresponds to unstudied areas where flood hazards are undetermined, but possible.

6.0 **FLOOD INSURANCE RATE MAP**

The FIRM is designed for flood insurance and floodplain management applications.

For flood insurance applications, the map designates flood insurance risk zones as described in Section 5.0 and, in the 1-percent annual chance floodplains that were studied by detailed methods, shows selected whole-foot BFEs or average depths. Insurance agents use the zones and BFEs in conjunction with information on structures and their contents to assign premium rates for flood insurance policies.

For floodplain management applications, the map shows by tints, screens, and symbols, the 1-percent annual chance floodplain and the locations of selected cross sections used in the hydraulic analyses and floodway computations.

The countywide FIRM presents flooding information for the entire geographic area of Rankin County. Previously, FIRMs were prepared for each incorporated community and the unincorporated areas of the County identified as flood-prone. This countywide FIRM also includes flood-hazard information that was presented separately on Flood Boundary and Floodway Maps (FBFMs), where applicable. Historical data relating to the maps prepared for each community up to and including this countywide FIS, are presented in Table 10, "Community Map History."

COMMUNITY NAME	INITIAL IDENTIFICATION	FLOOD HAZARD BOUNDARY MAP REVISIONS DATE	FIRM EFFECTIVE DATE	FIRM REVISIONS DATE
Brandon, City of	June 7, 1974	July 30, 1976	September 17, 1980	September 18, 1986 November 5, 2003
Florence, City of	August 23, 1974	January 30, 1976	August 15, 1980	November 5, 2003
Flowood, City of	November 11, 1977	--	December 15, 1982	March 1, 1984 July 5, 1994 November 5, 2003
Jackson, City of	December 6, 1974	October 17, 1975	April 1, 1980	June 25, 1982 September 18, 1986 December 18, 1988 April 4, 1994 June 16, 1999
Pearl, City of	June 21, 1974	February 27, 1976	December 15, 1982	March 17, 1997 November 5, 2003
Pearl River Valley Water Valley Water Supply District	October 18, 1974	--	March 2, 1993	January 19, 1996
Pelahatchie, Town of	June 7, 1974	January 30, 1976	January 2, 1981	November 5, 2003
Puckett, Township of	August 23, 1974	July 2, 1976 February 8, 1980	December 1, 1990	November 5, 2003
Rankin County (Unincorporated Areas)	December 13, 1974	November 4, 1977	December 15, 1982	June 19, 1989 June 16, 1993 November 5, 2003
Richland, City of	April 28, 1978	--	February 2, 1983	March 15, 1994 November 5, 2003

TABLE 10

FEDERAL EMERGENCY MANAGEMENT AGENCY

**RANKIN COUNTY, MS
AND INCORPORATED AREAS**

COMMUNITY MAP HISTORY

7.0 **OTHER STUDIES**

This FIS report either supersedes or is compatible with all previous studies published on streams studied in this report and should be considered authoritative for the purposes of the NFIP.

8.0 **LOCATION OF DATA**

Information concerning the pertinent data used in the preparation of this study can be obtained by contacting Federal Insurance and Mitigation Division, FEMA Region IV, Koger-Center — Rutgers Building, 3003 Chamblee Tucker Road, Atlanta, GA 30341.

9.0 **BIBLIOGRAPHY AND REFERENCES**

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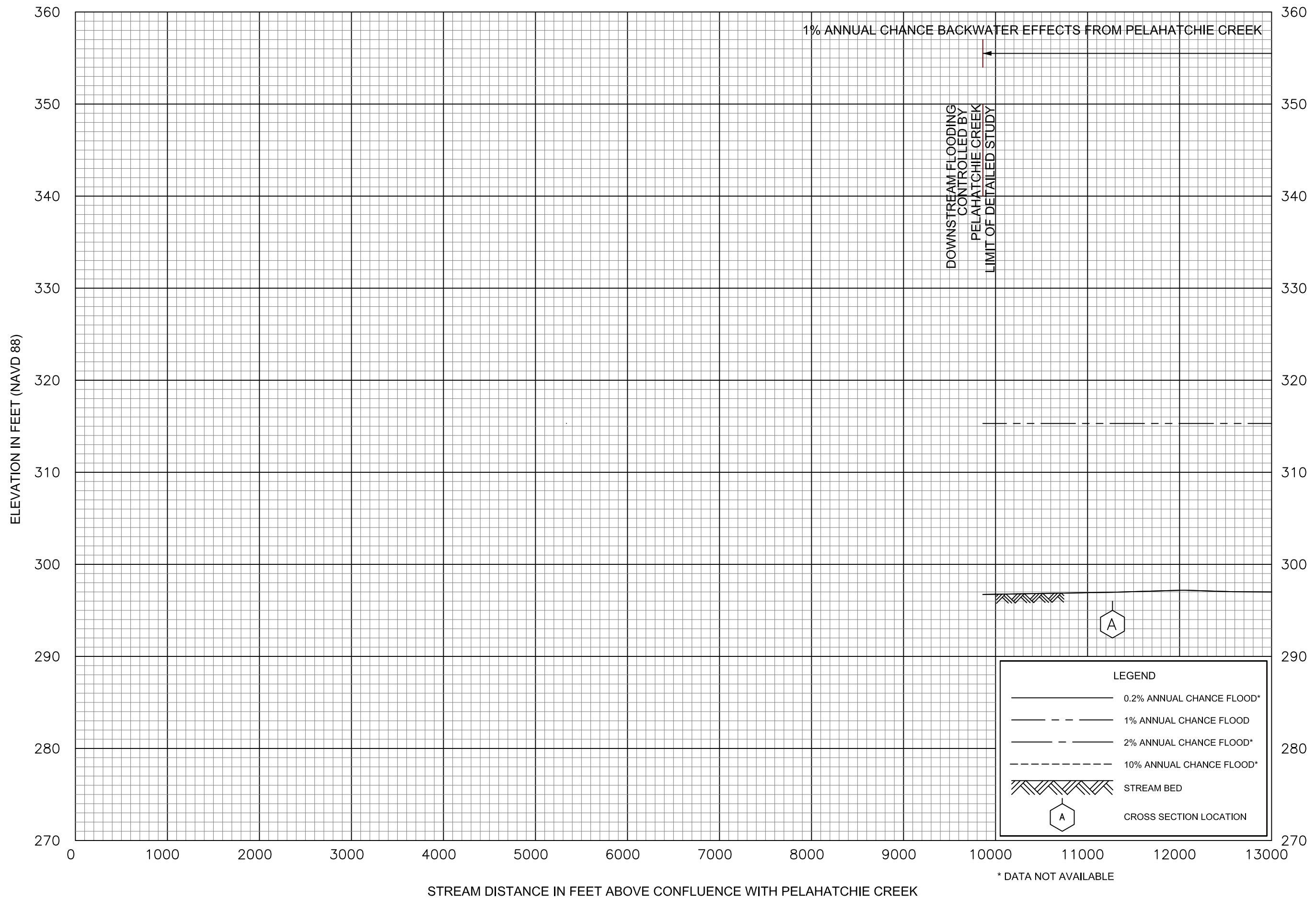
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FEDERAL EMERGENCY MANAGEMENT AGENCY

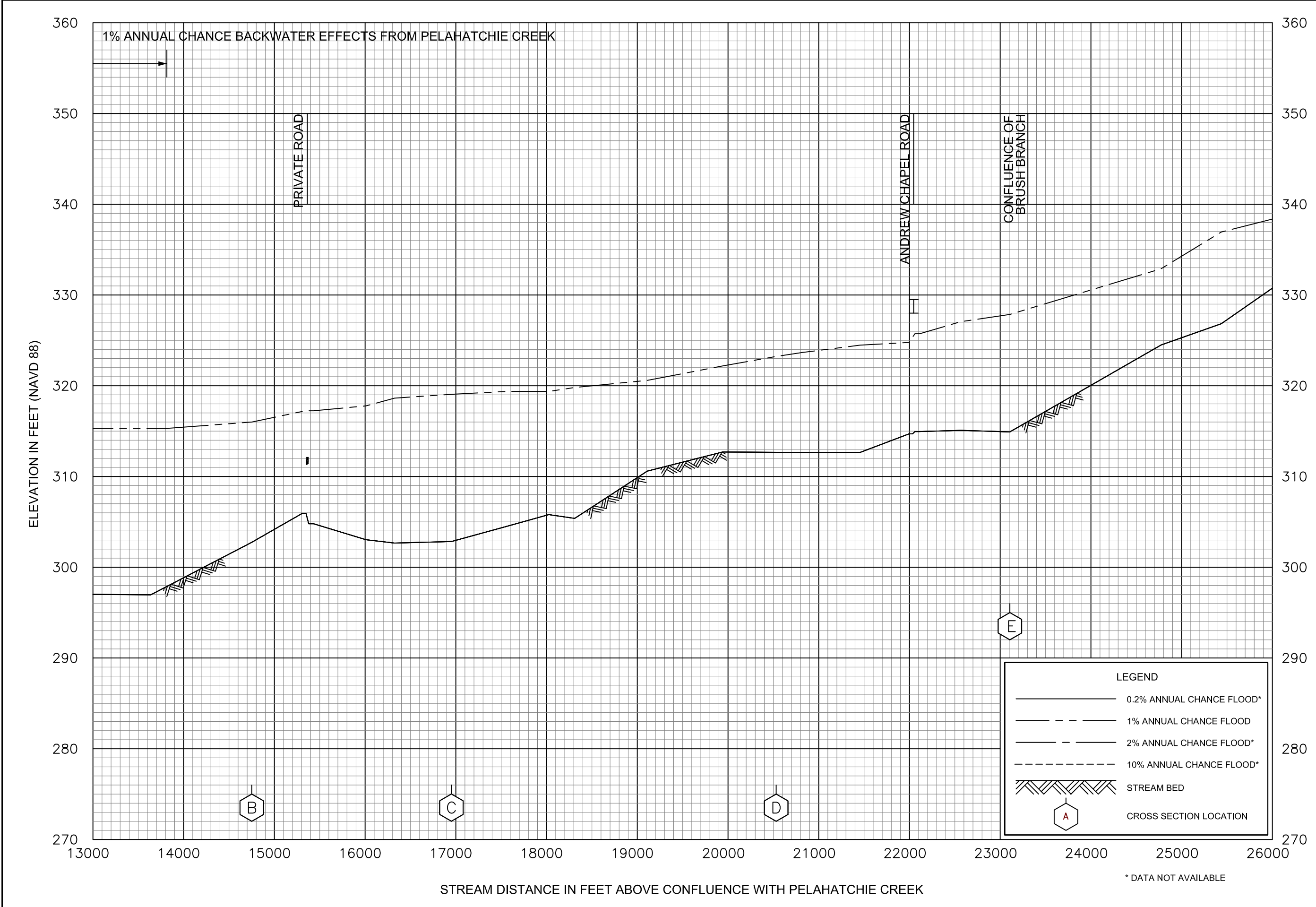
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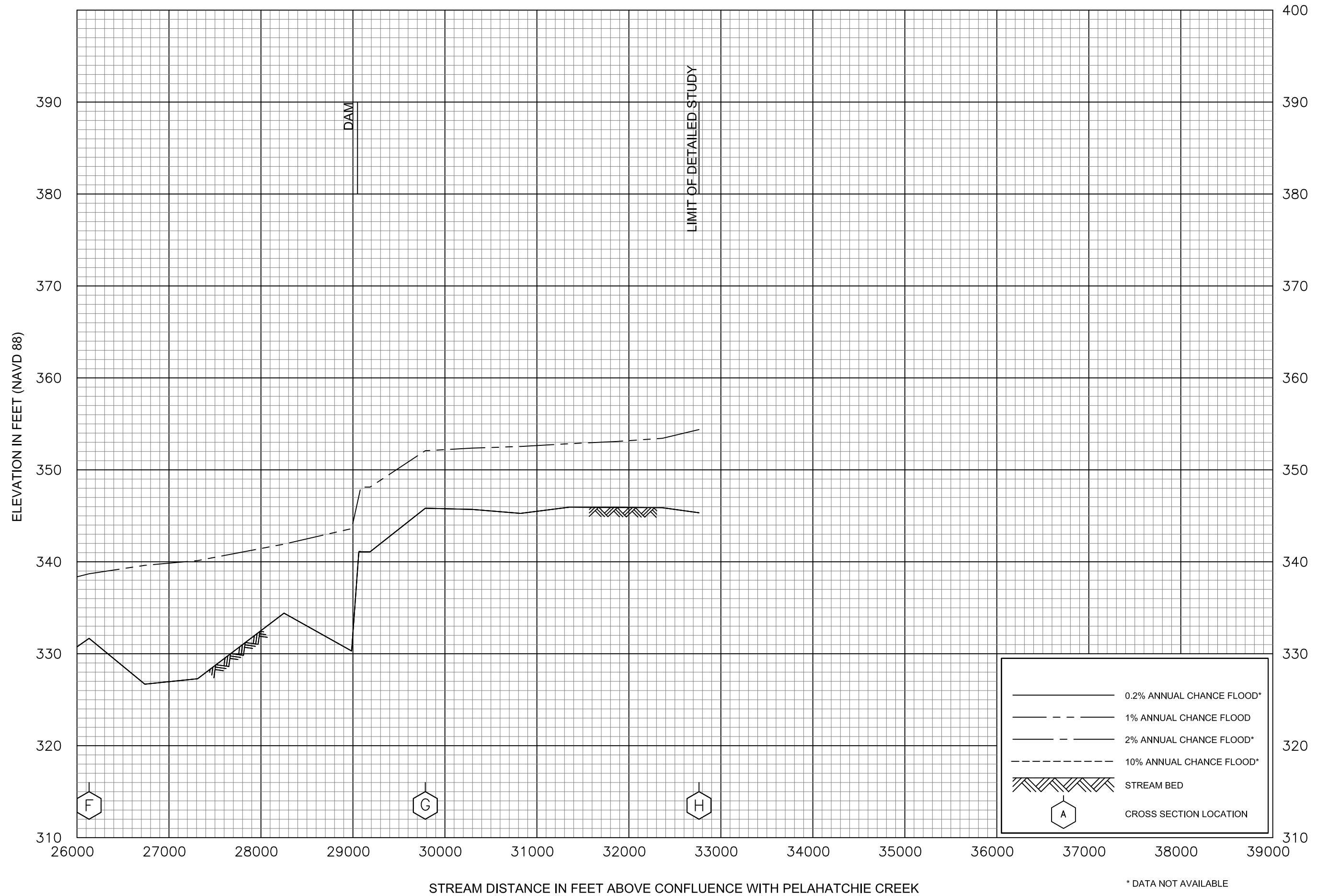
AND INCORPORATED AREAS

FLOOD PROFILES

BRUSH CREEK

02P





FEDERAL EMERGENCY MANAGEMENT AGENCY

RANKIN COUNTY, MS

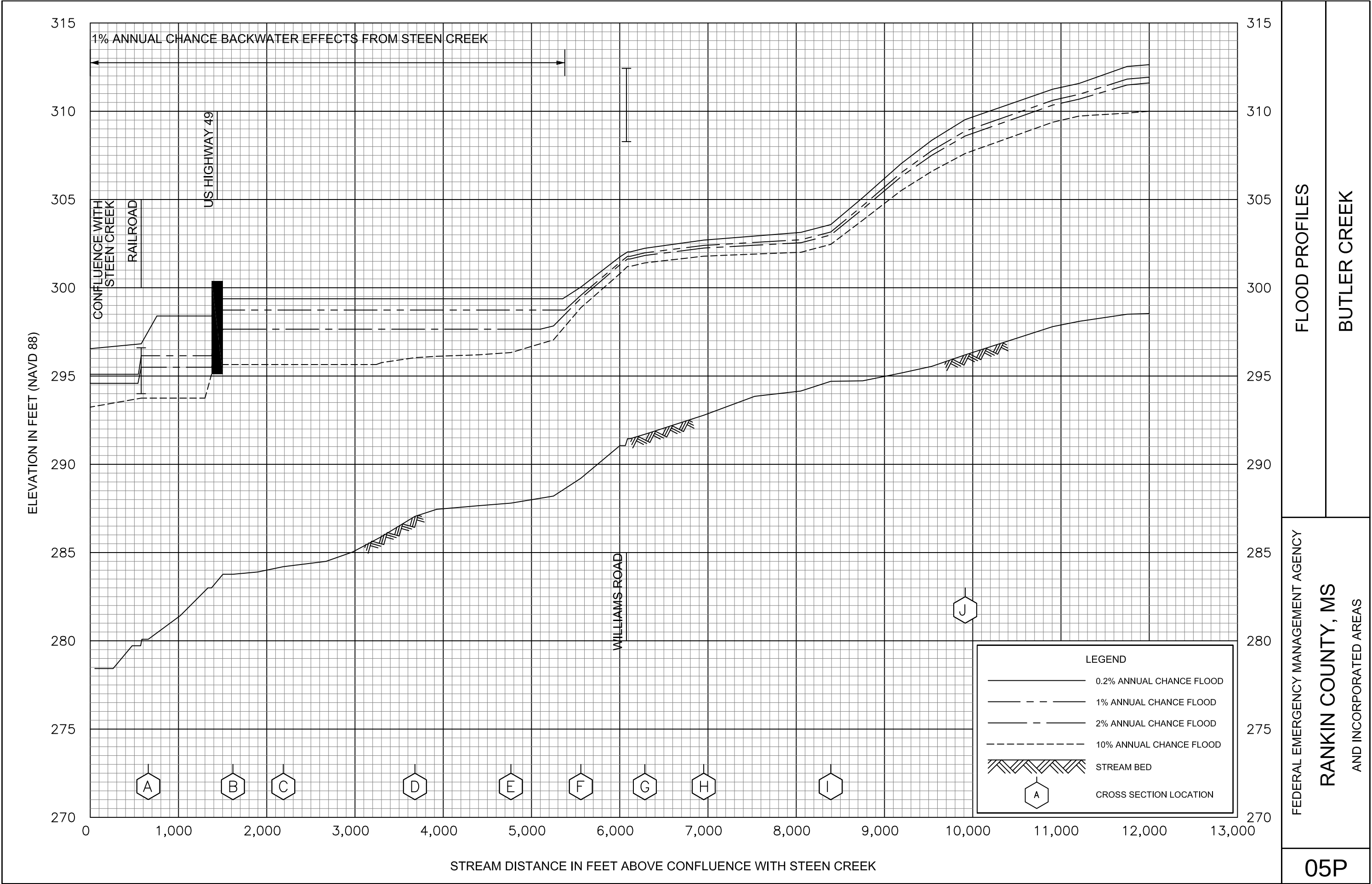
AND INCORPORATED AREAS

FLOOD PROFILES

BRUSH CREEK

04P

* DATA NOT AVAILABLE



FLOOD PROFILES

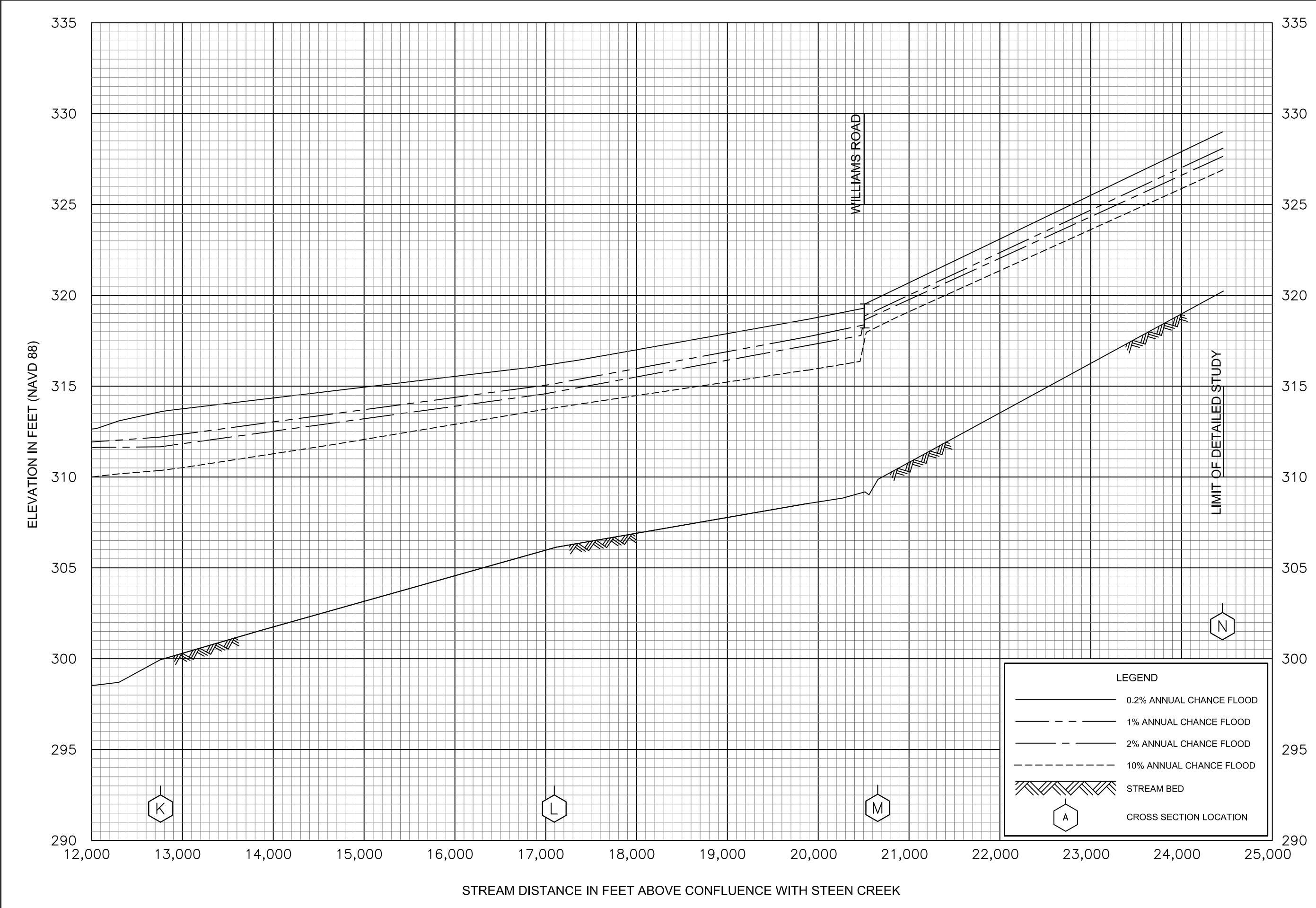
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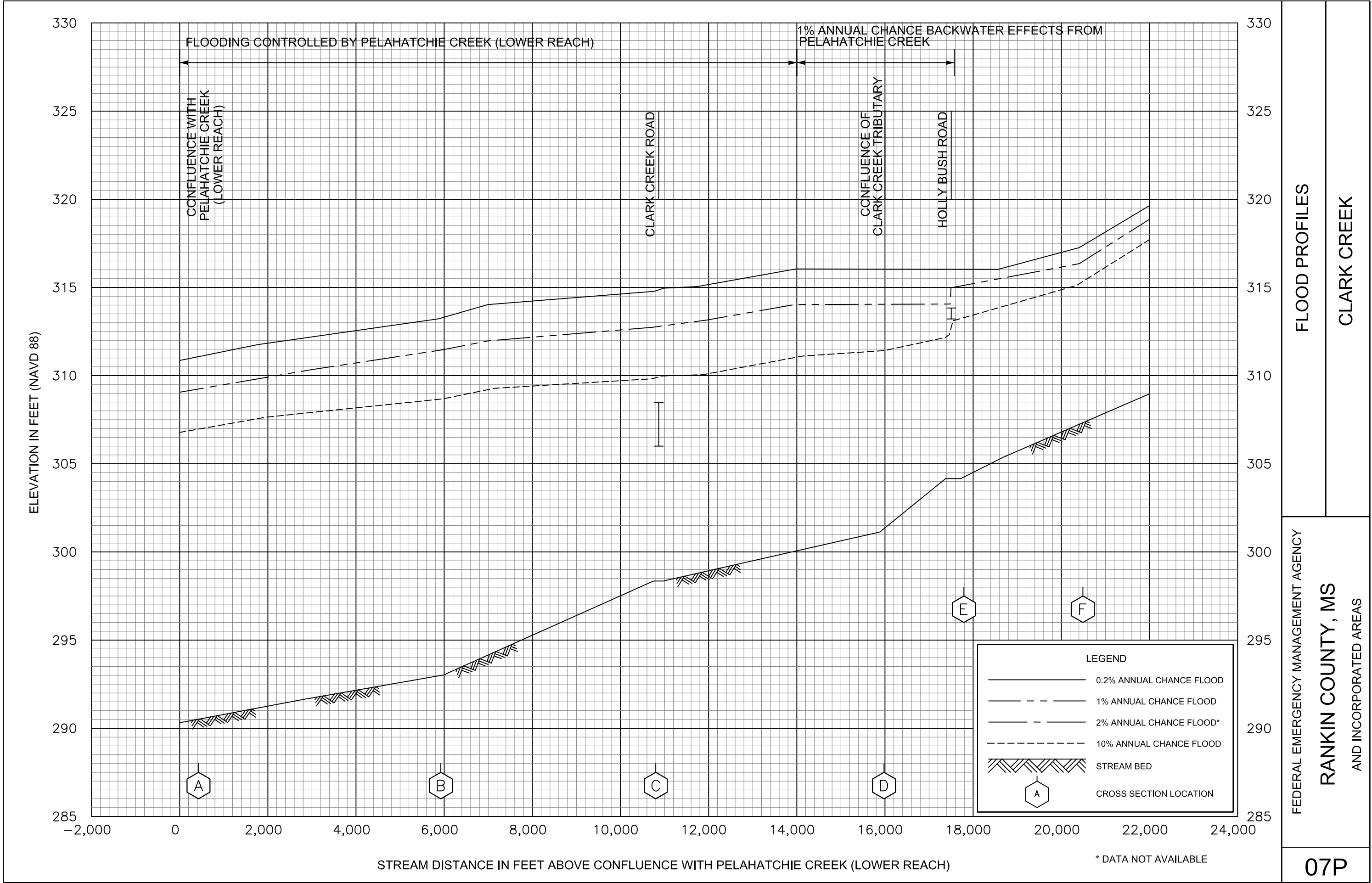
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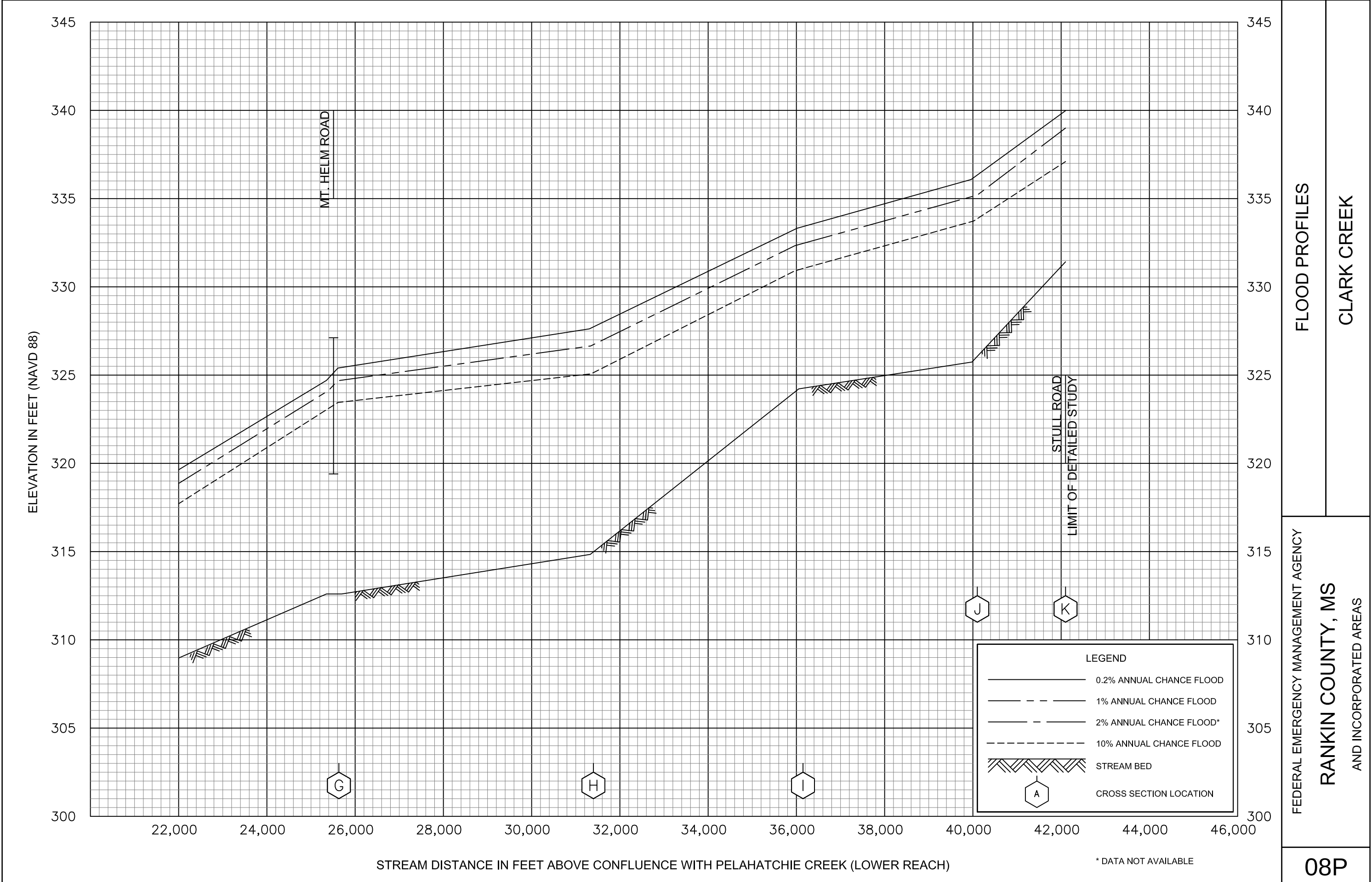
RANKIN COUNTY, MS

AND INCORPORATED AREAS

05P







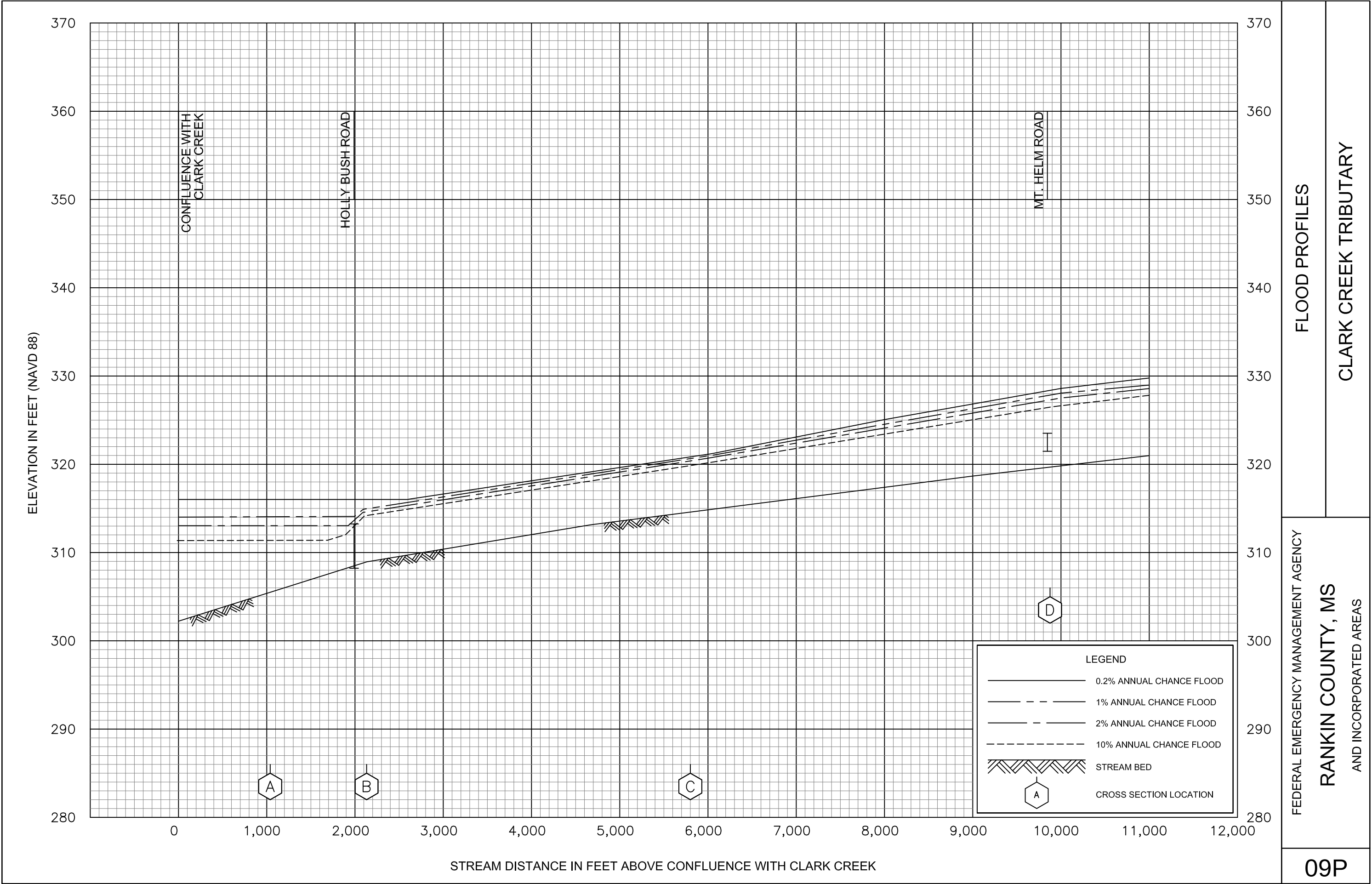
FLOOD PROFILES

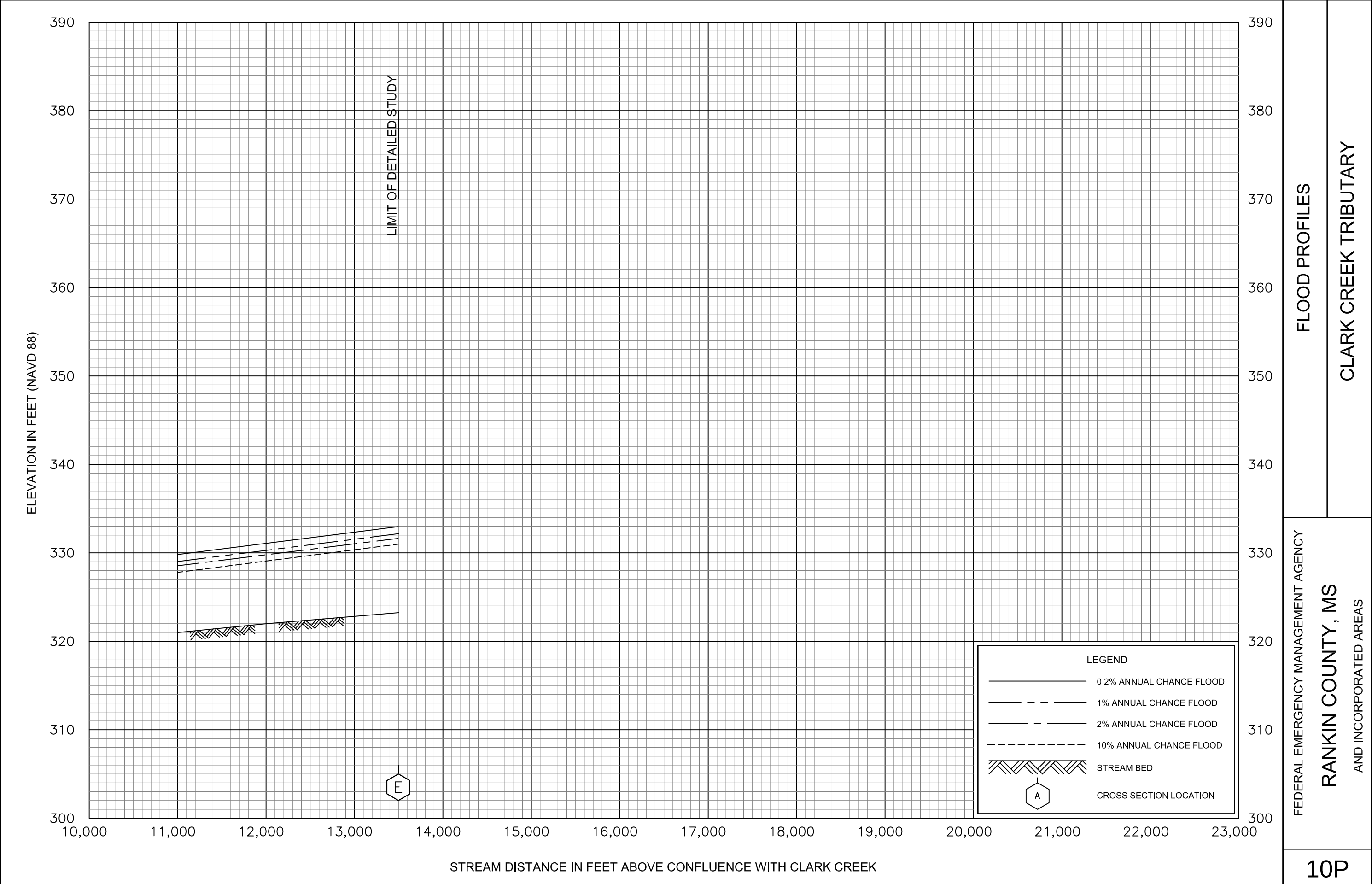
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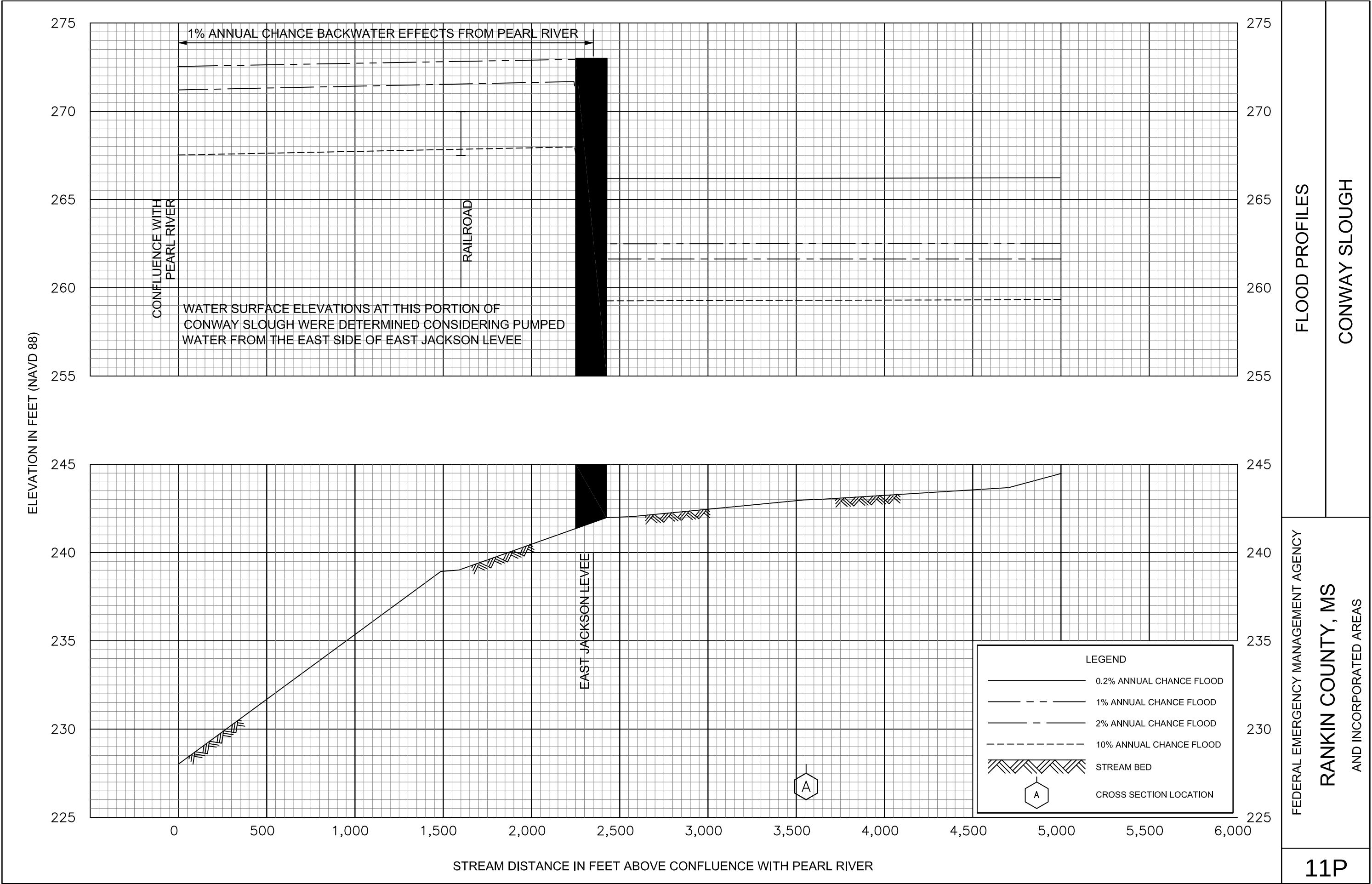
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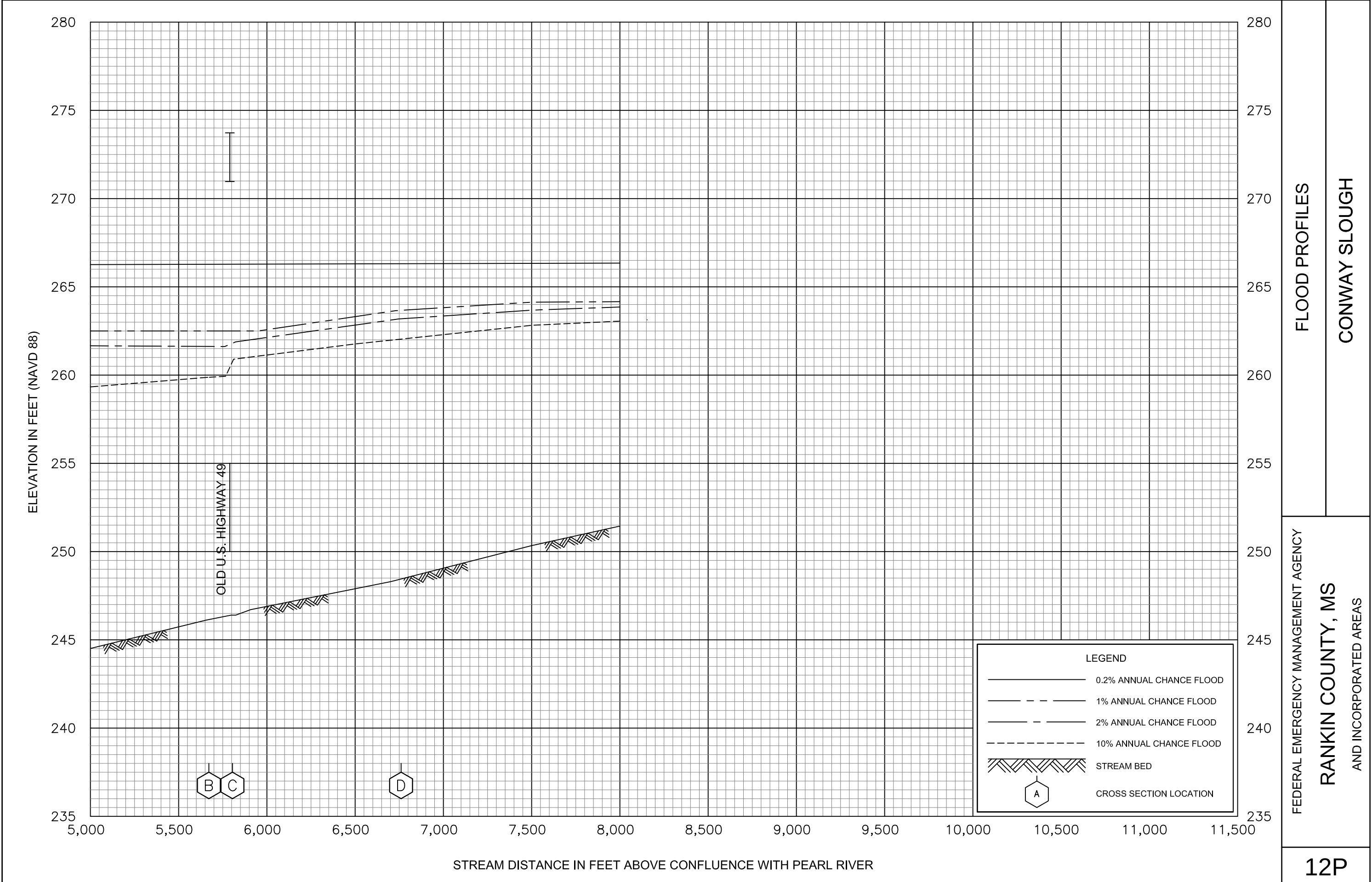
RANKIN COUNTY, MS
AND INCORPORATED AREAS

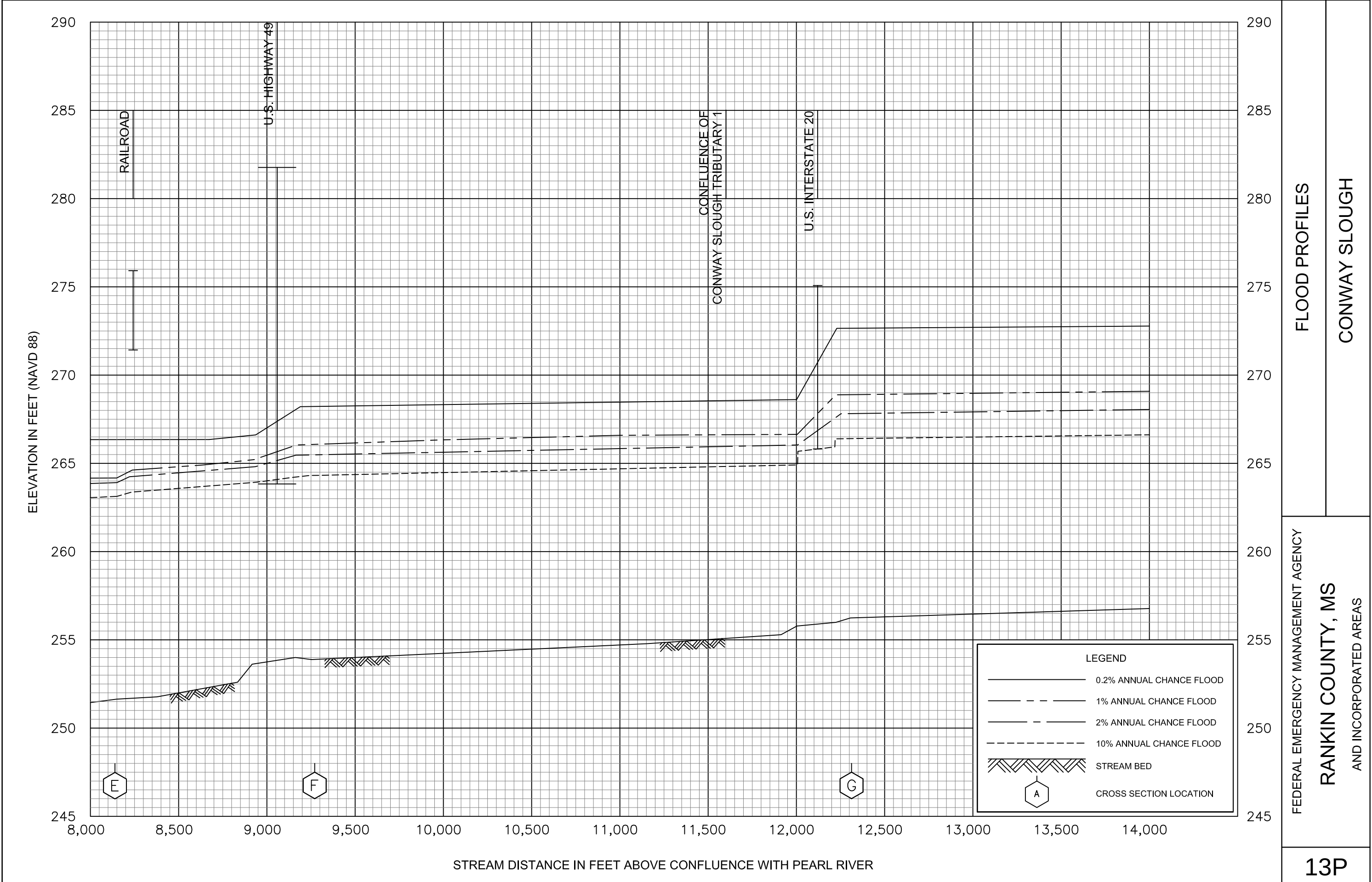
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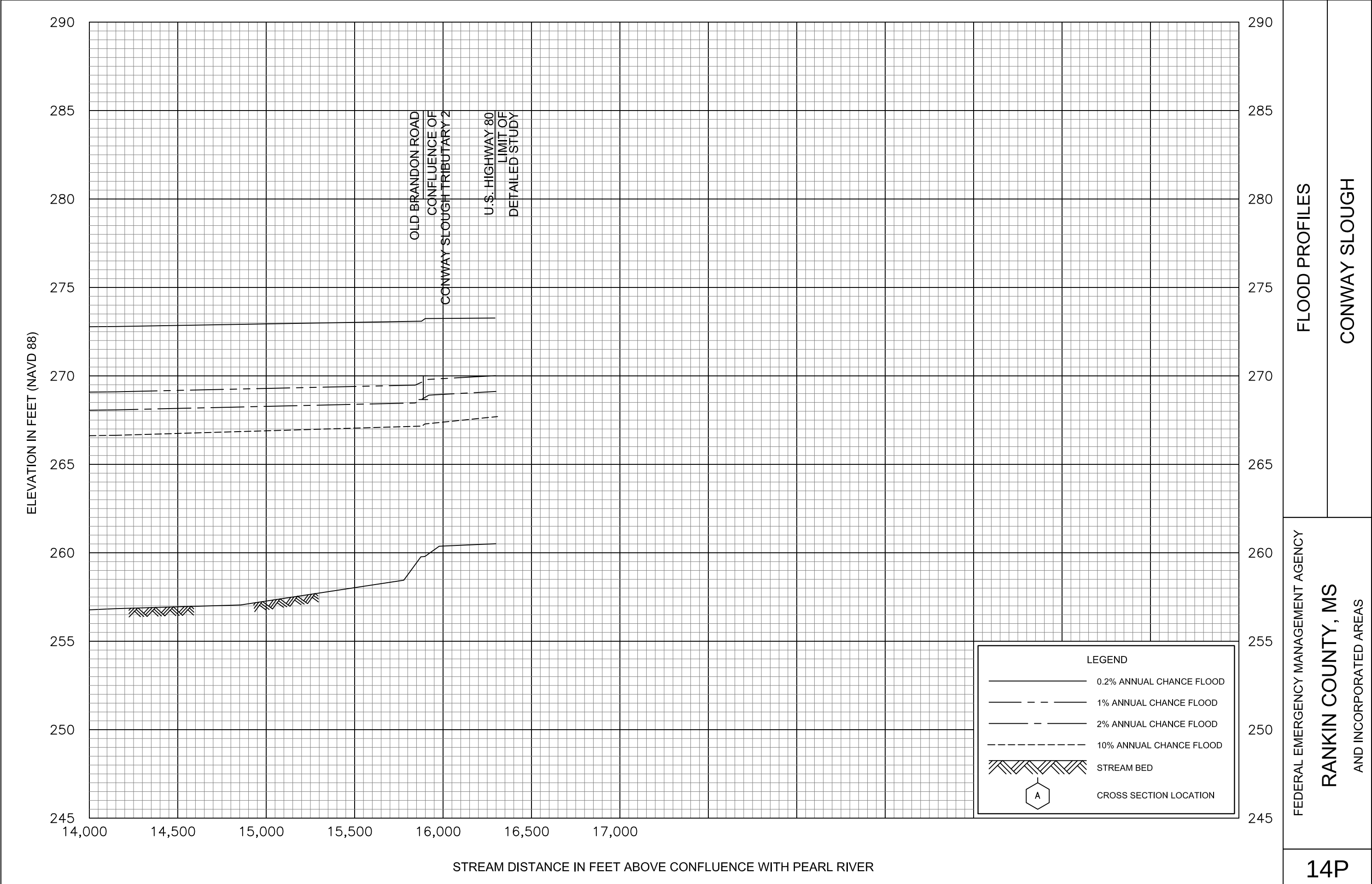


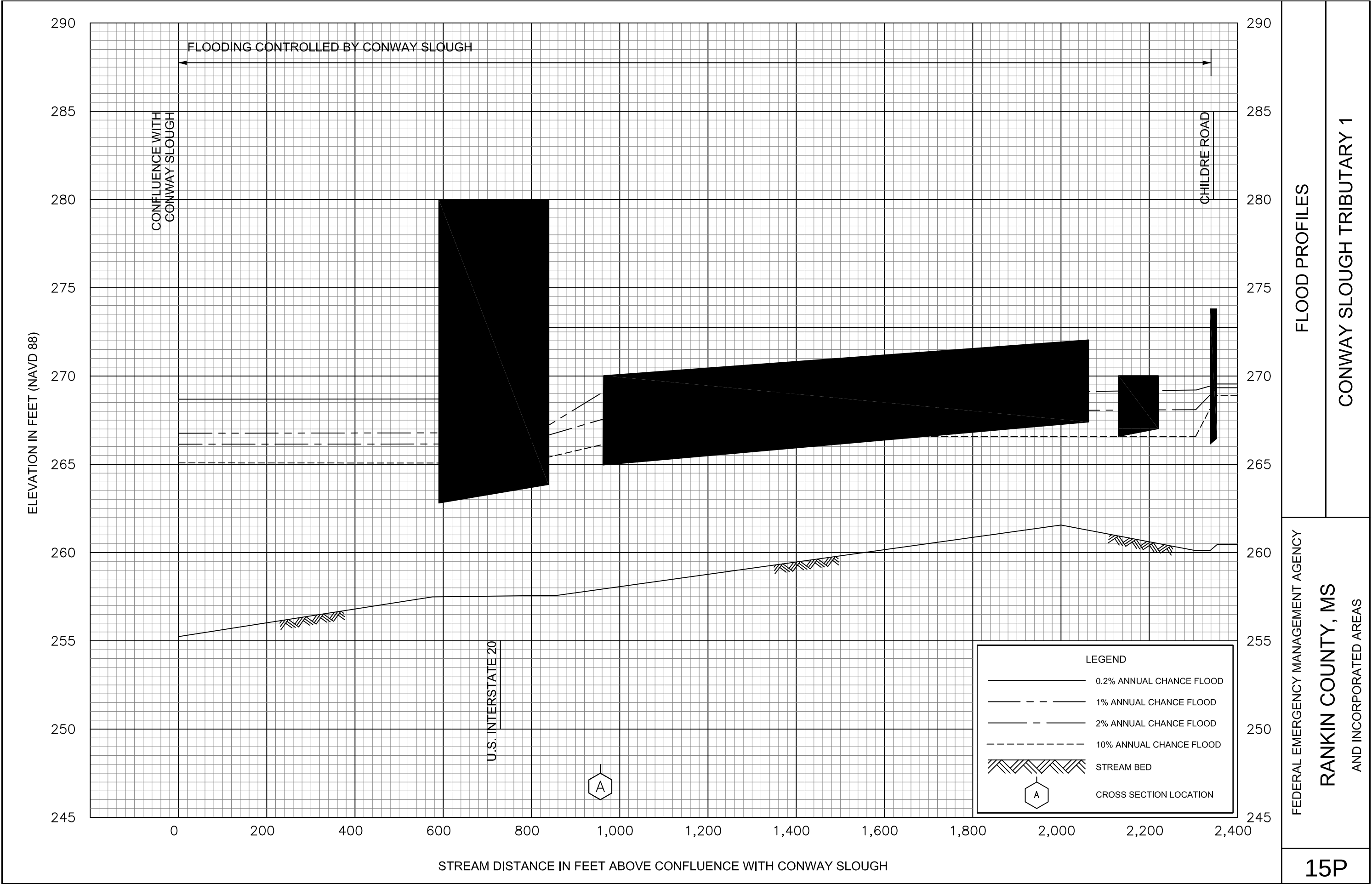


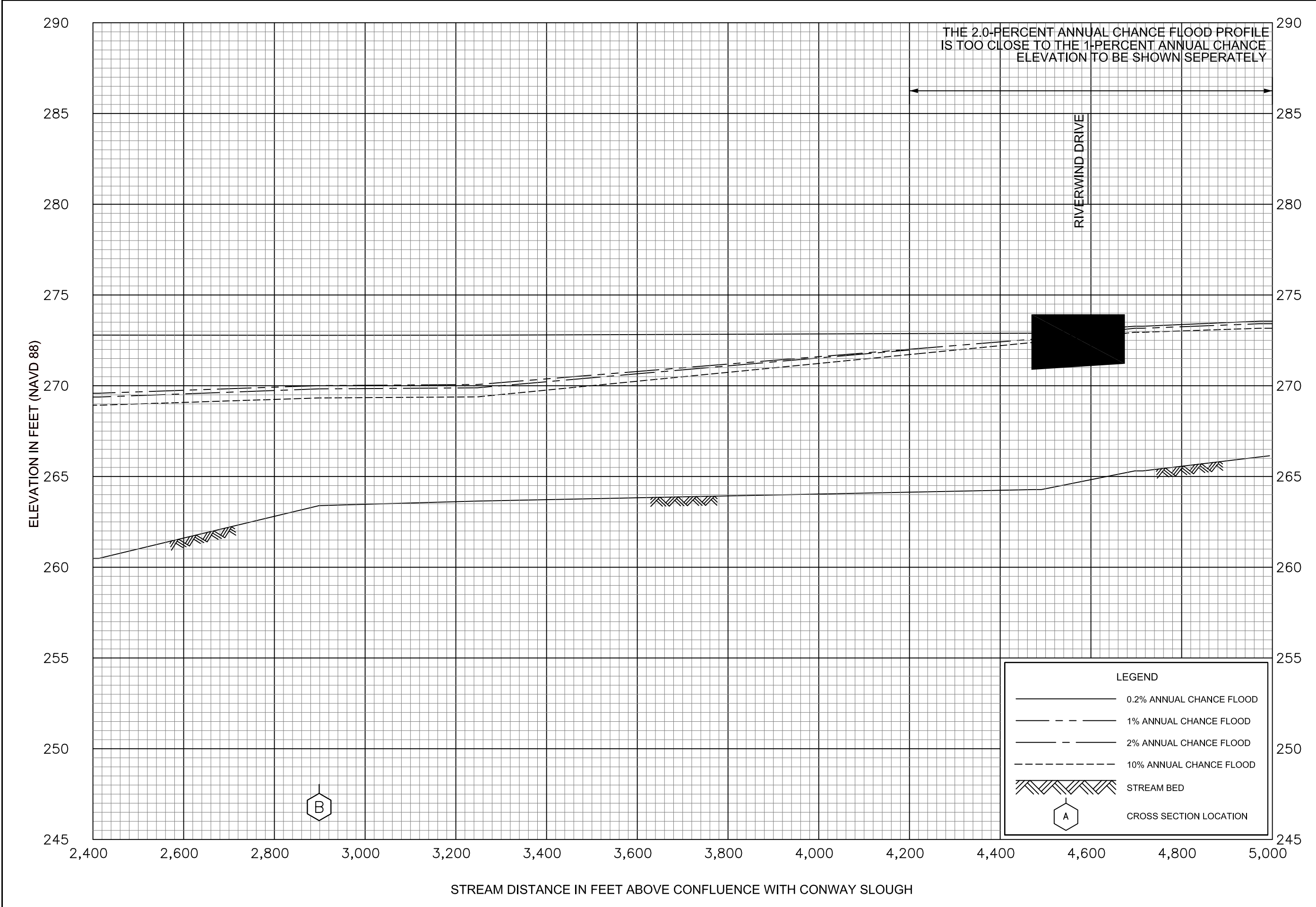


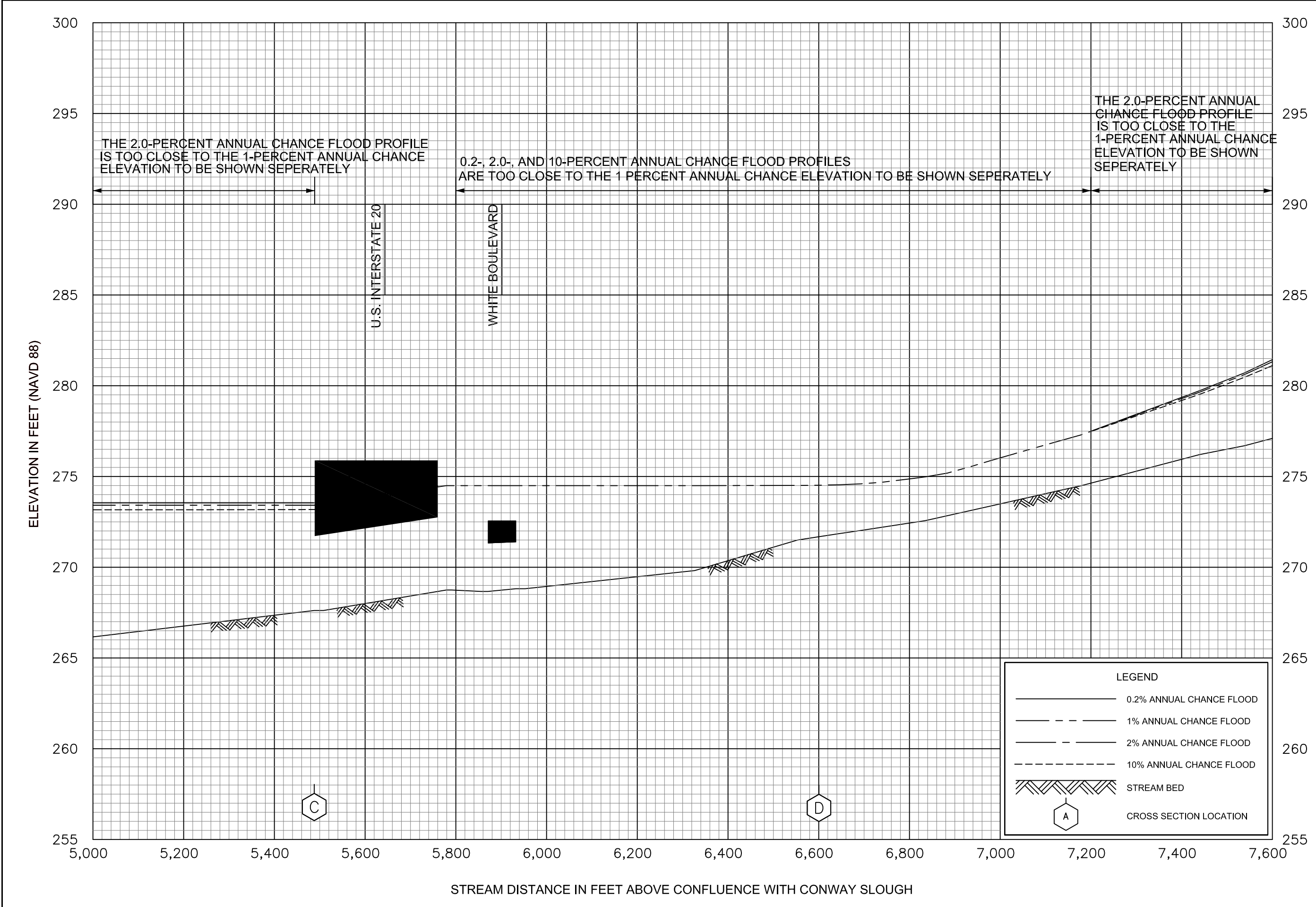


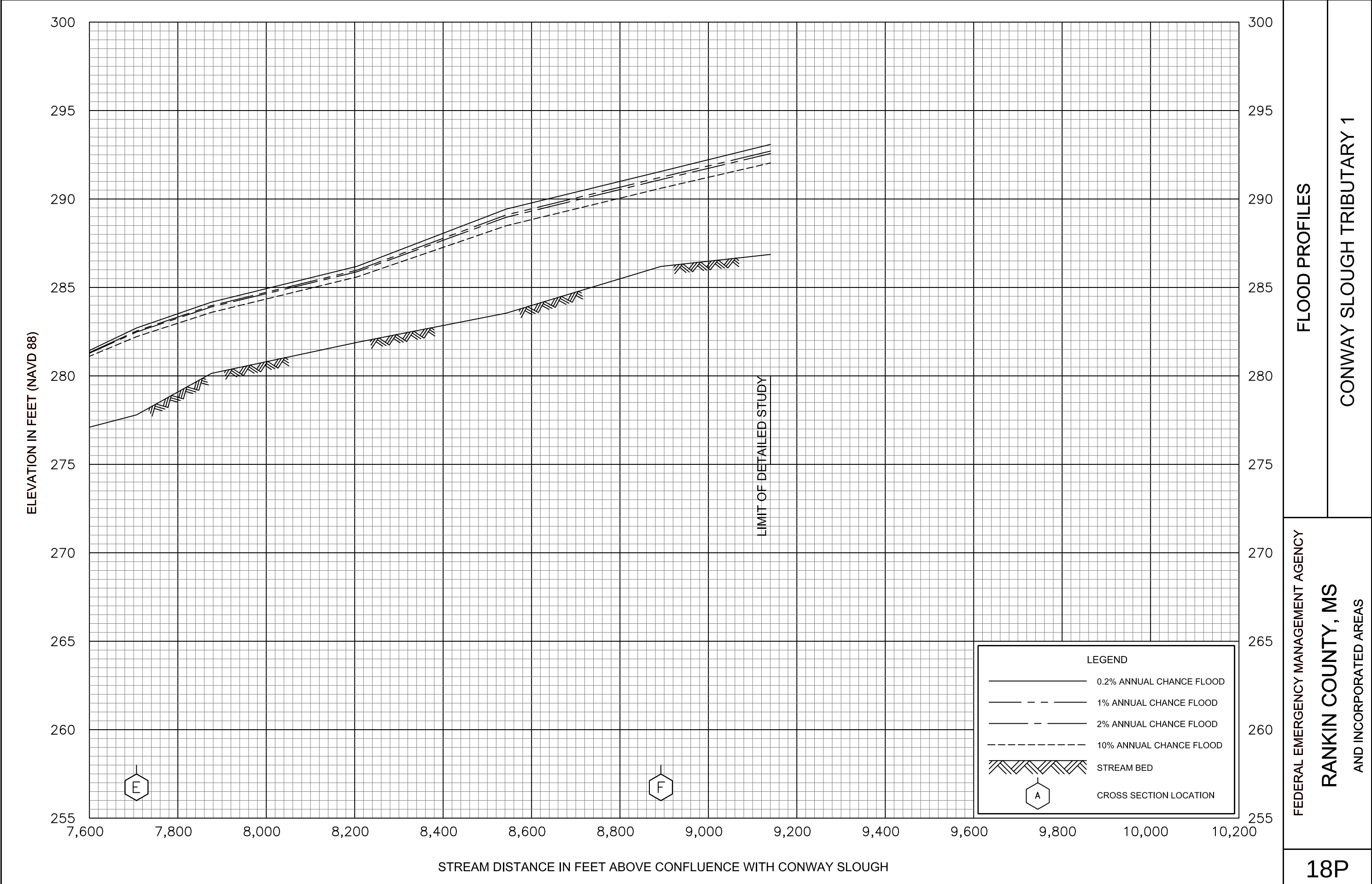


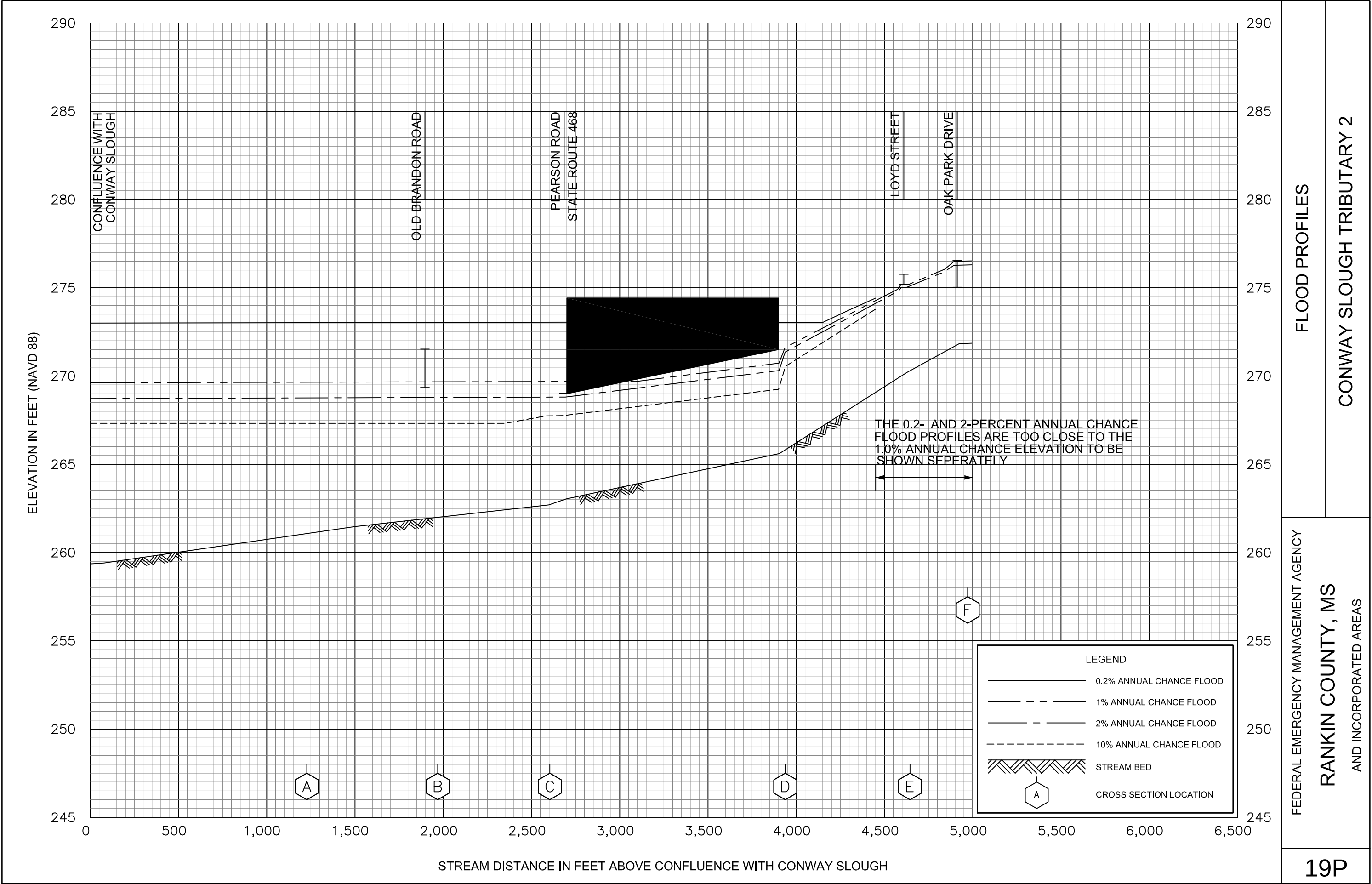


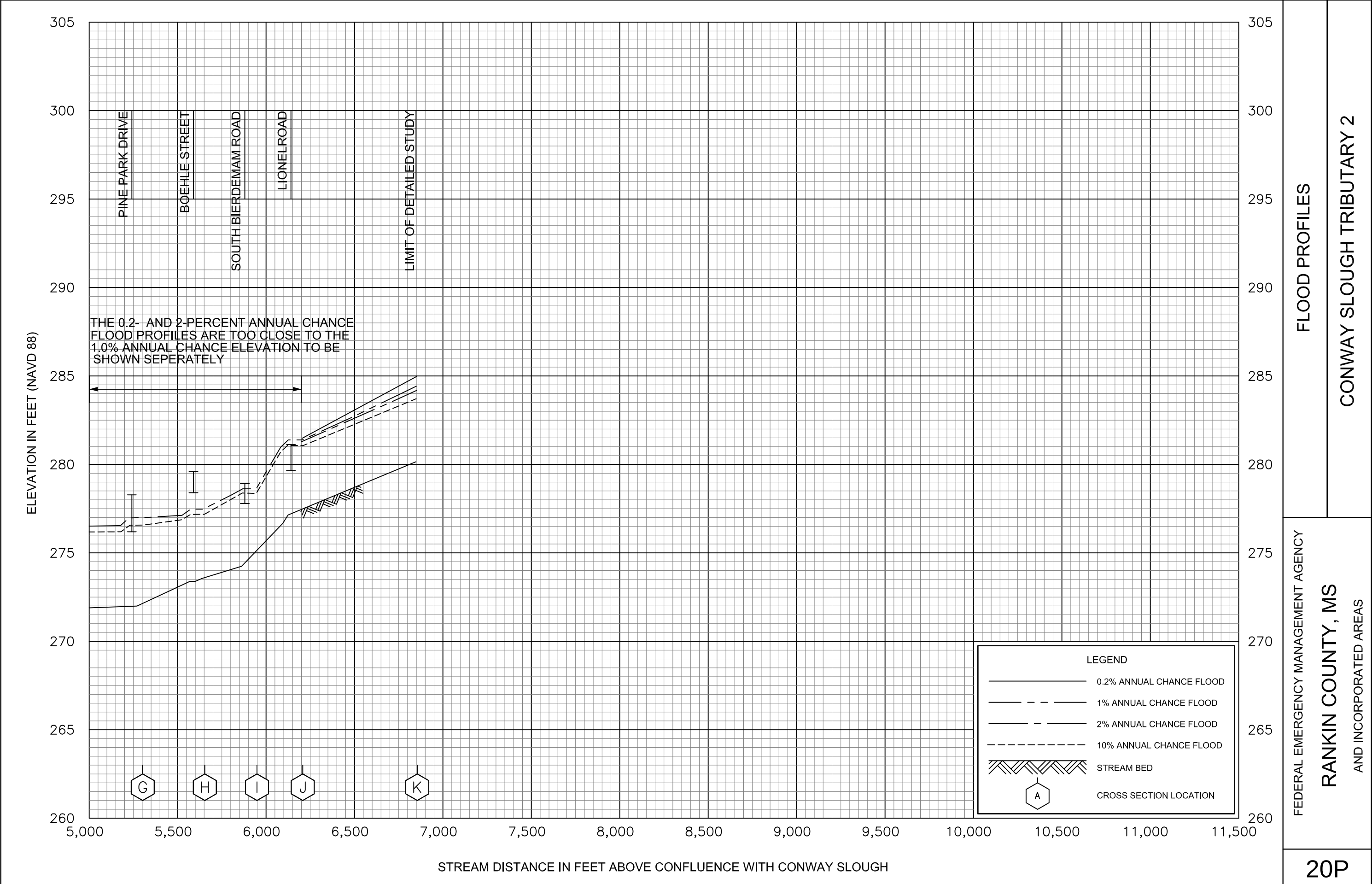


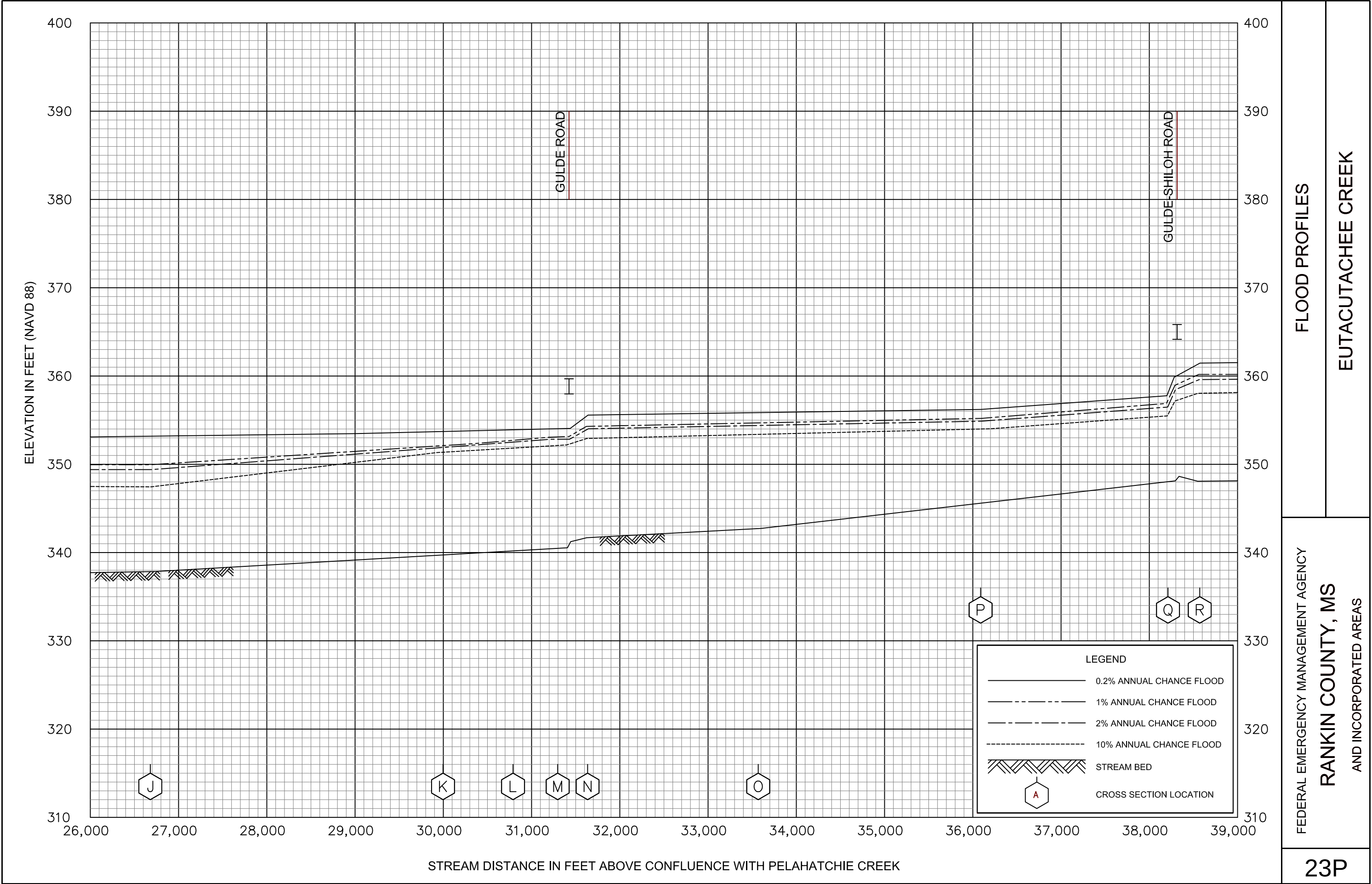


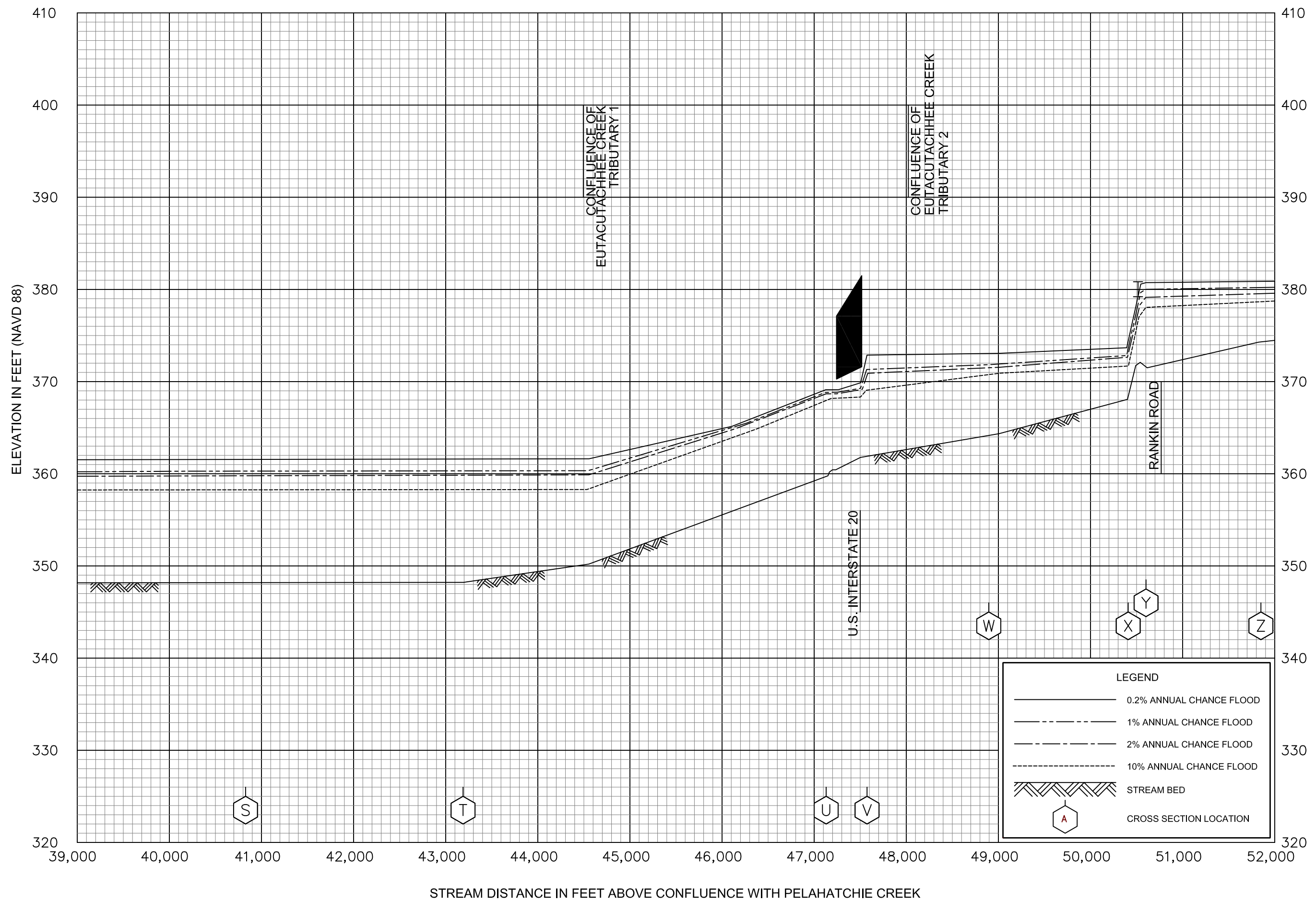


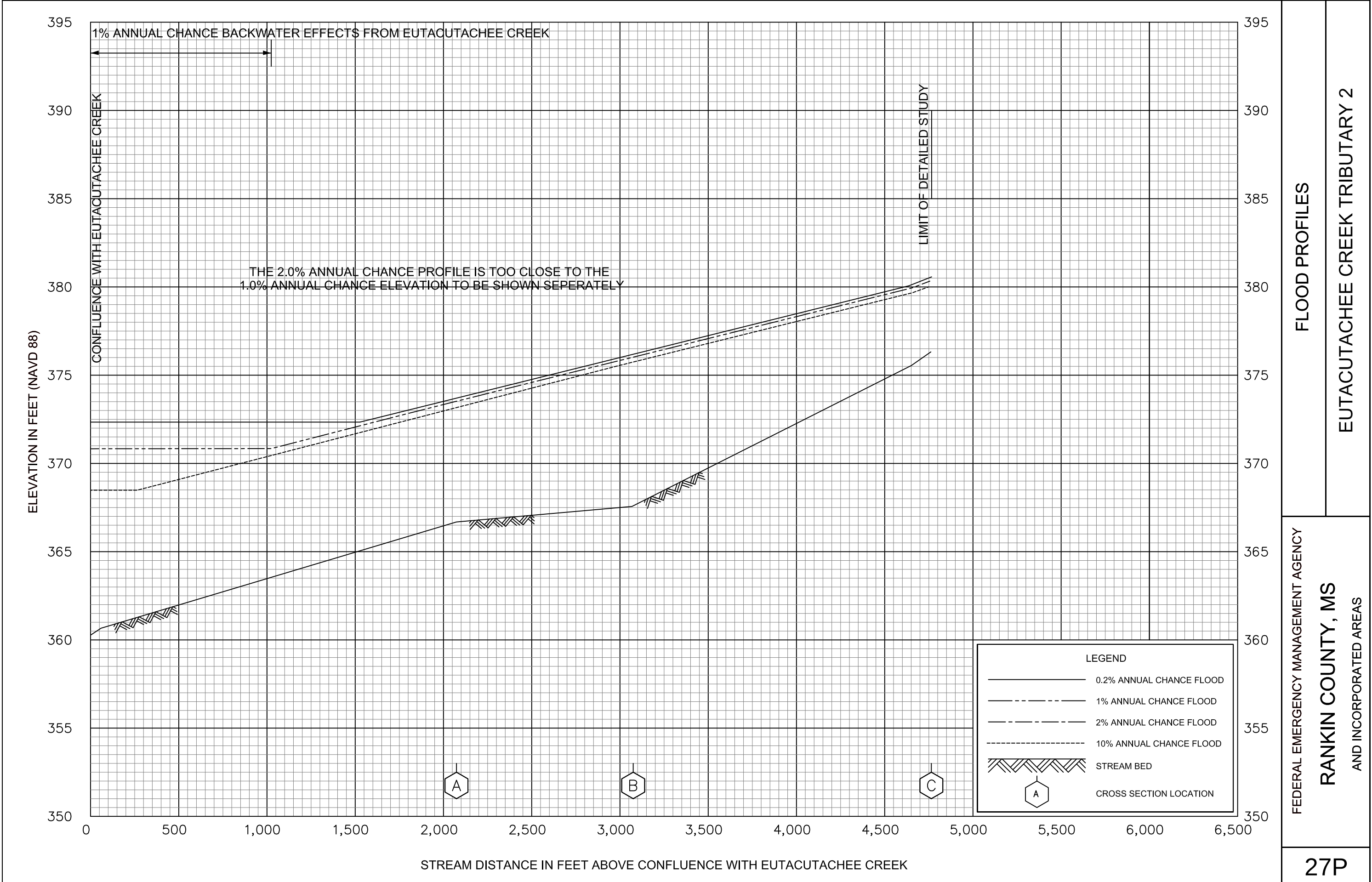


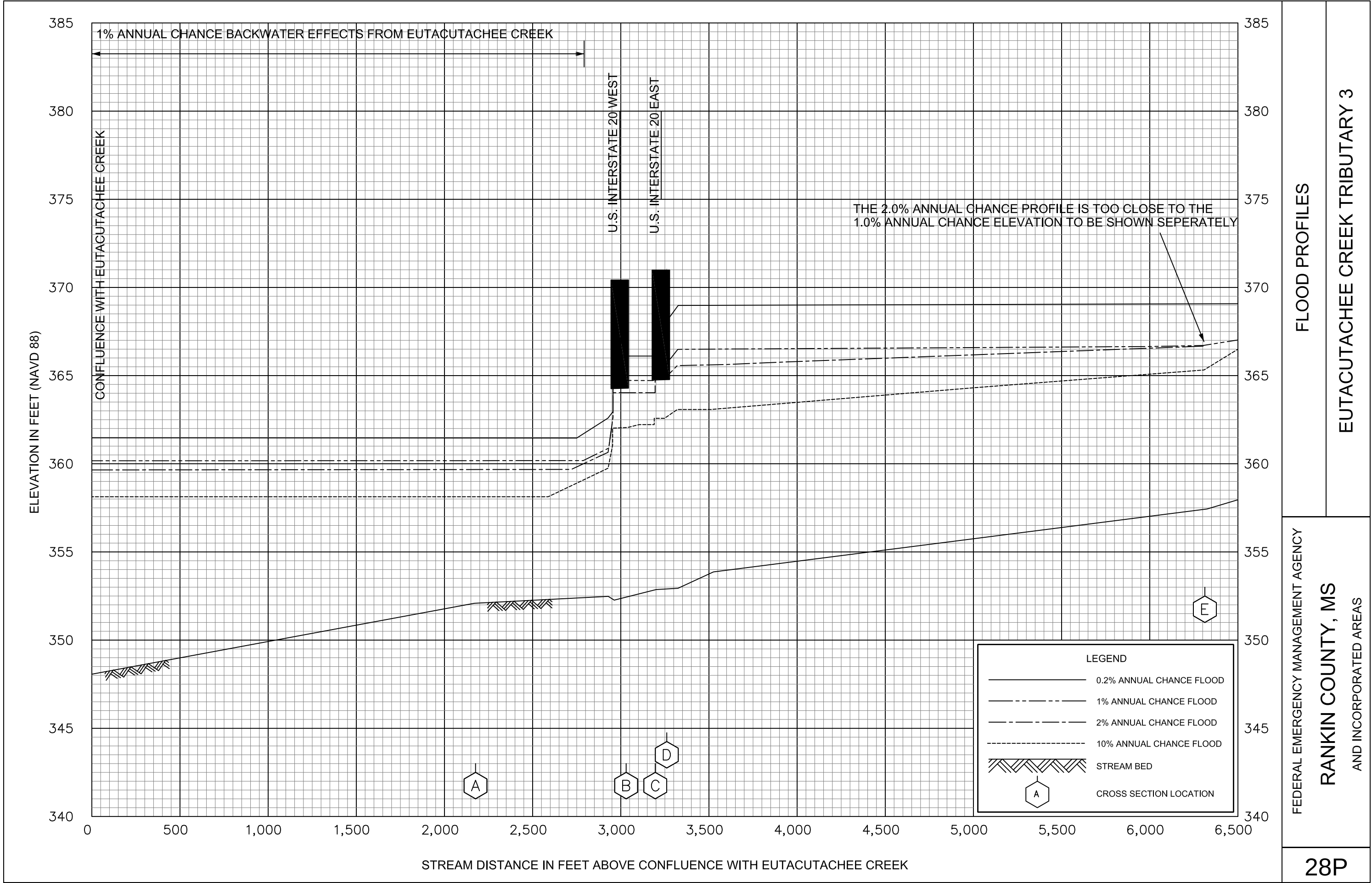






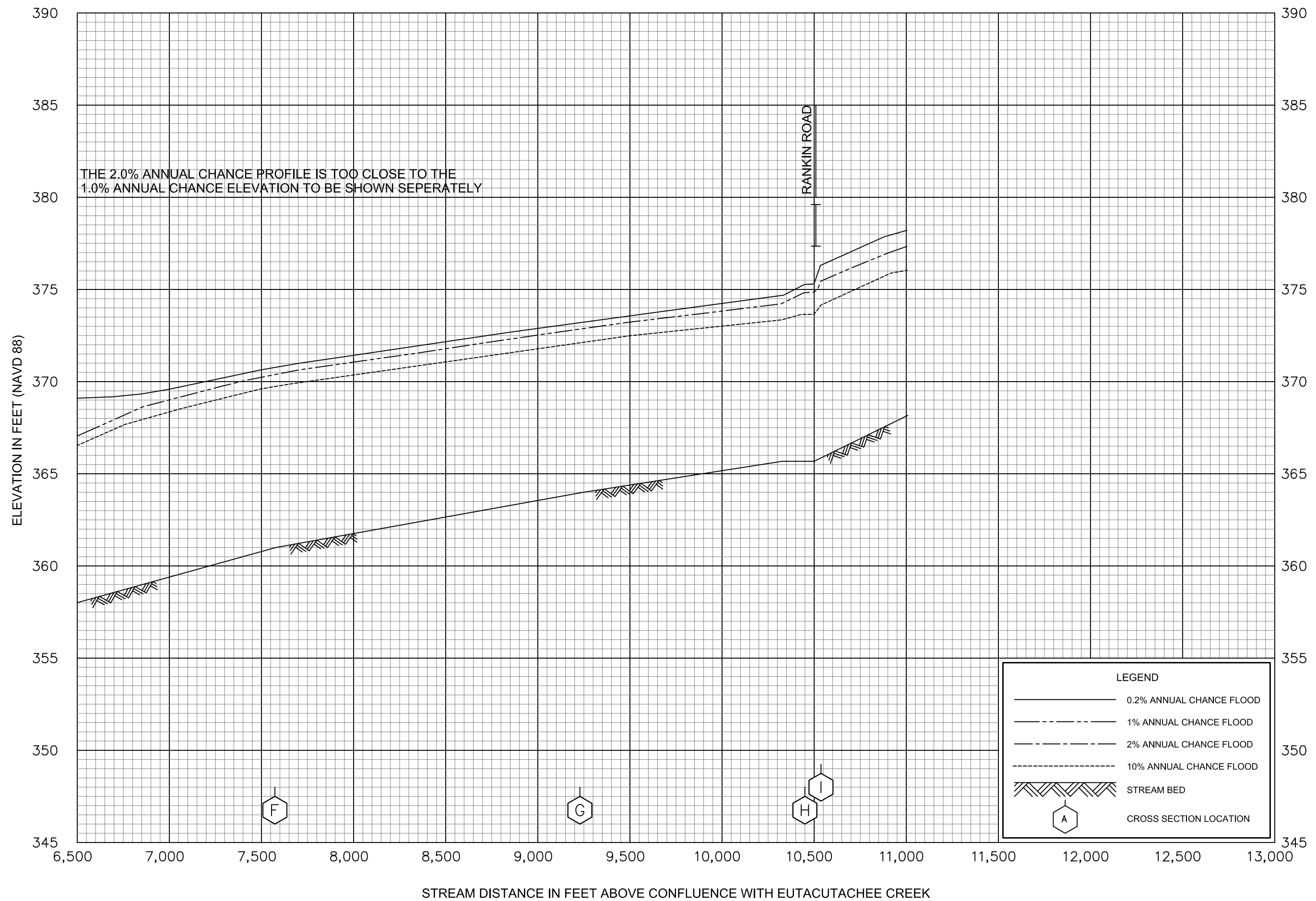


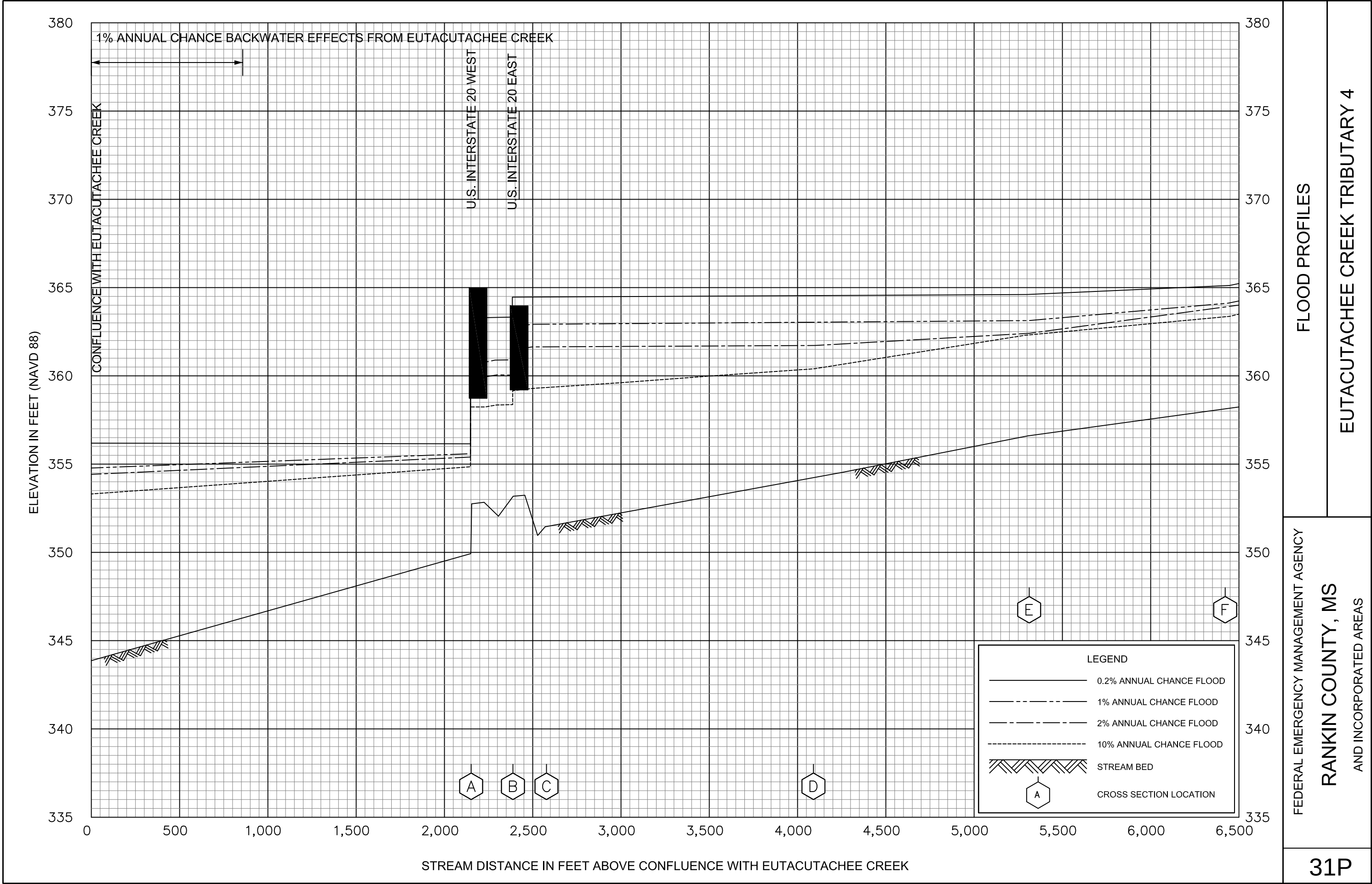


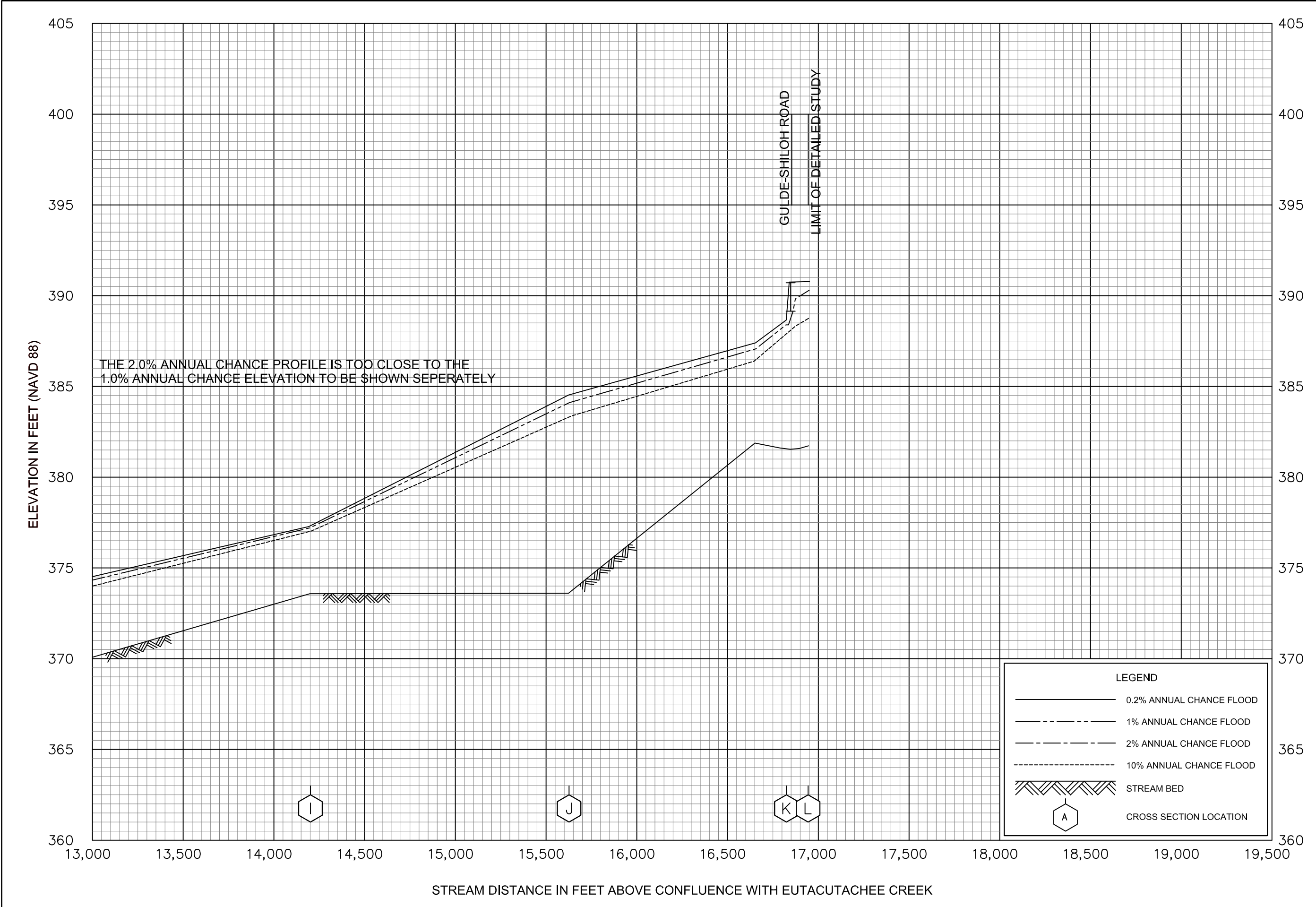


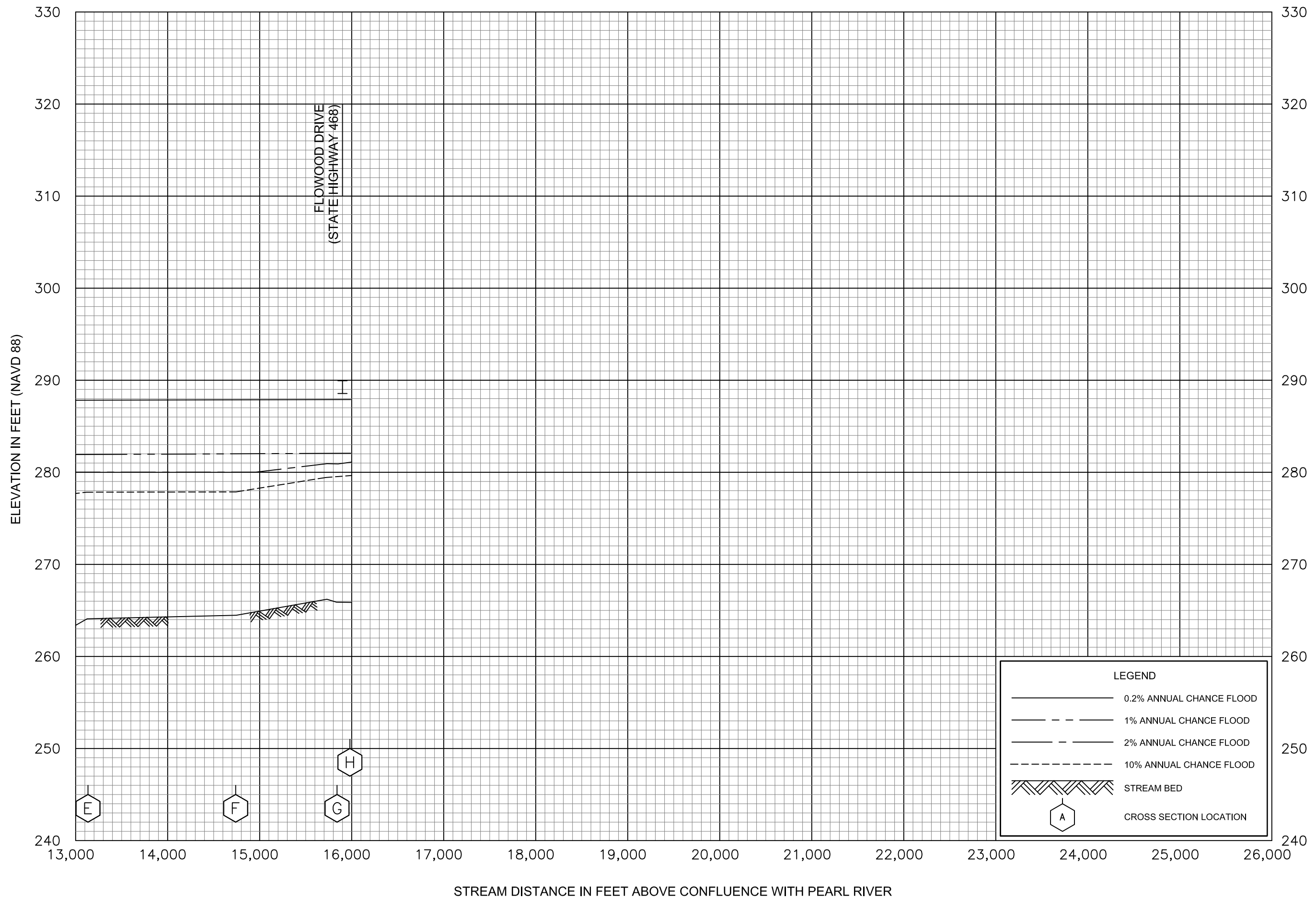
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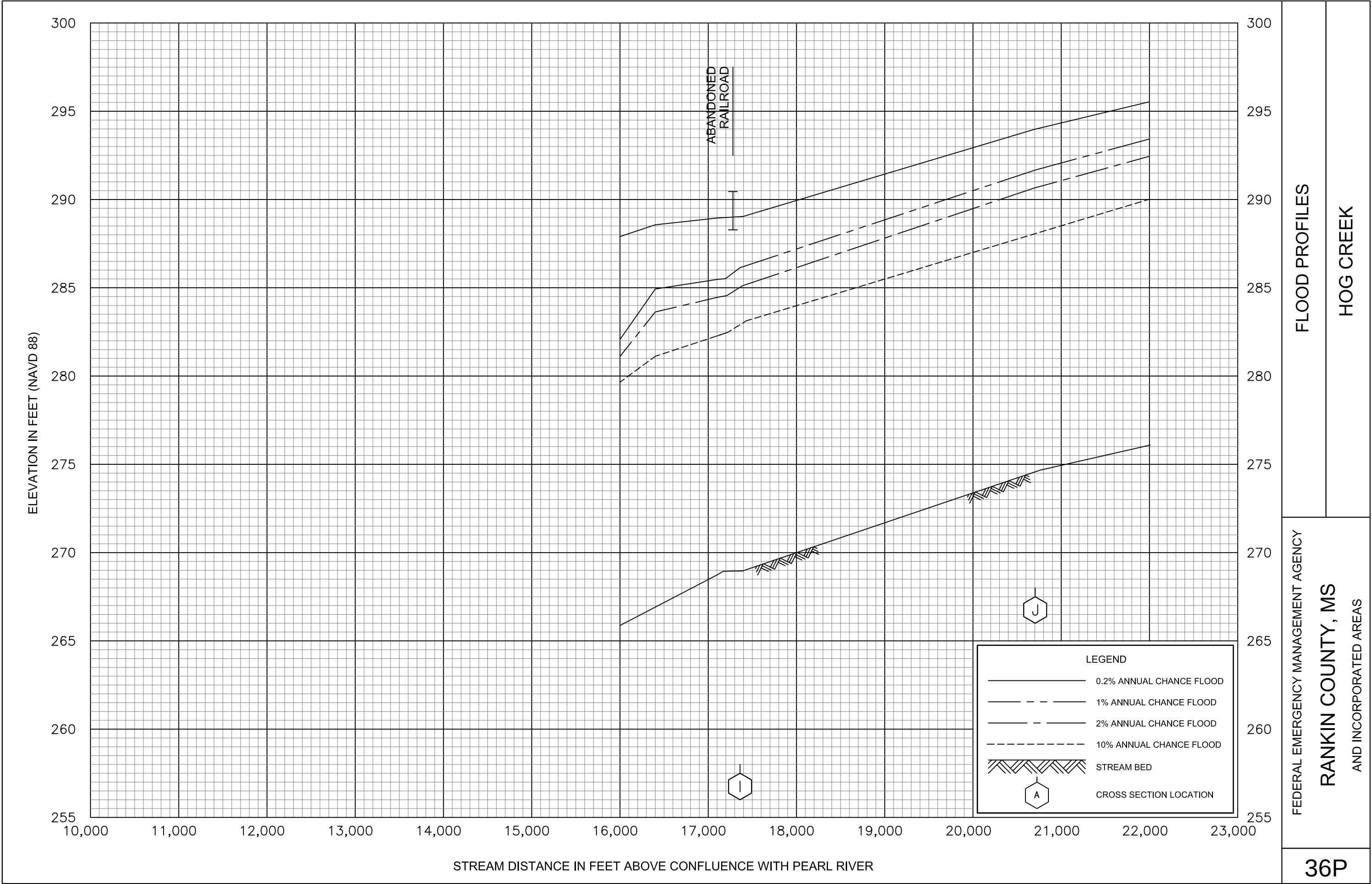
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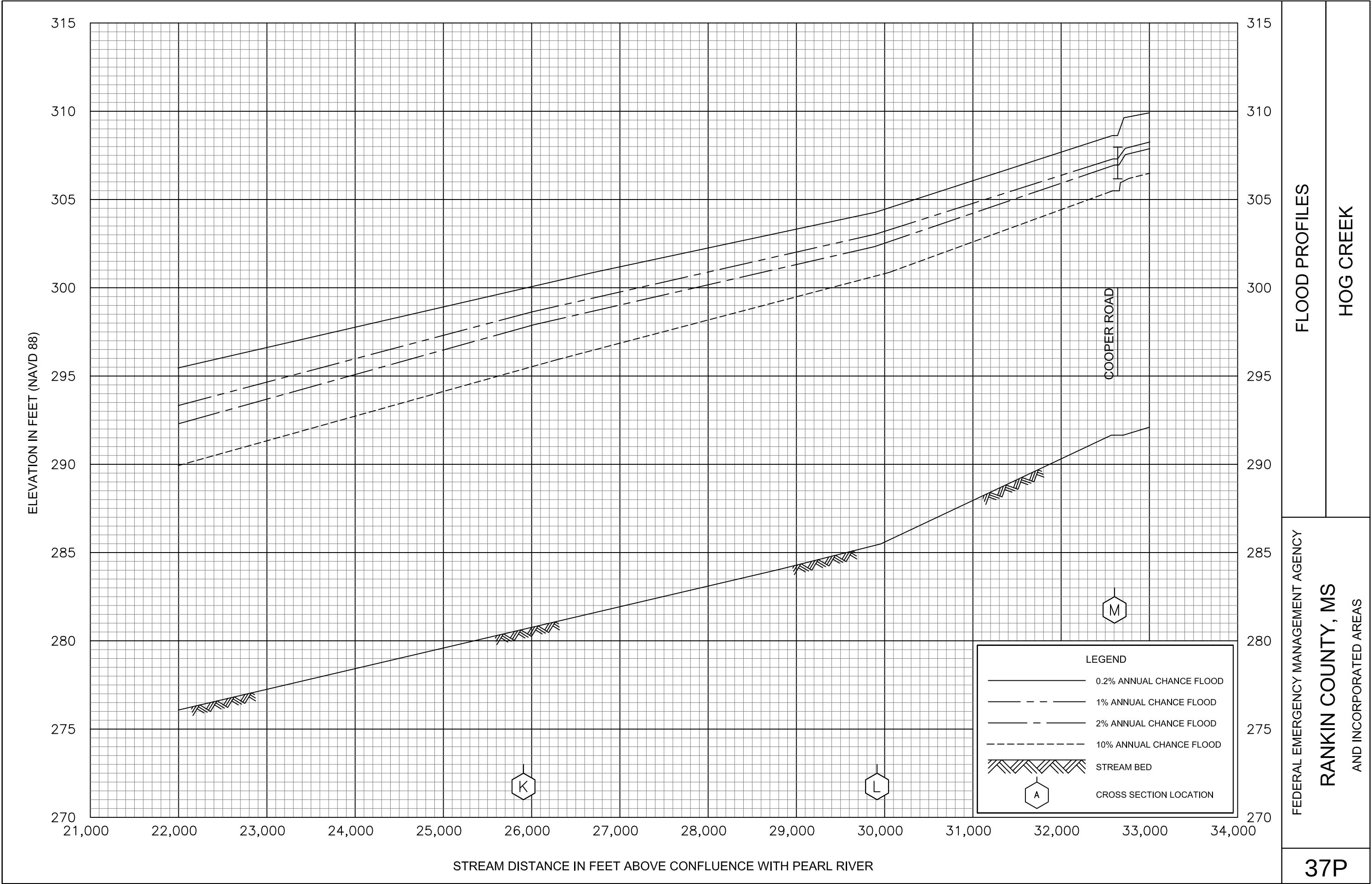












FLOOD PROFILES

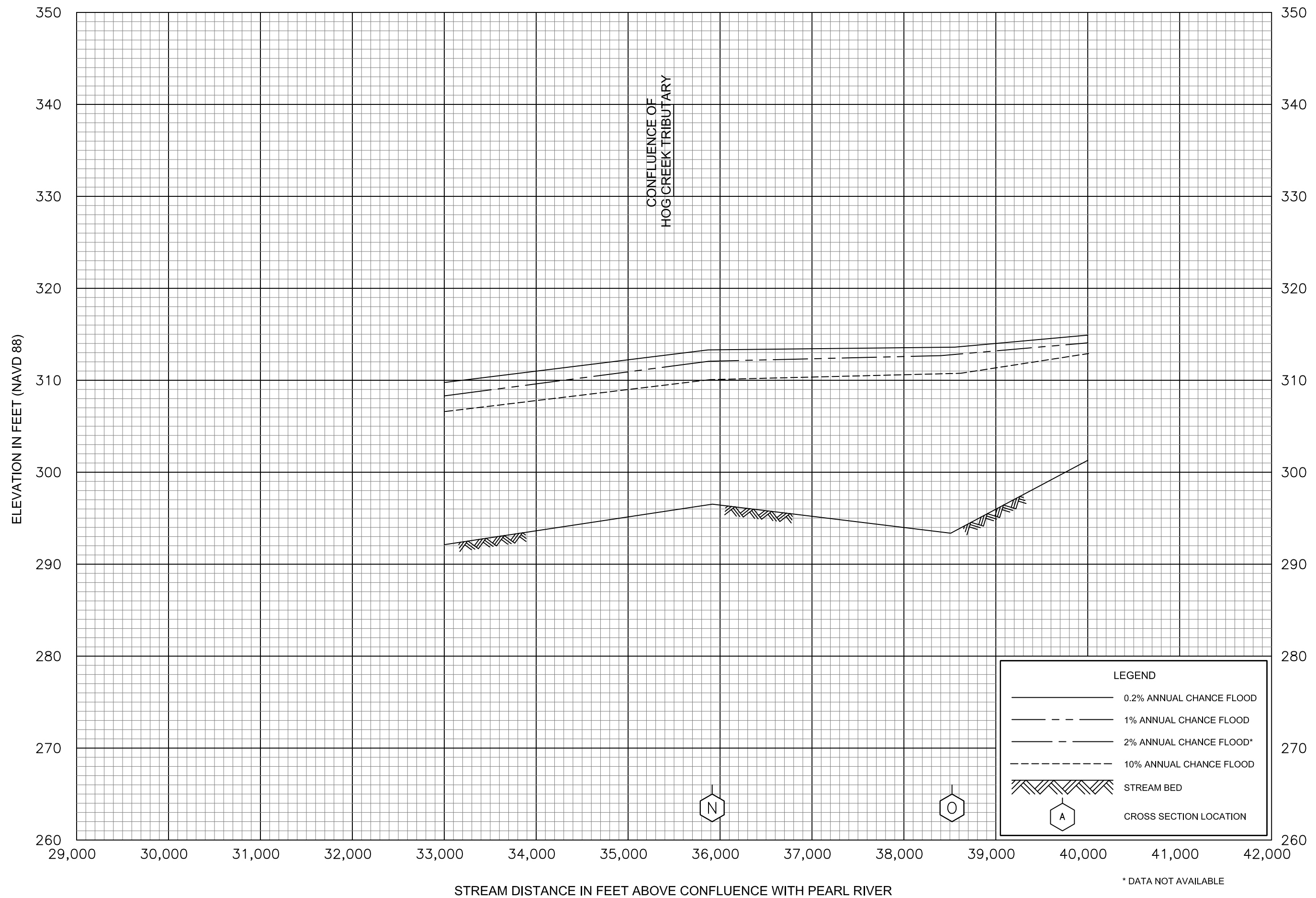
HOG CREEK

FEDERAL EMERGENCY MANAGEMENT AGENCY

RANKIN COUNTY, MS

AND INCORPORATED AREAS

37P



FEDERAL EMERGENCY MANAGEMENT AGENCY

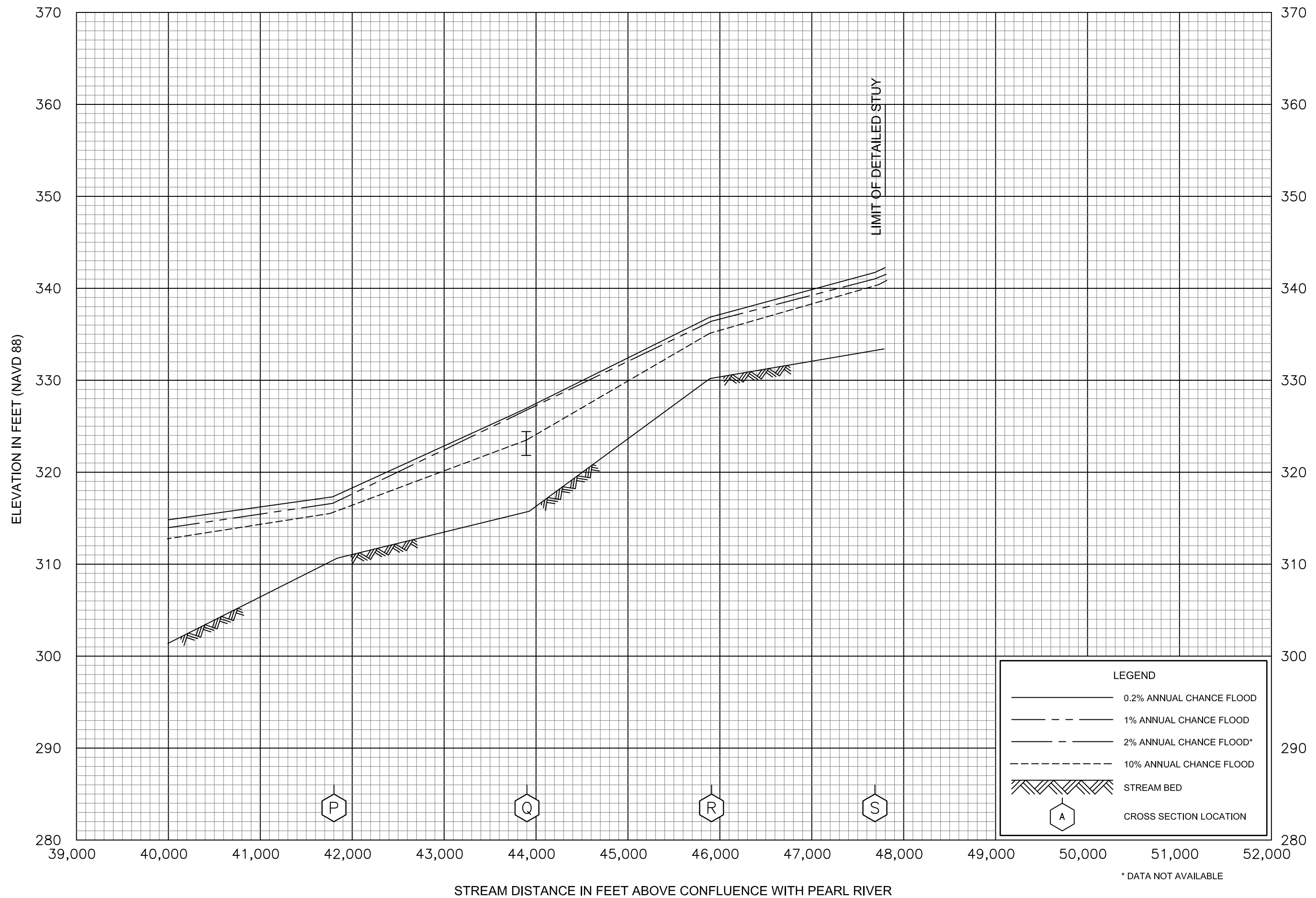
RANKIN COUNTY, MS

AND INCORPORATED AREAS

FLOOD PROFILES

HOG CREEK

38P



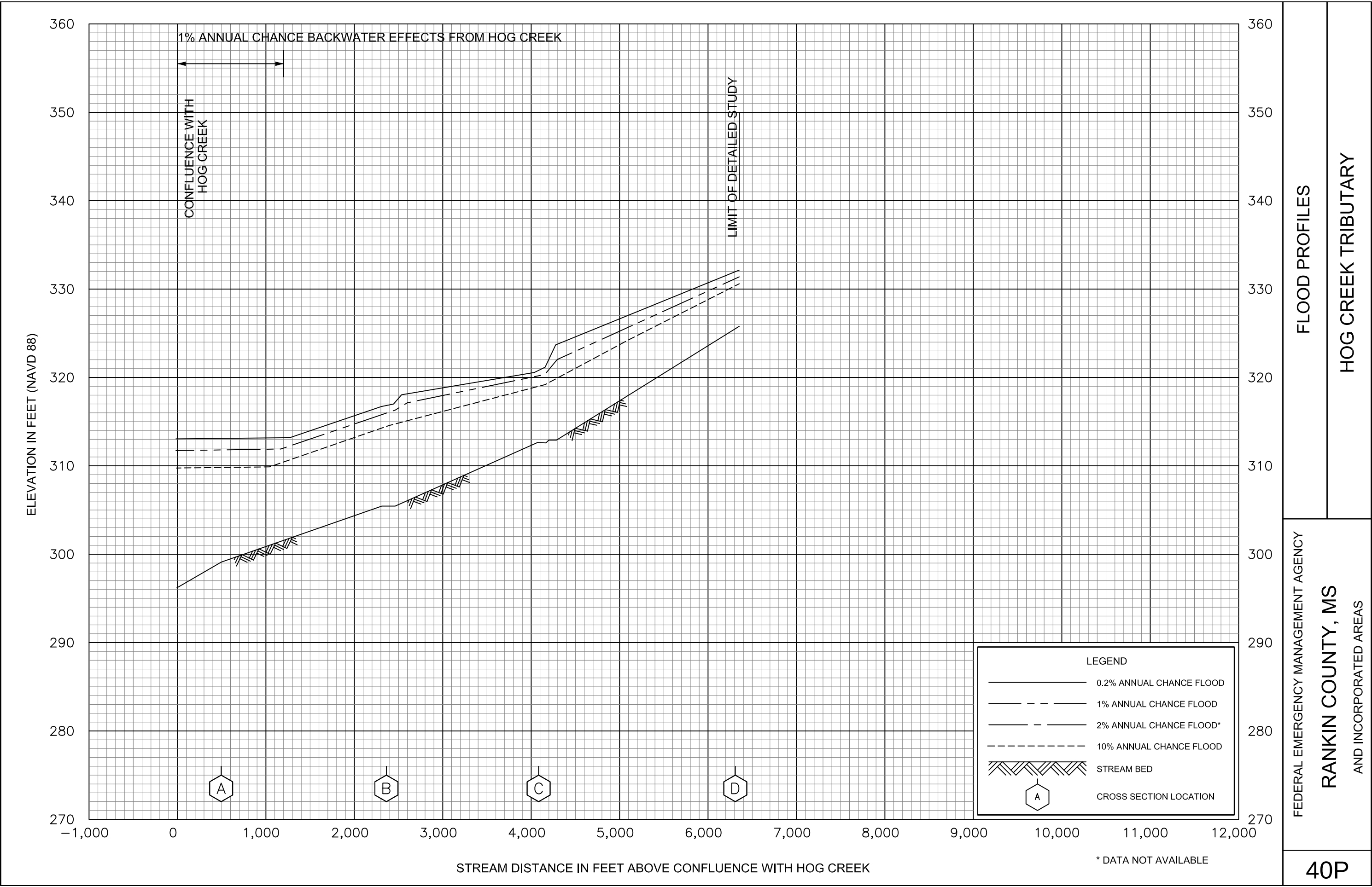
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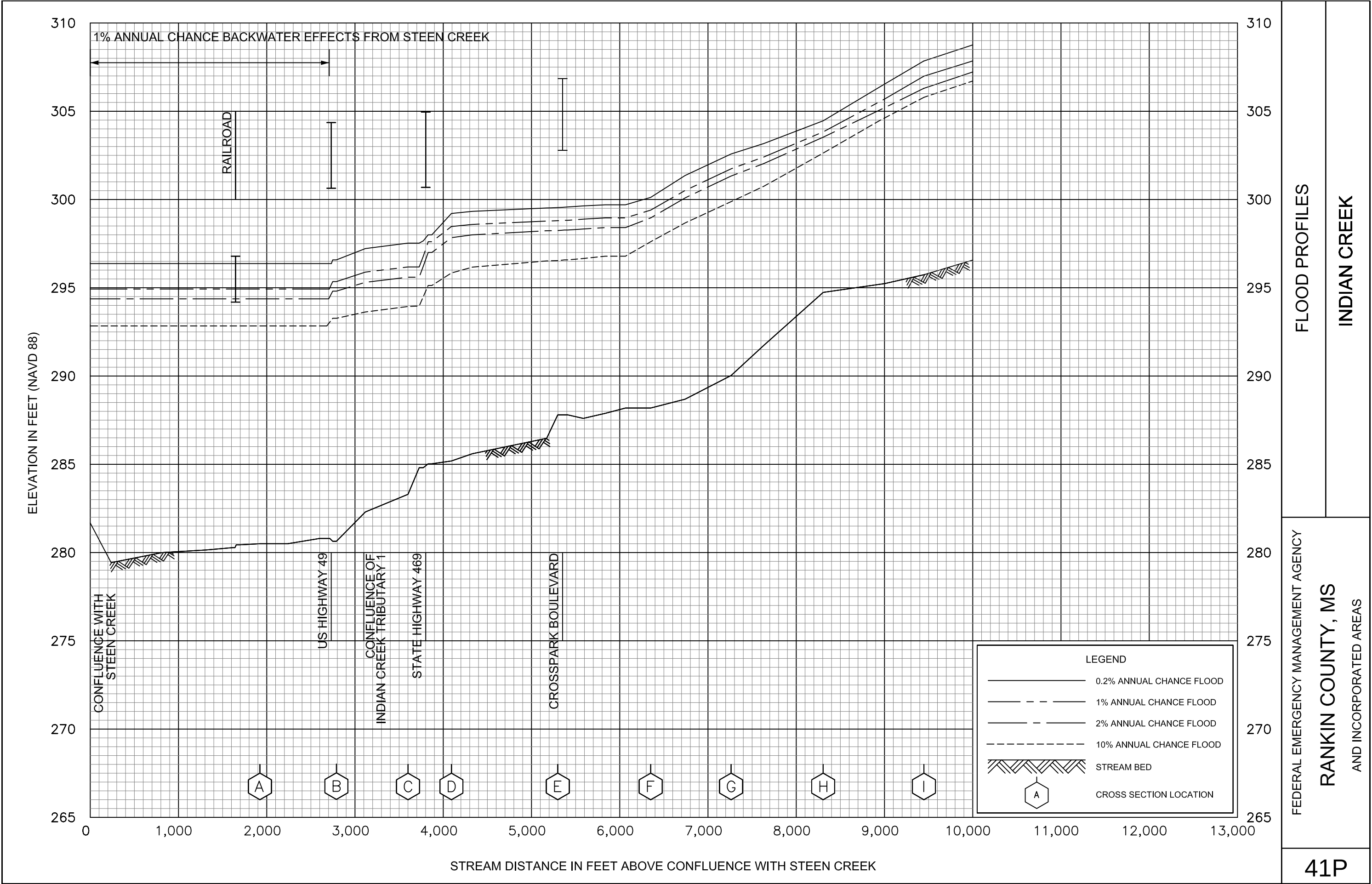
RANKIN COUNTY, MS AND INCORPORATED AREAS

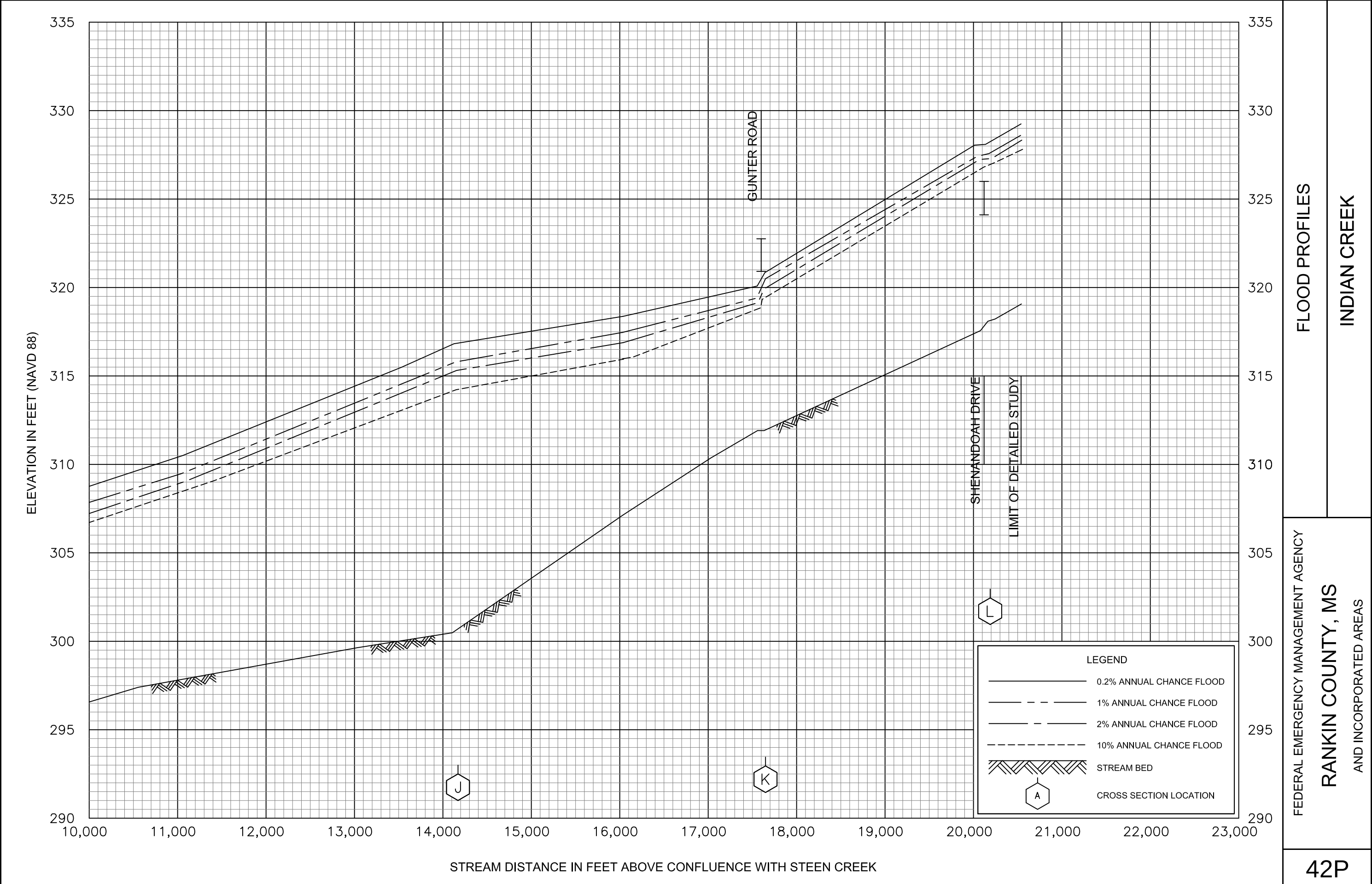
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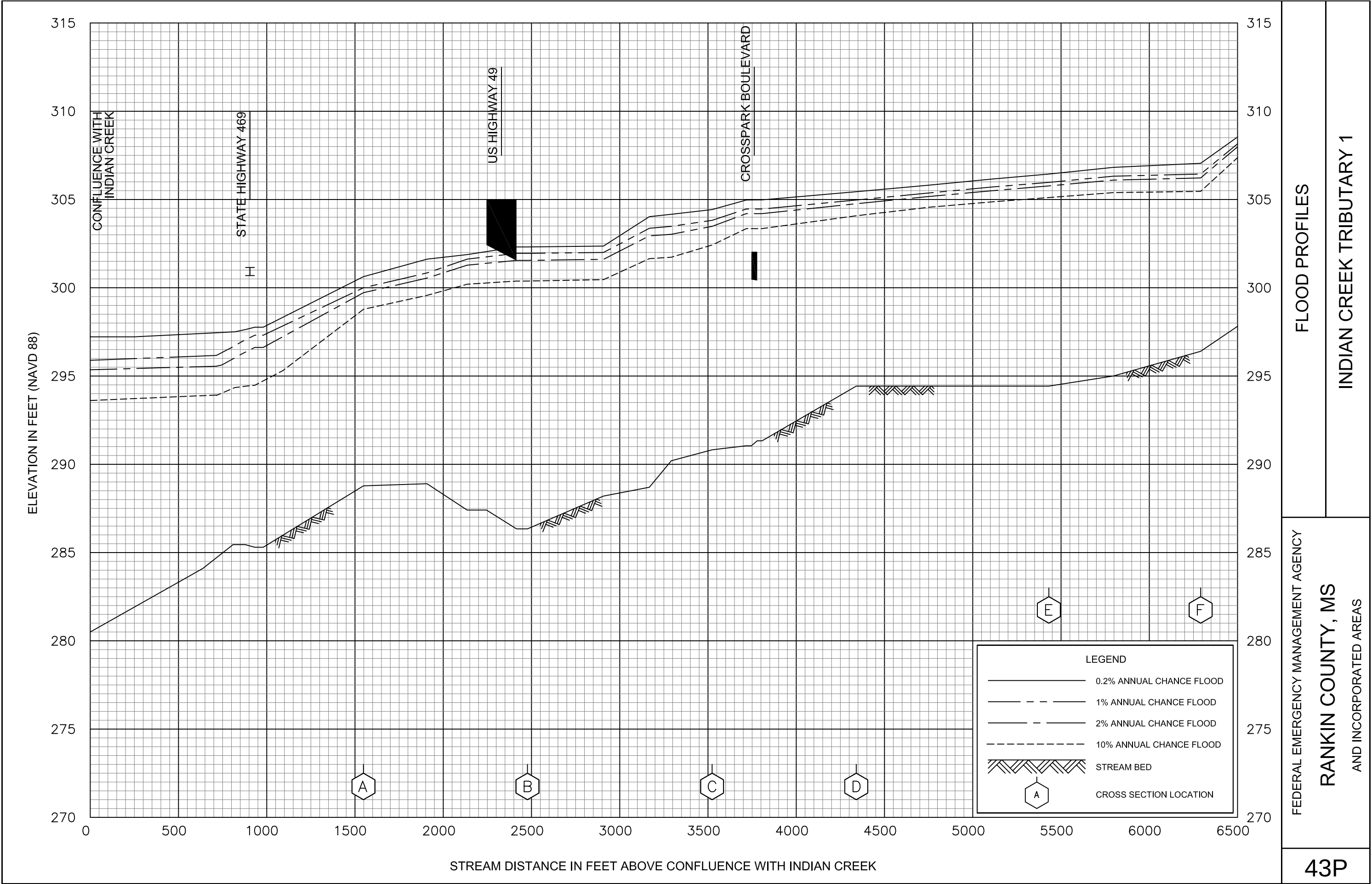
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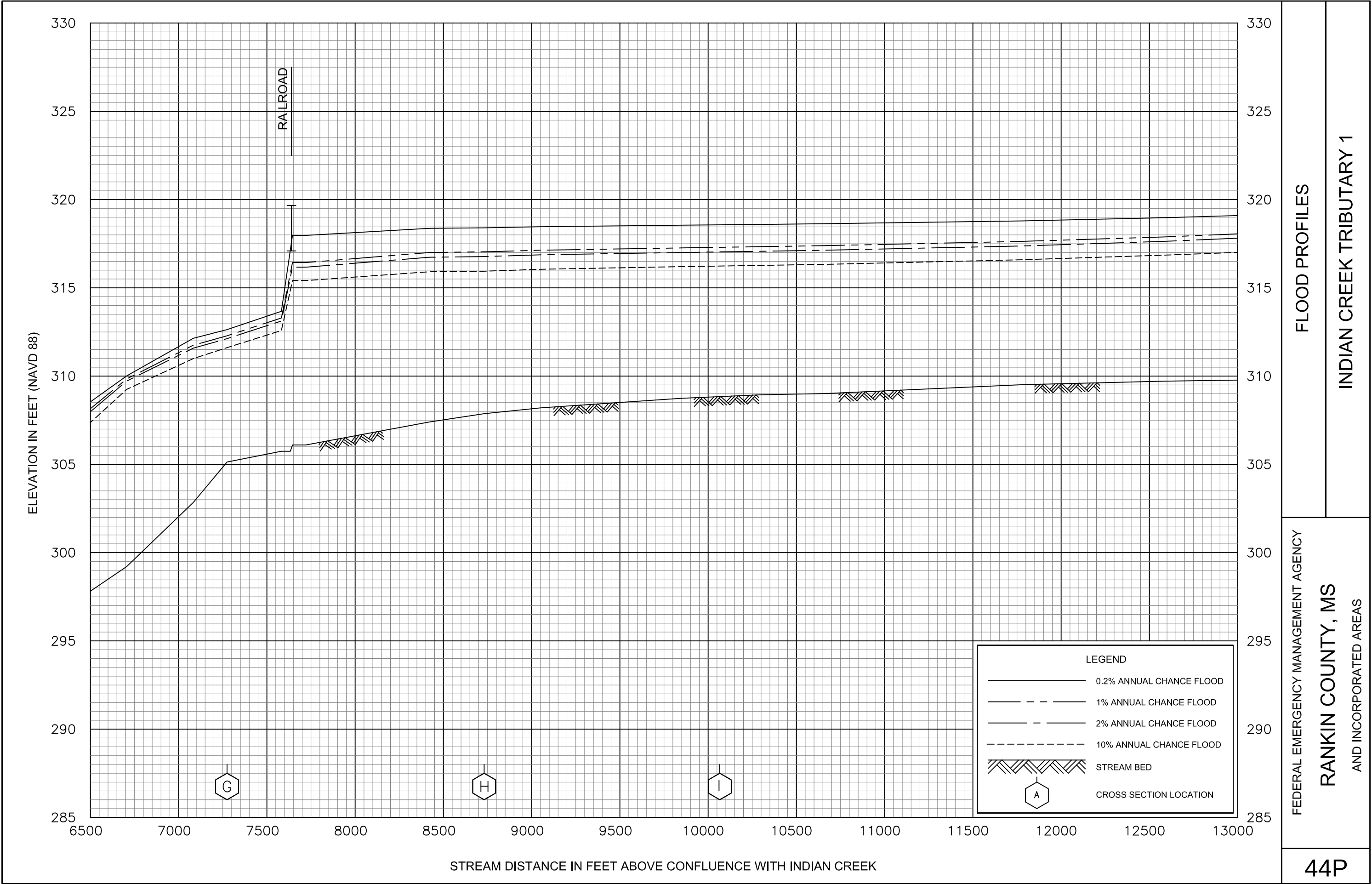
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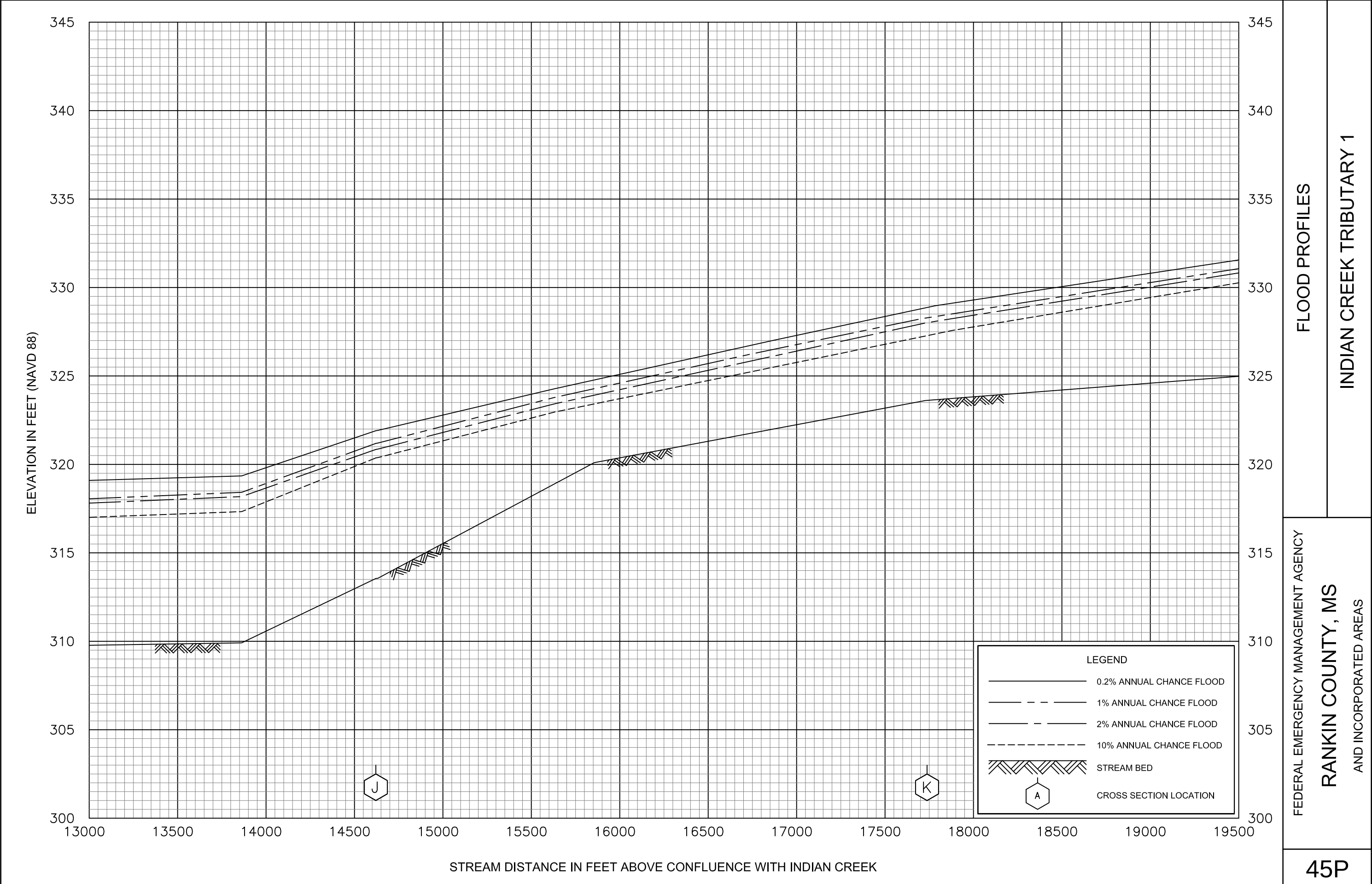


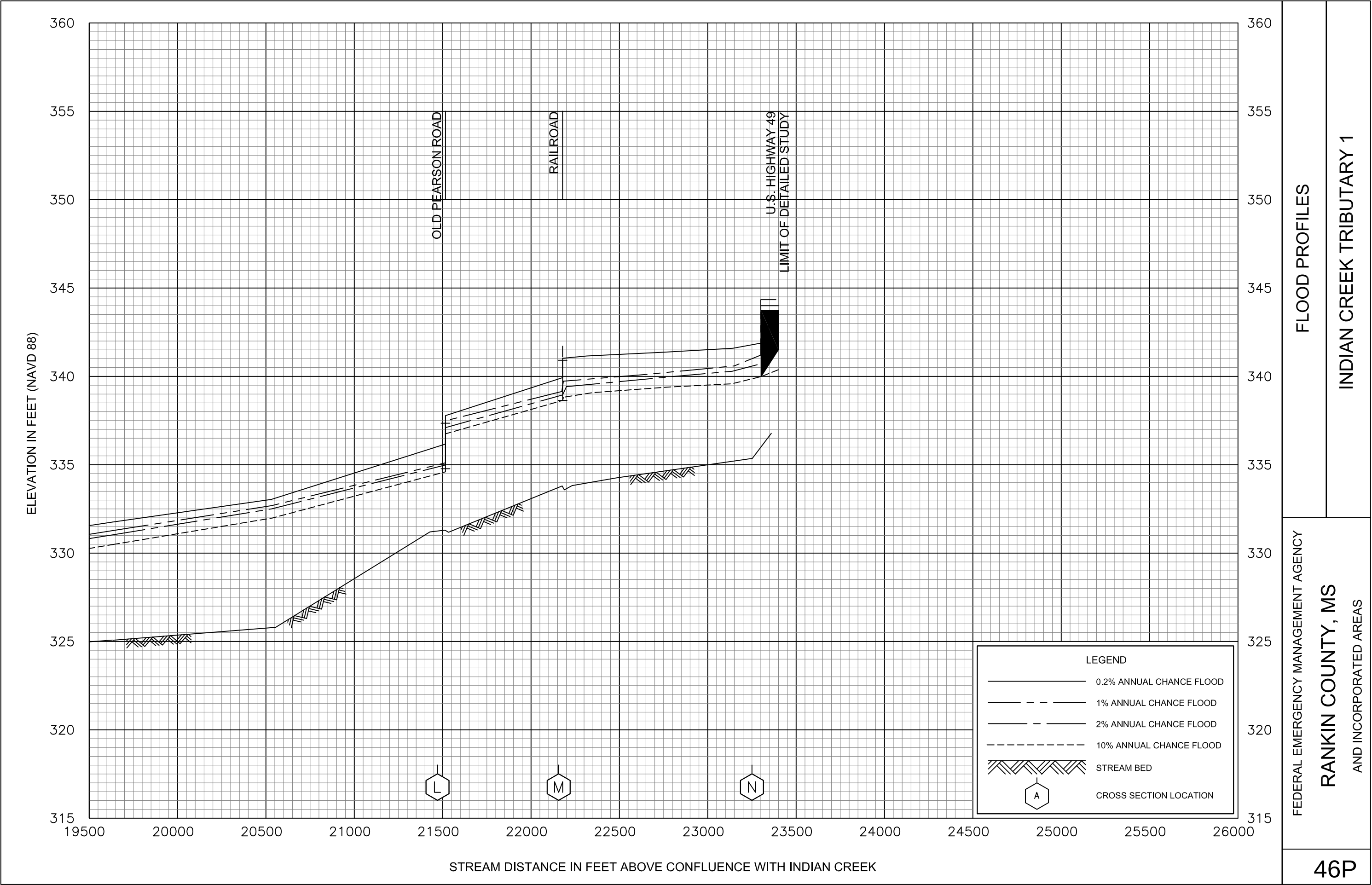


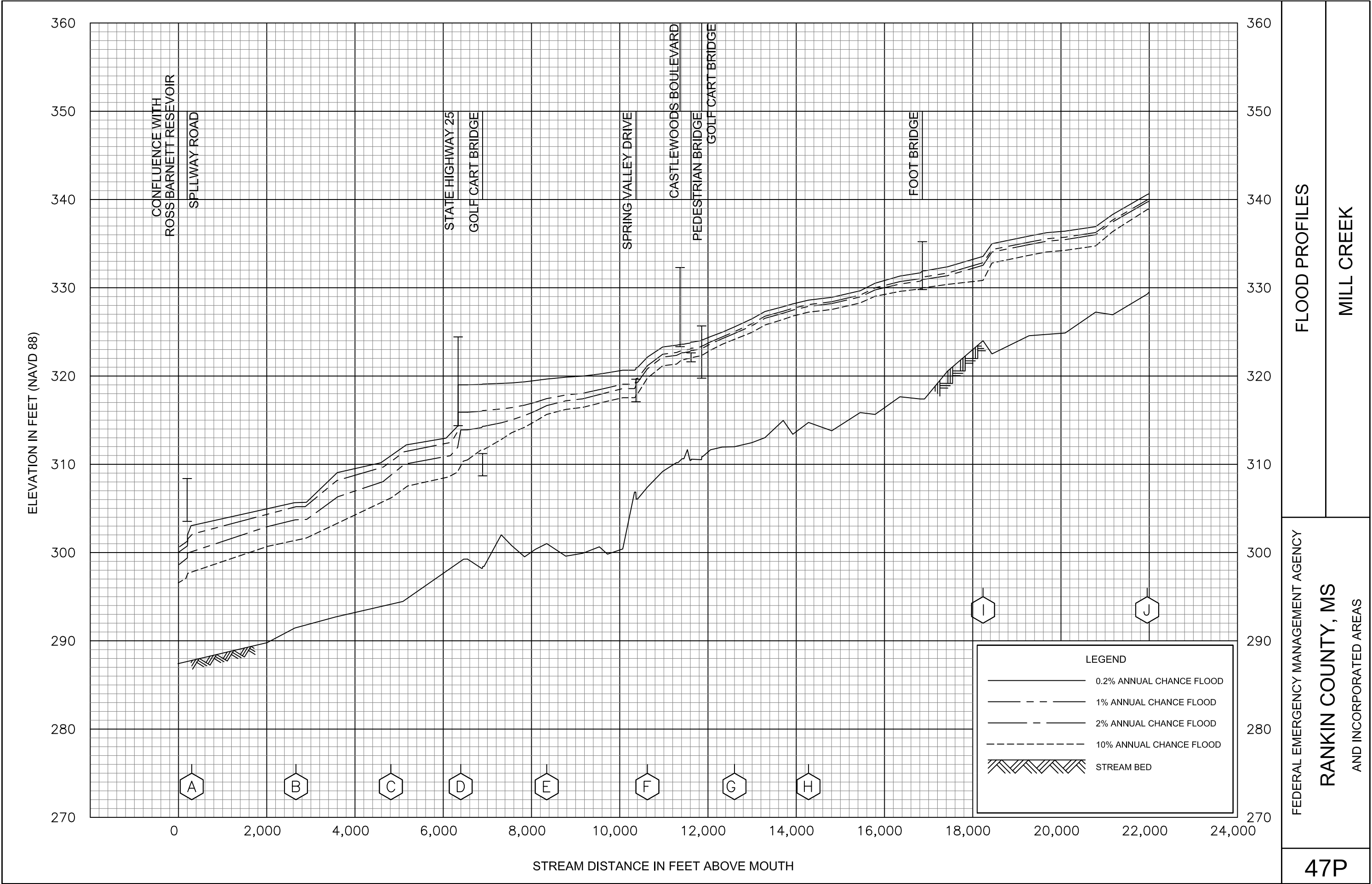


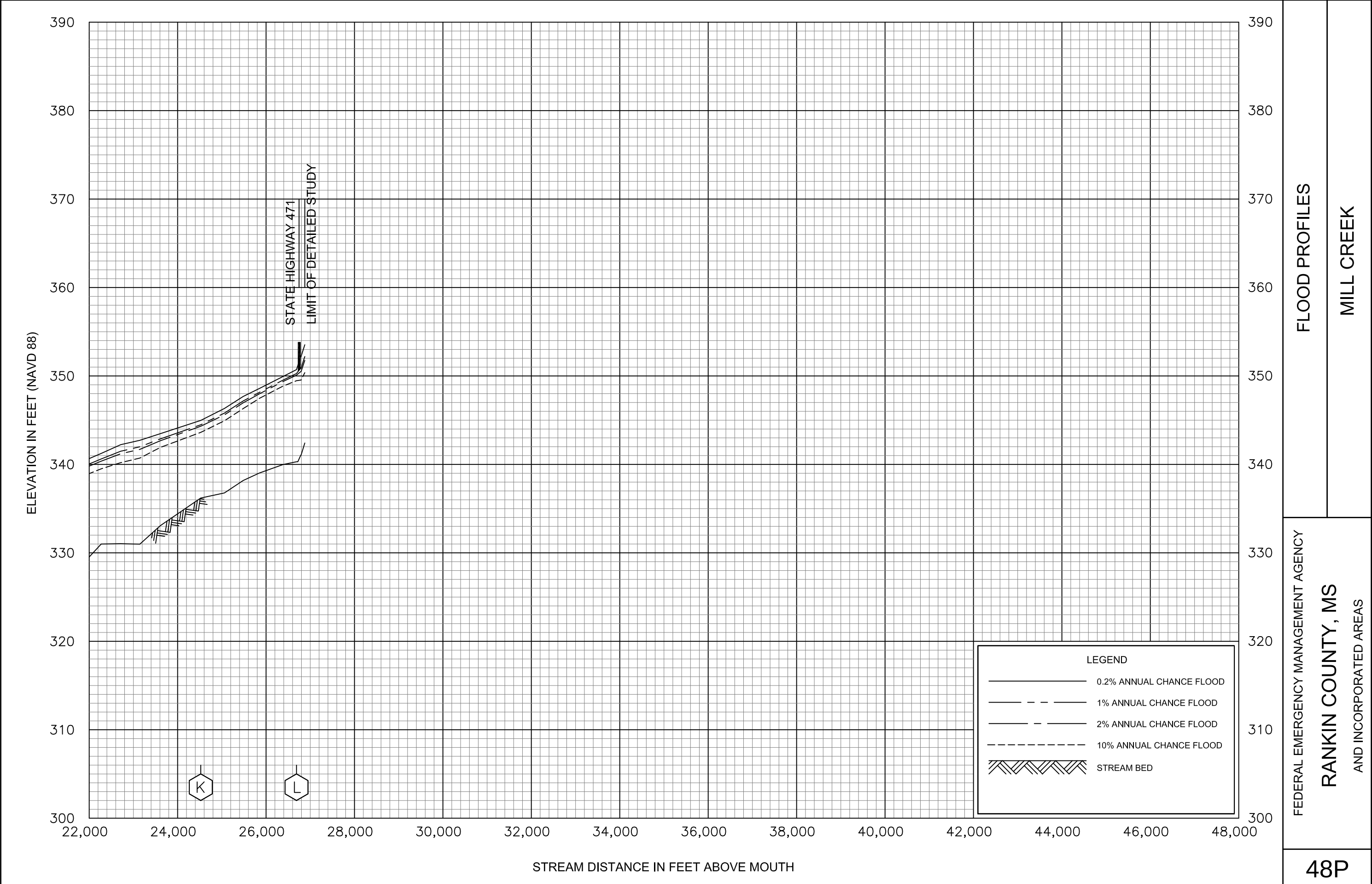


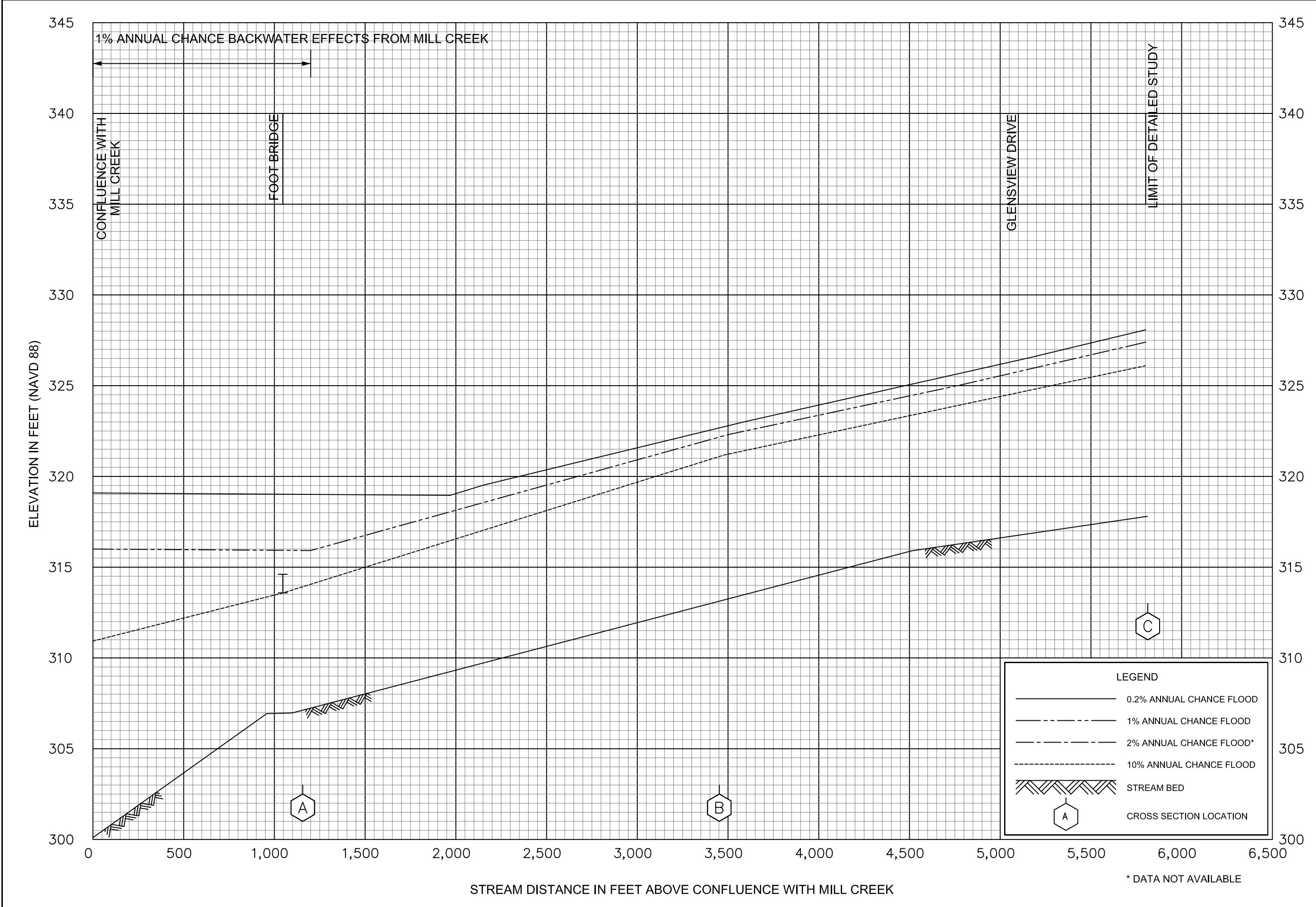


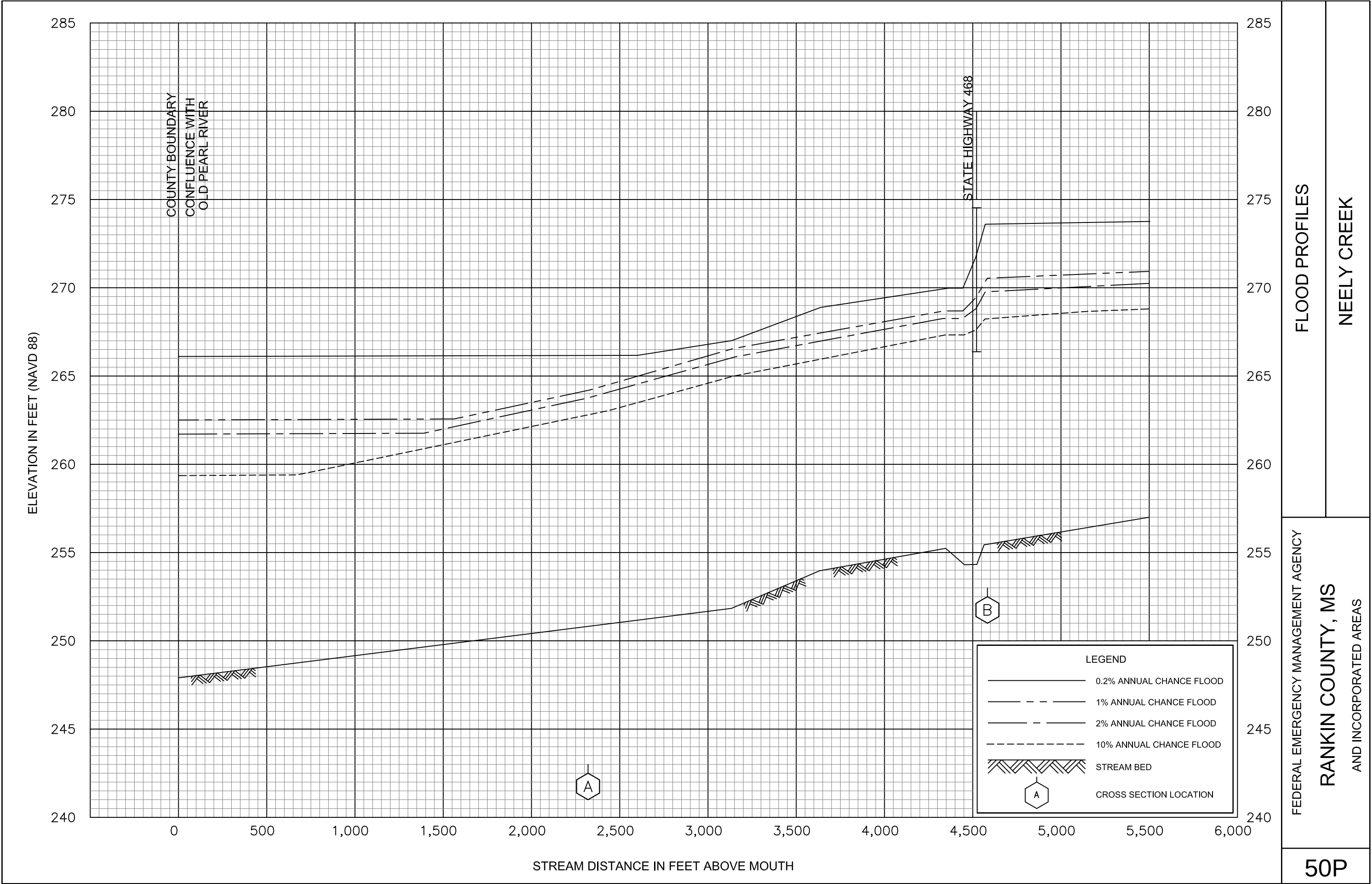


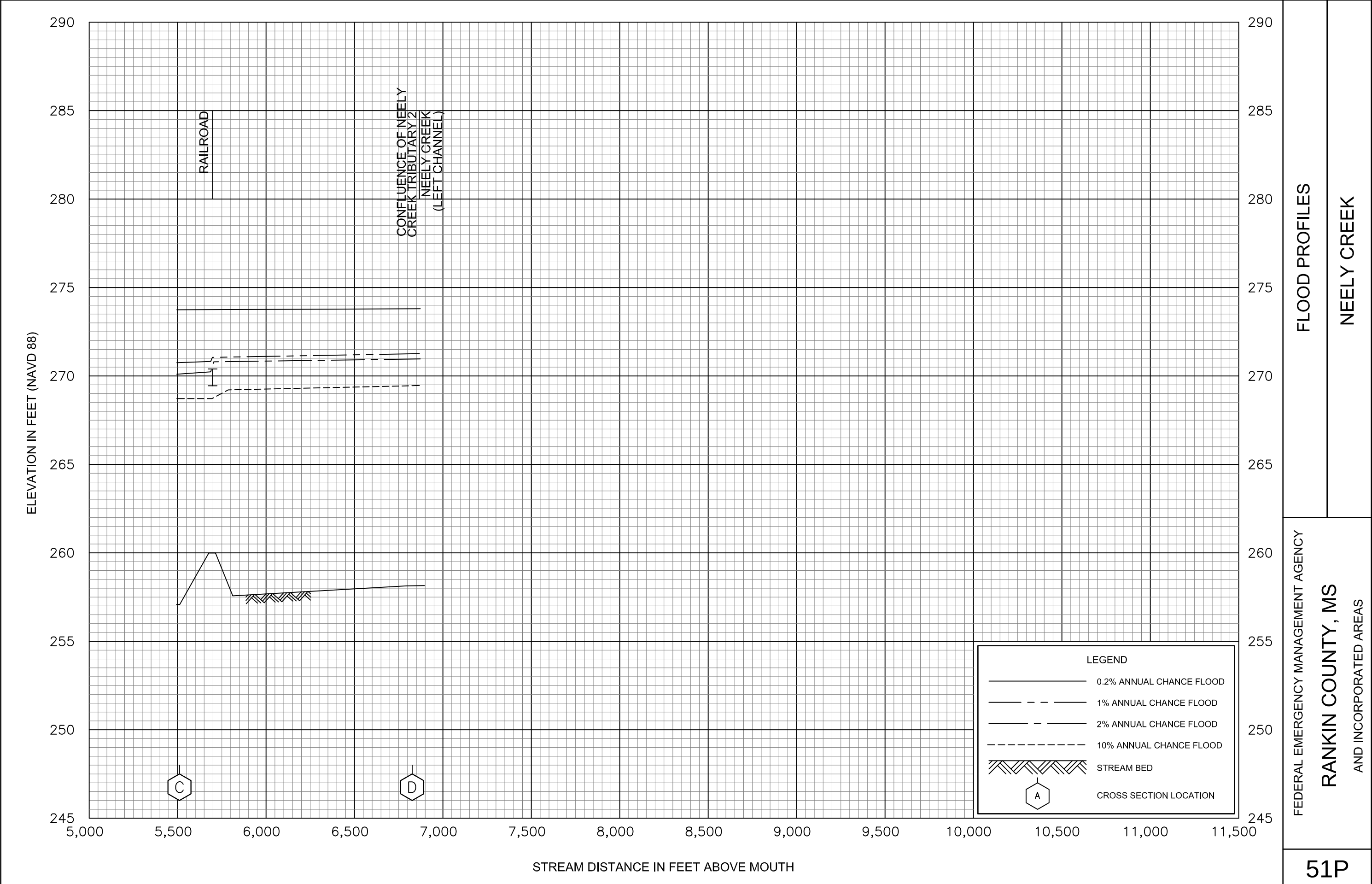












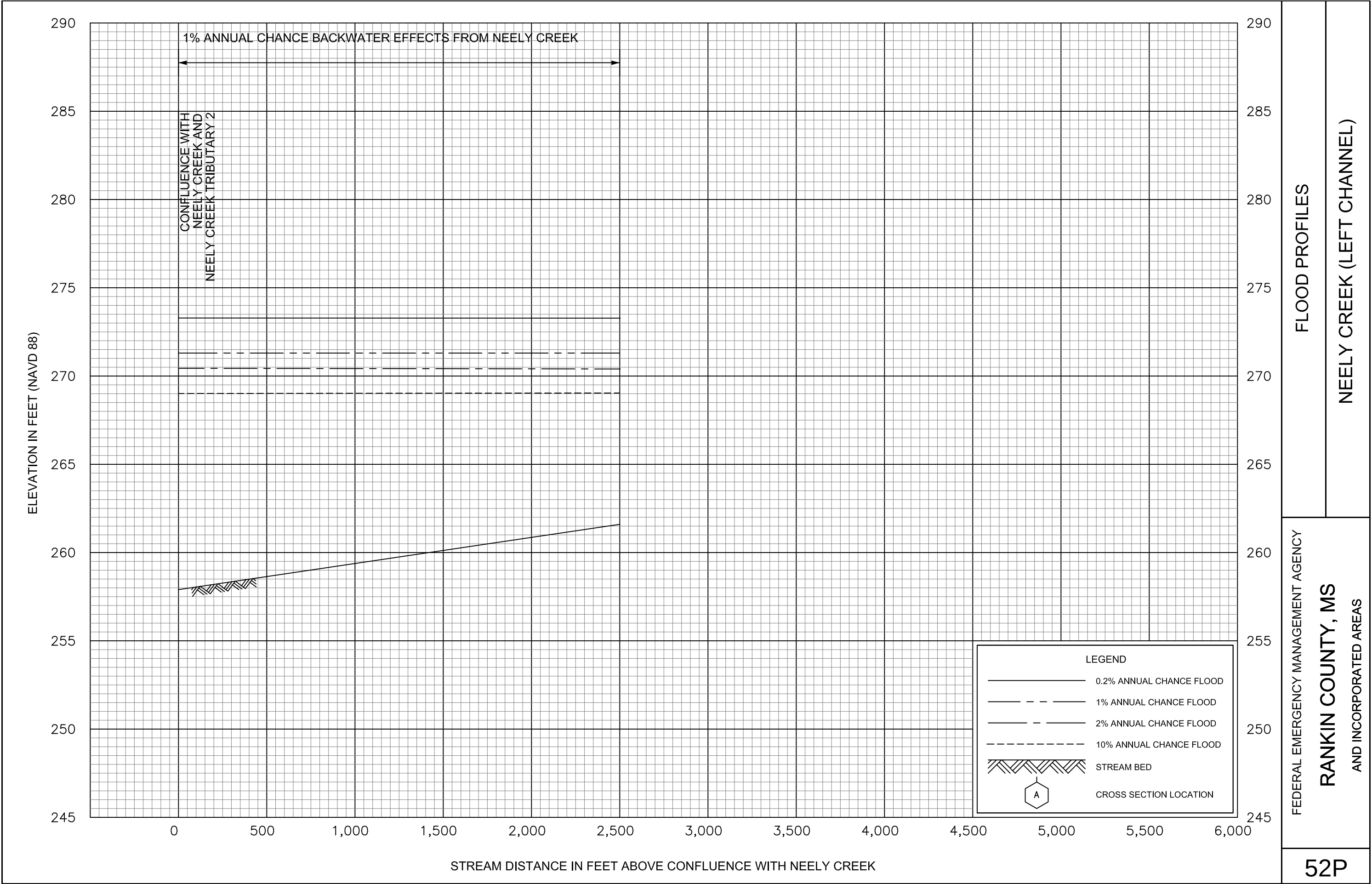
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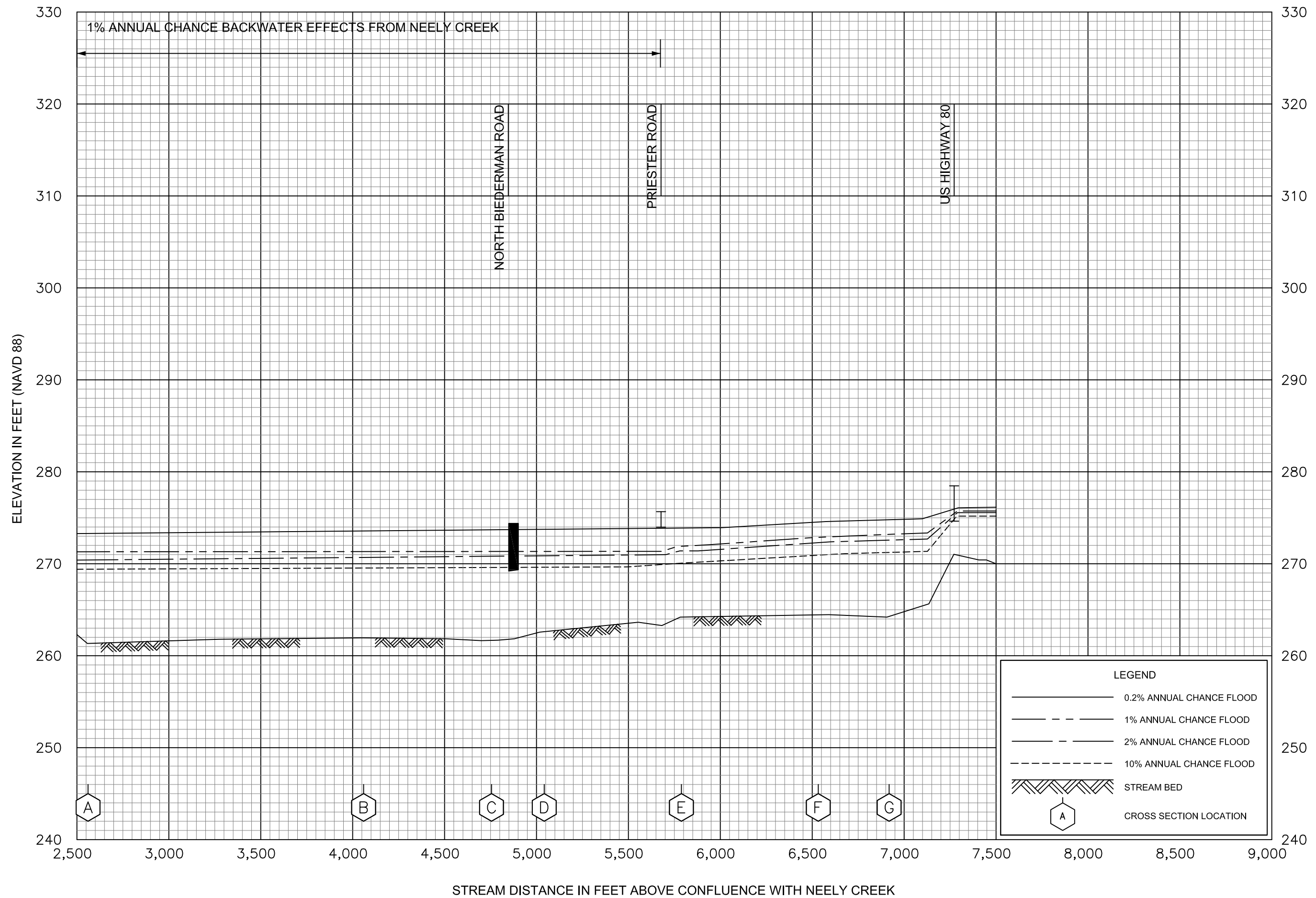
NEELY CREEK

FEDERAL EMERGENCY MANAGEMENT AGENCY

RANKIN COUNTY, MS

AND INCORPORATED AREAS





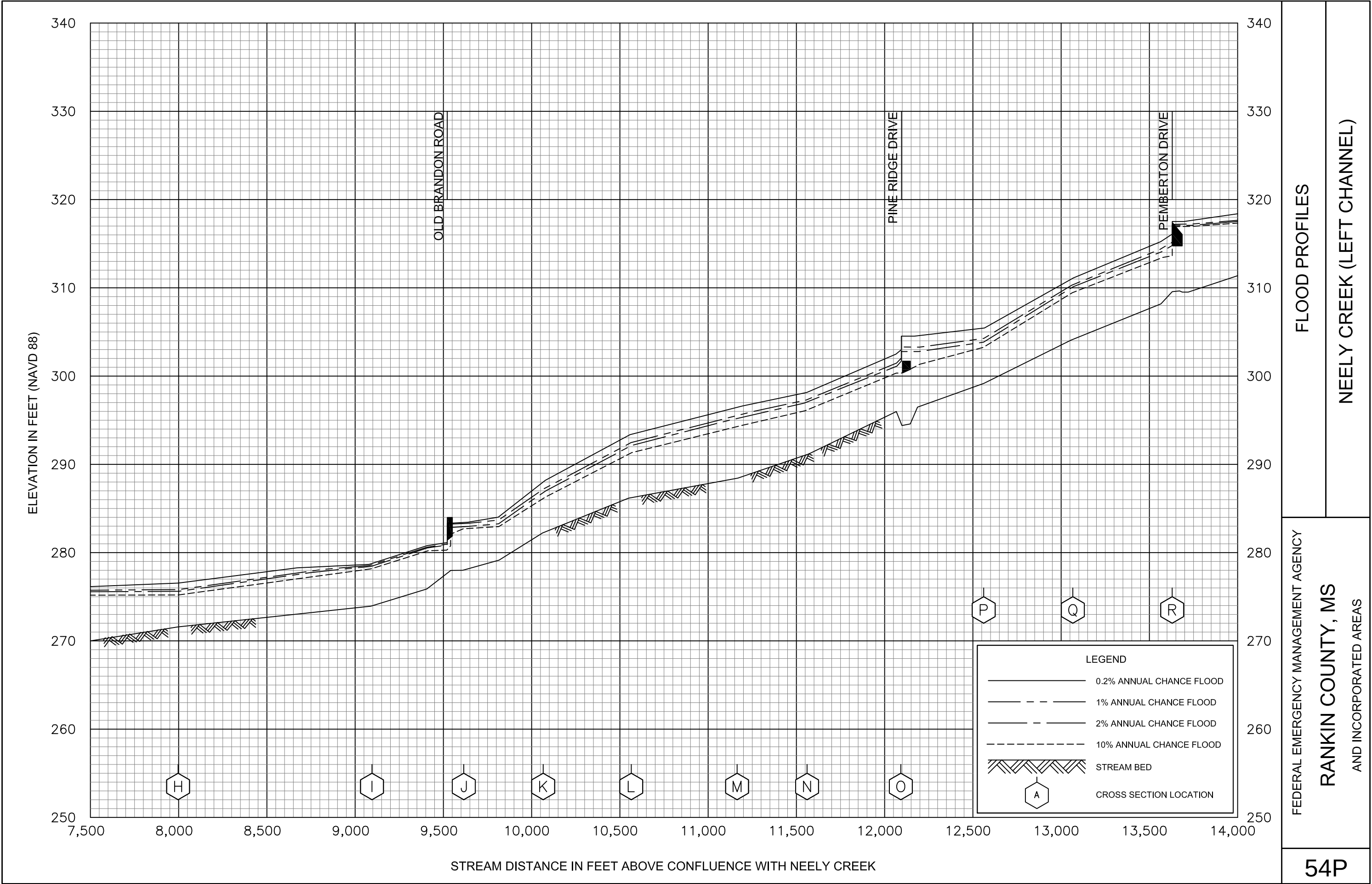
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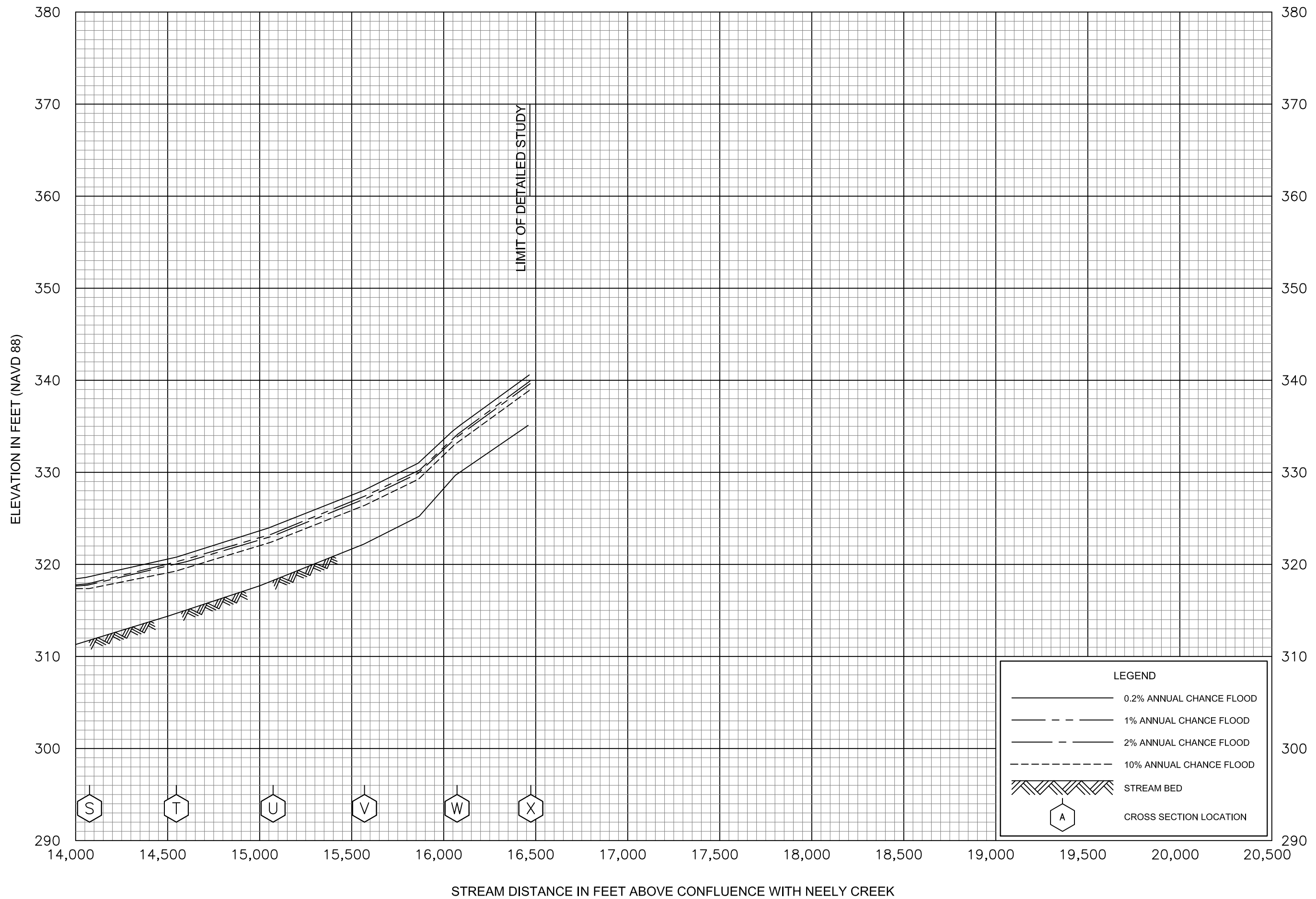
RANKIN COUNTY, MS AND INCORPORATED AREAS

FLOOD PROFILES

NEELY CREEK (LEFT CHANNEL)

53P





LIMIT OF DETAILED STUDY

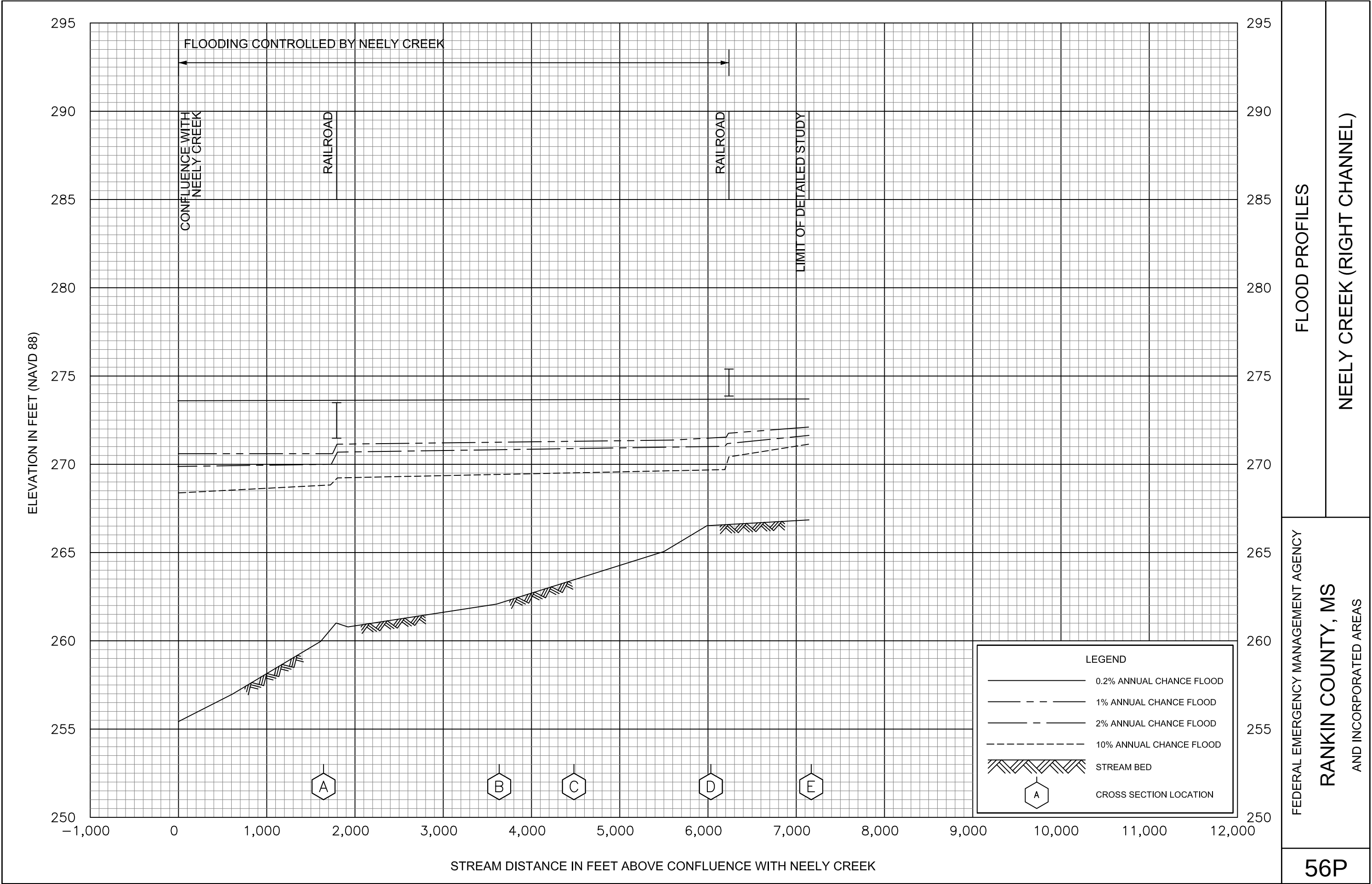
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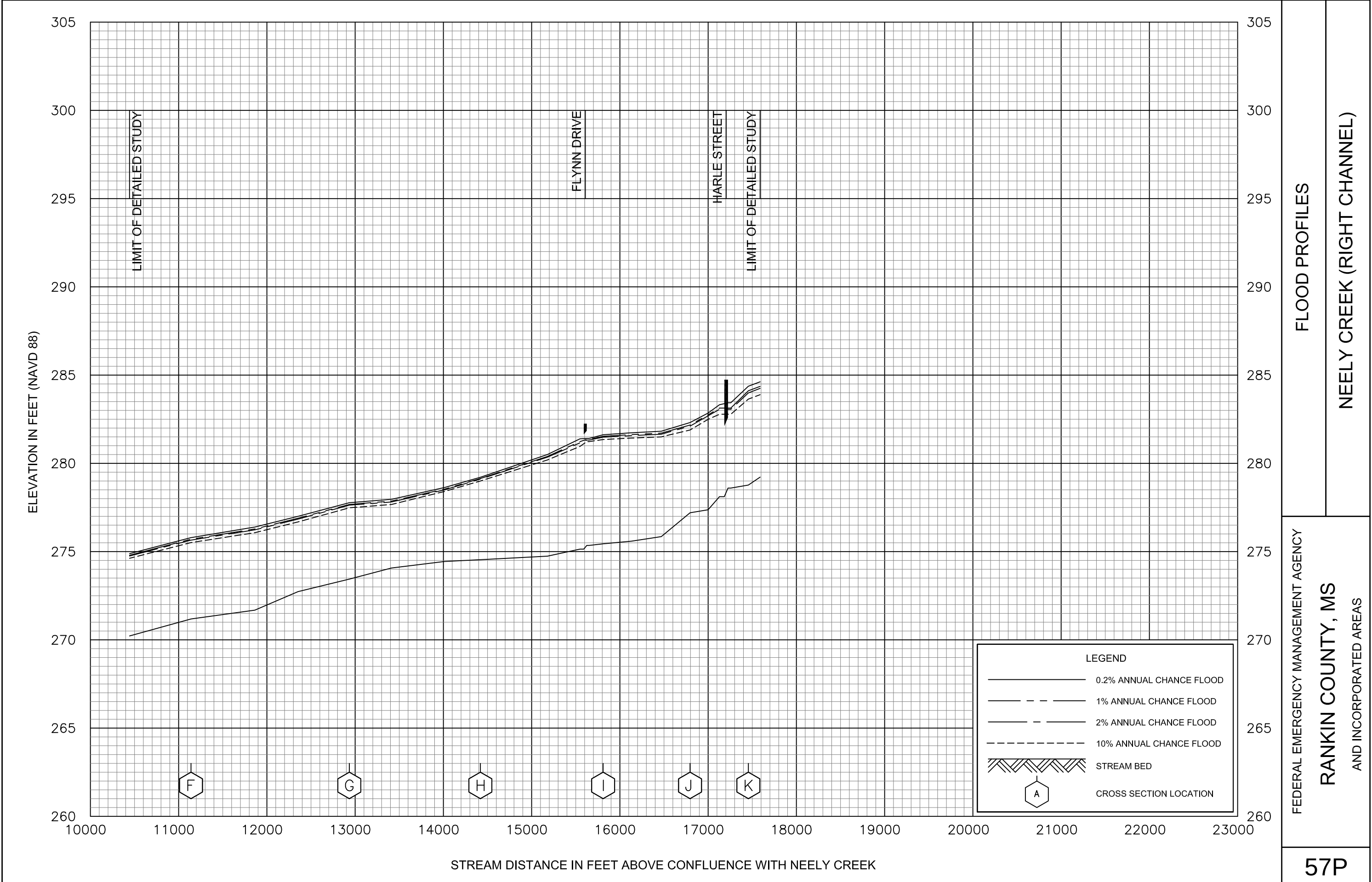
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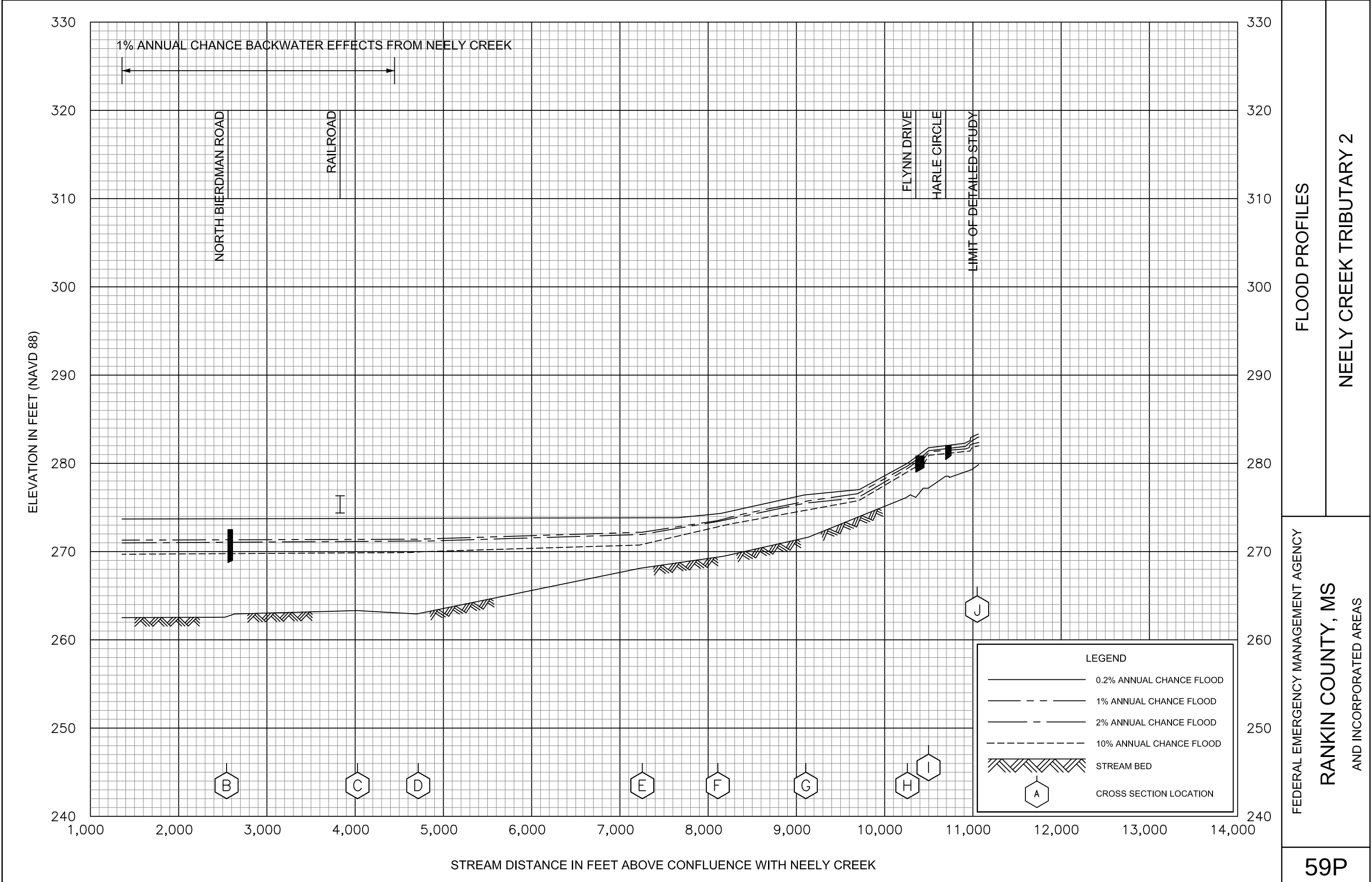
RANKIN COUNTY, MS

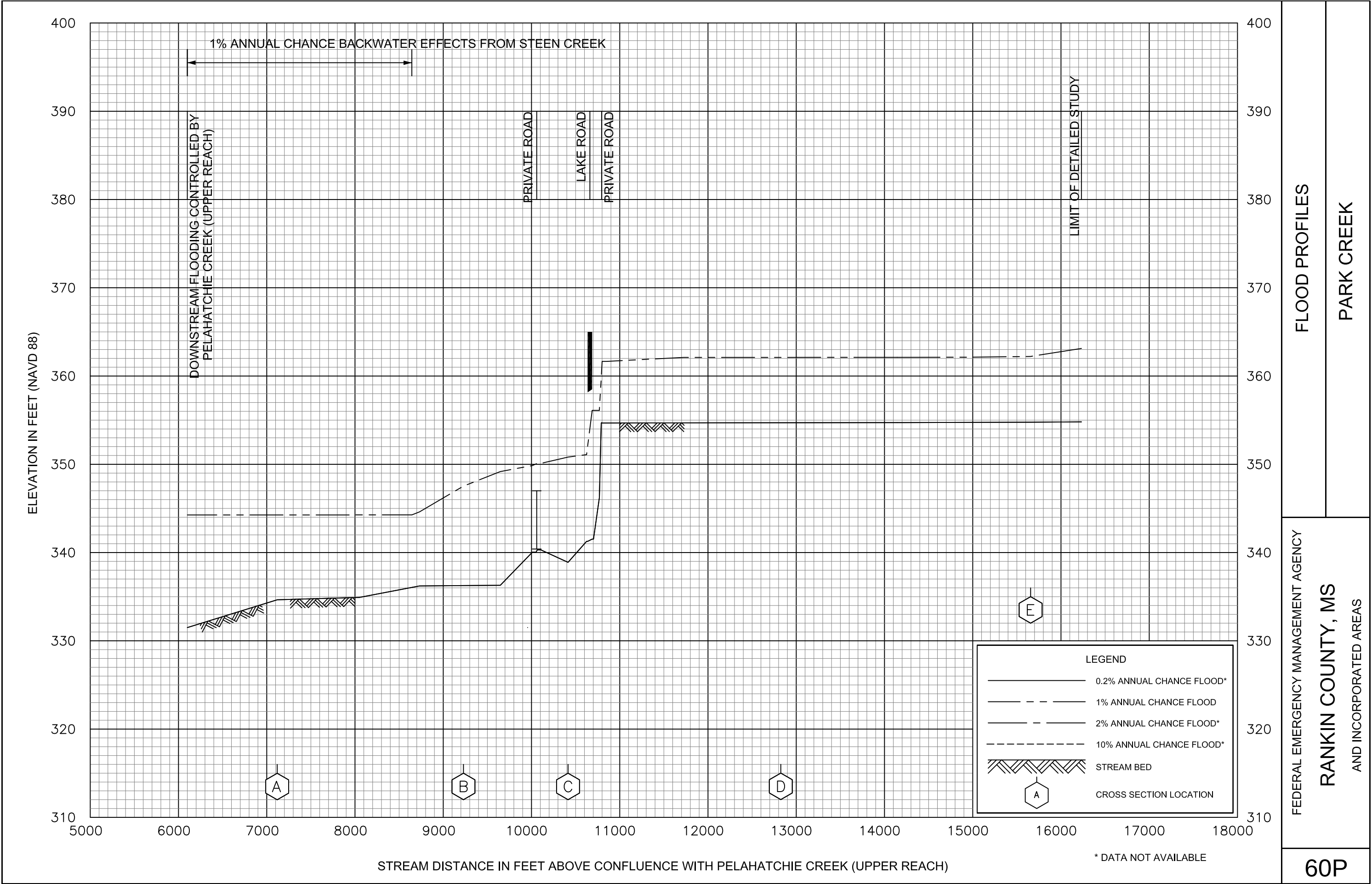
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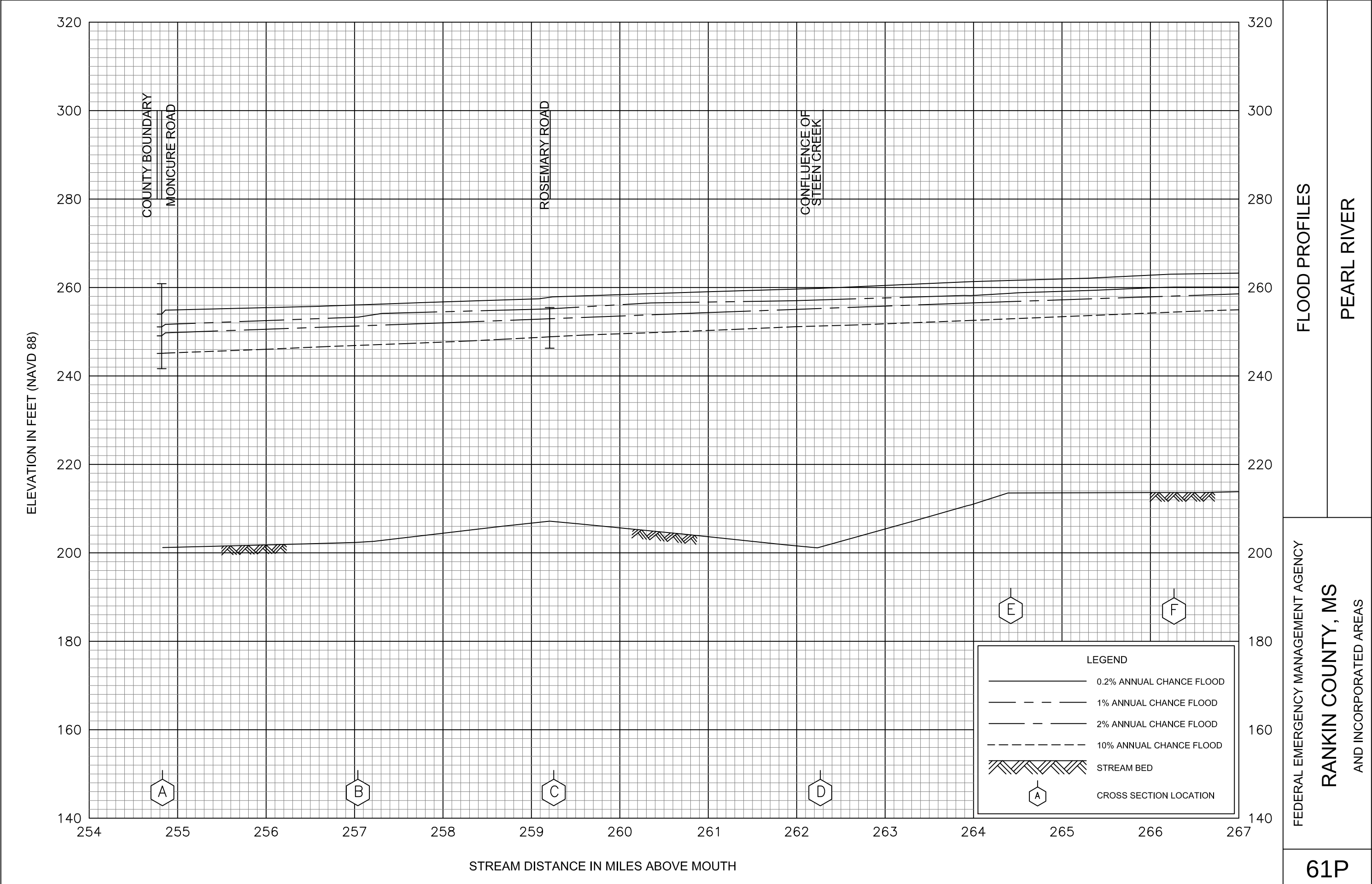
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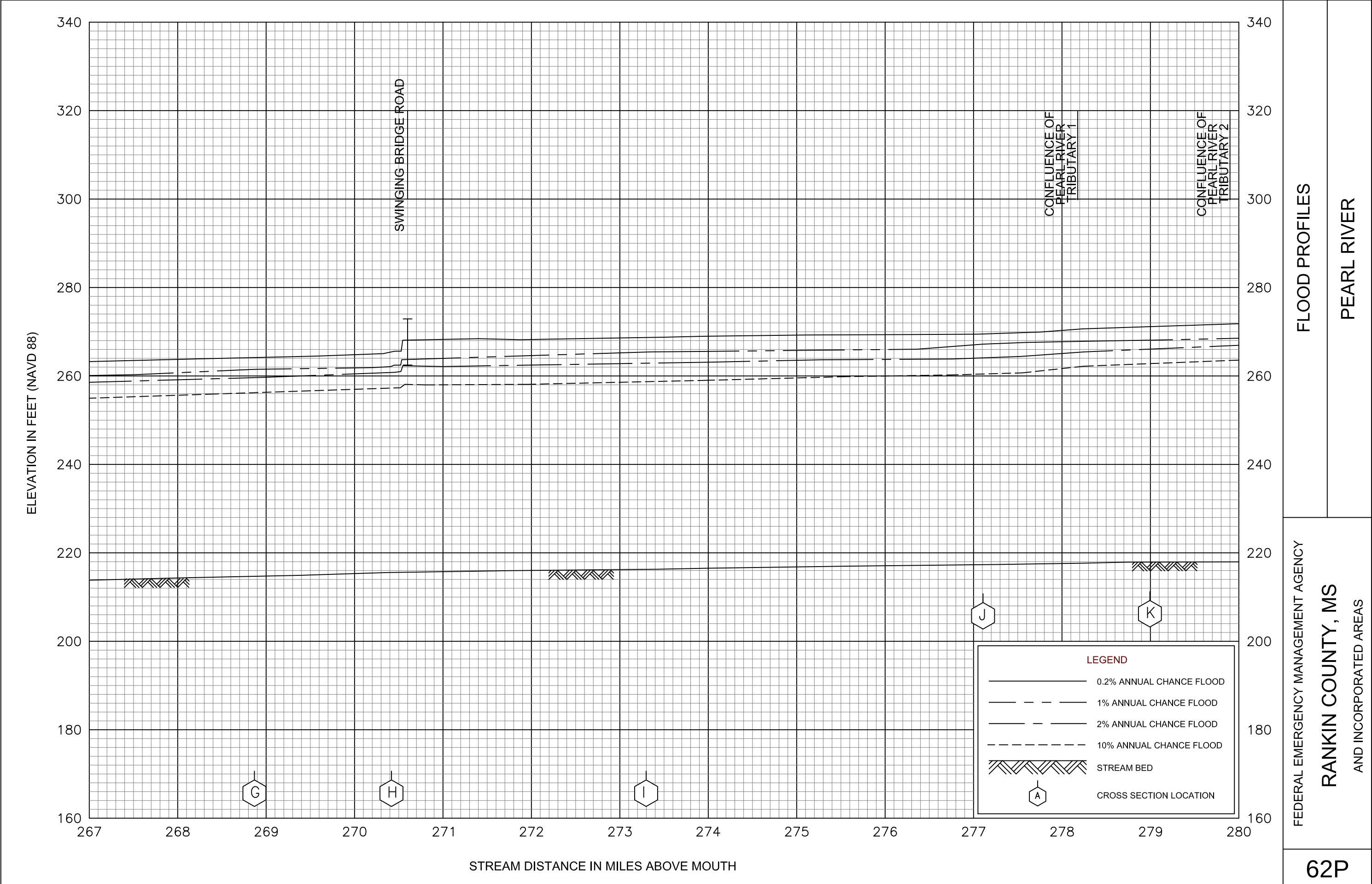


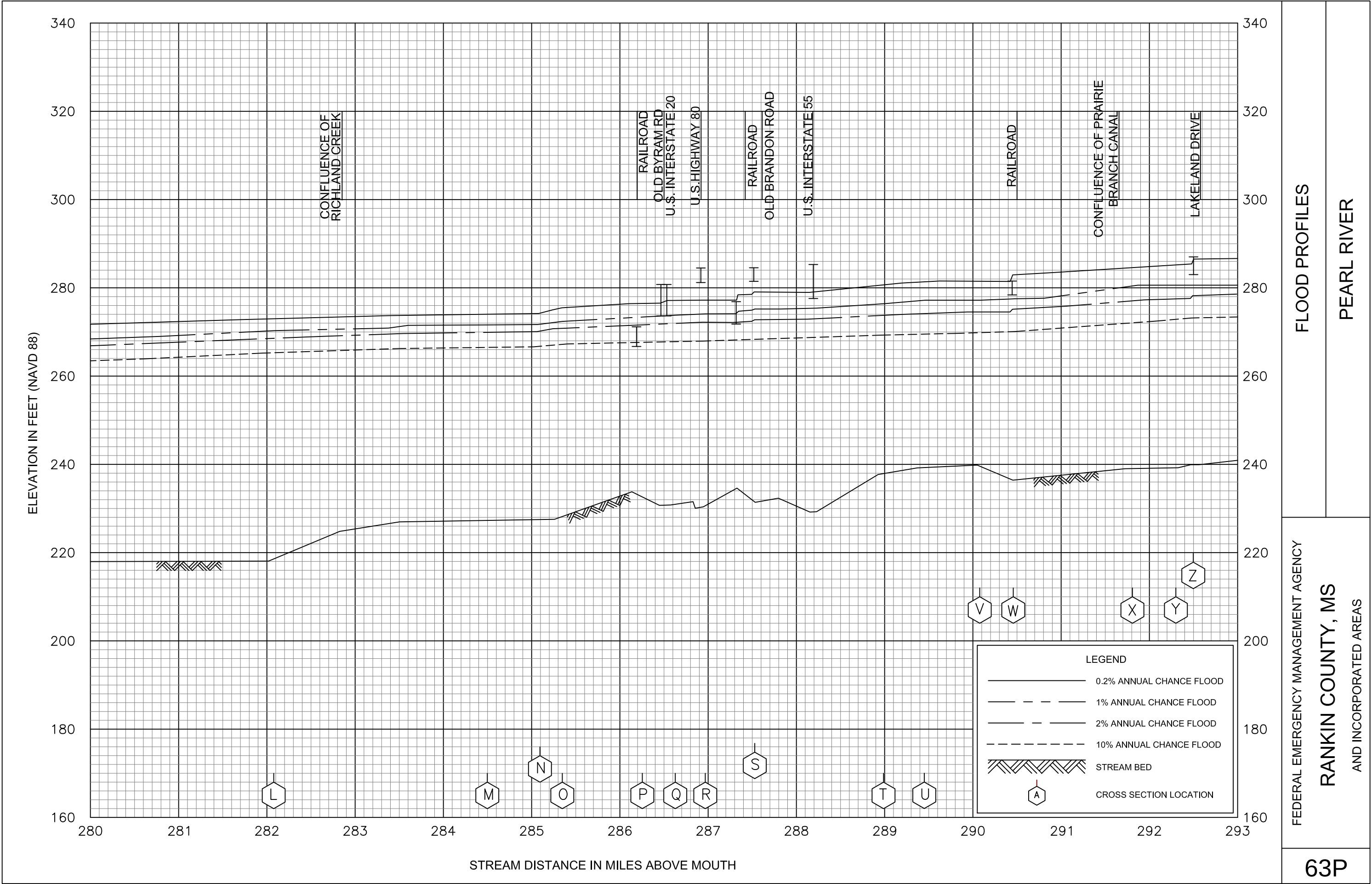


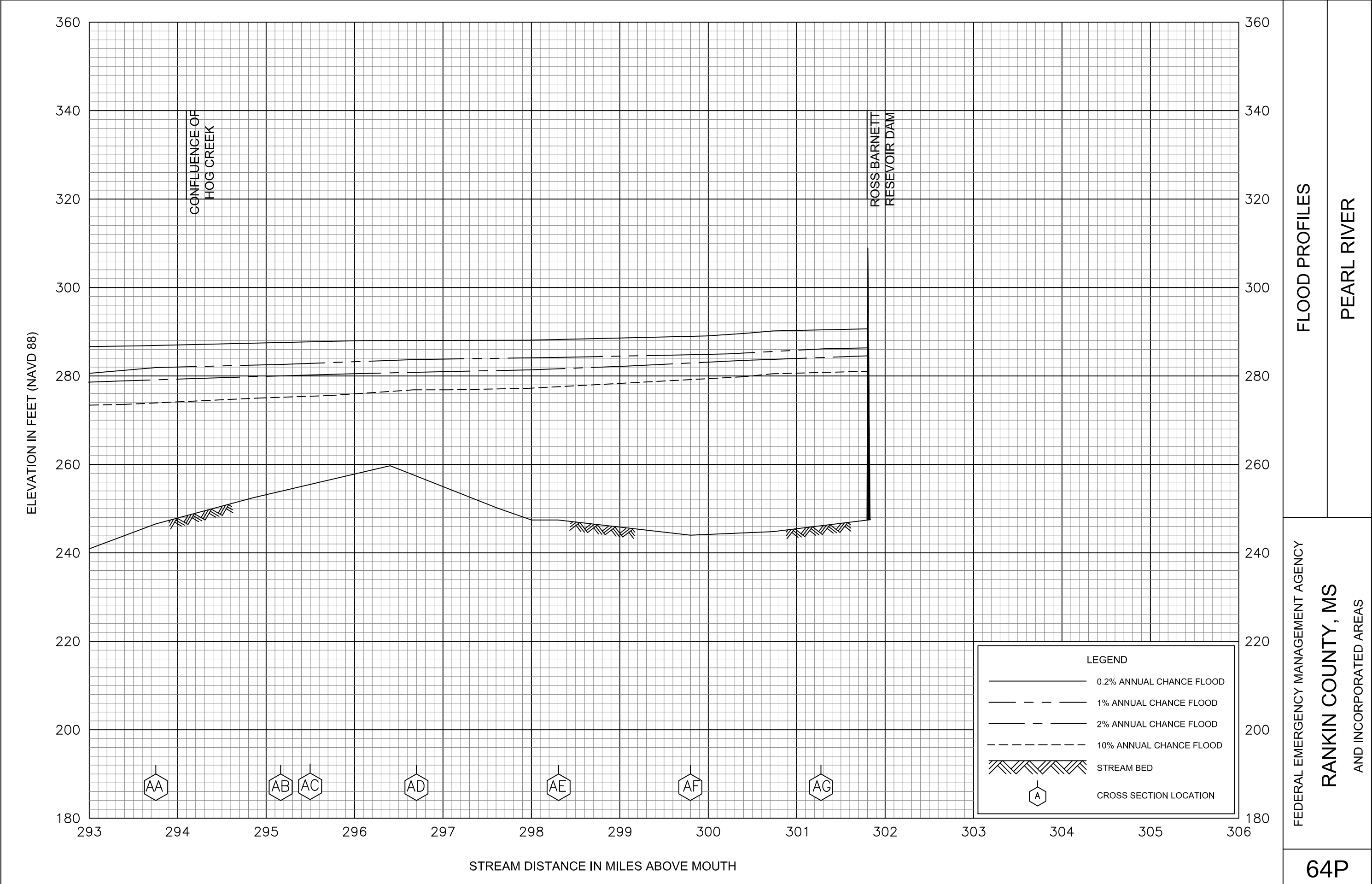


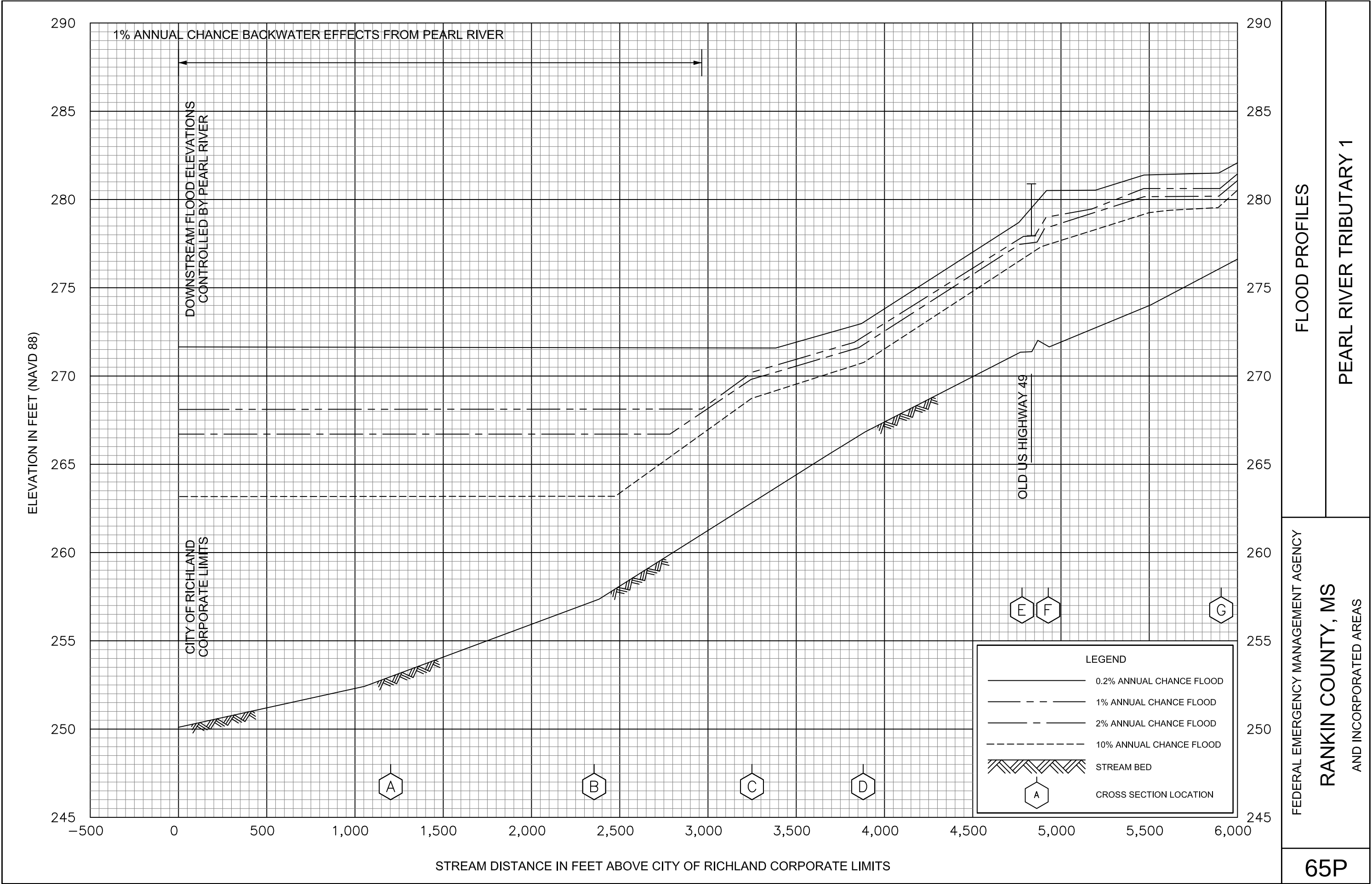


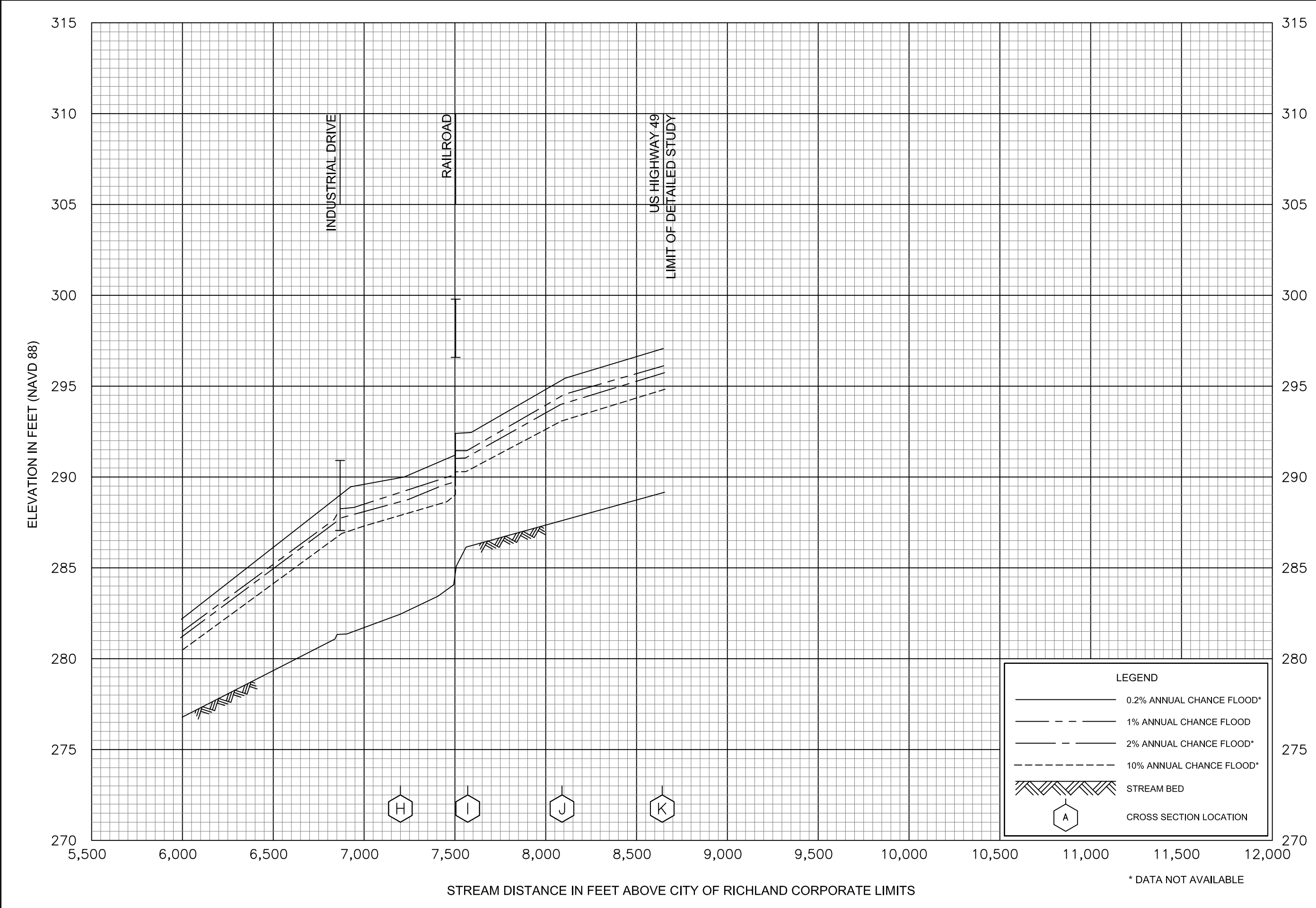


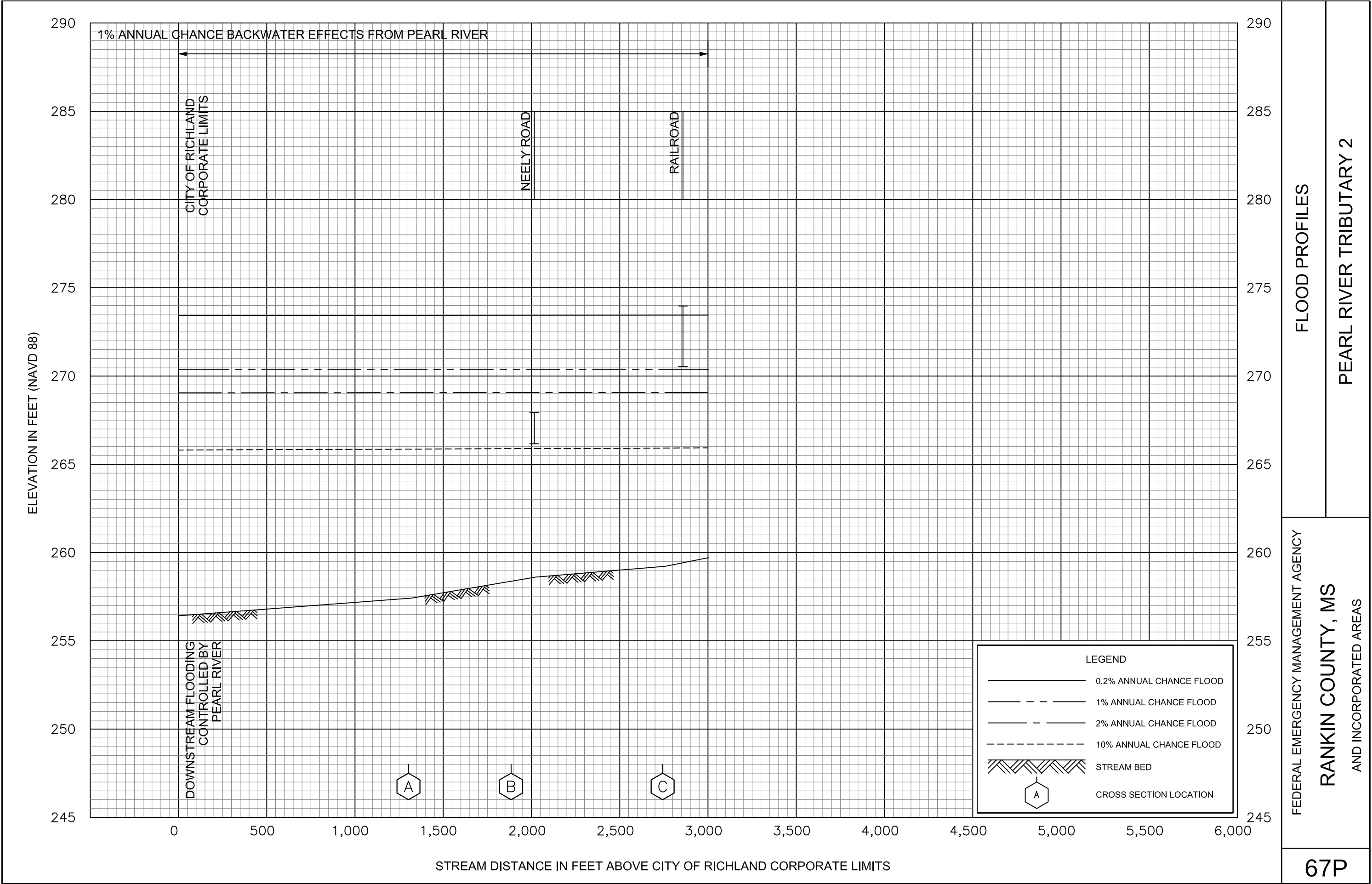


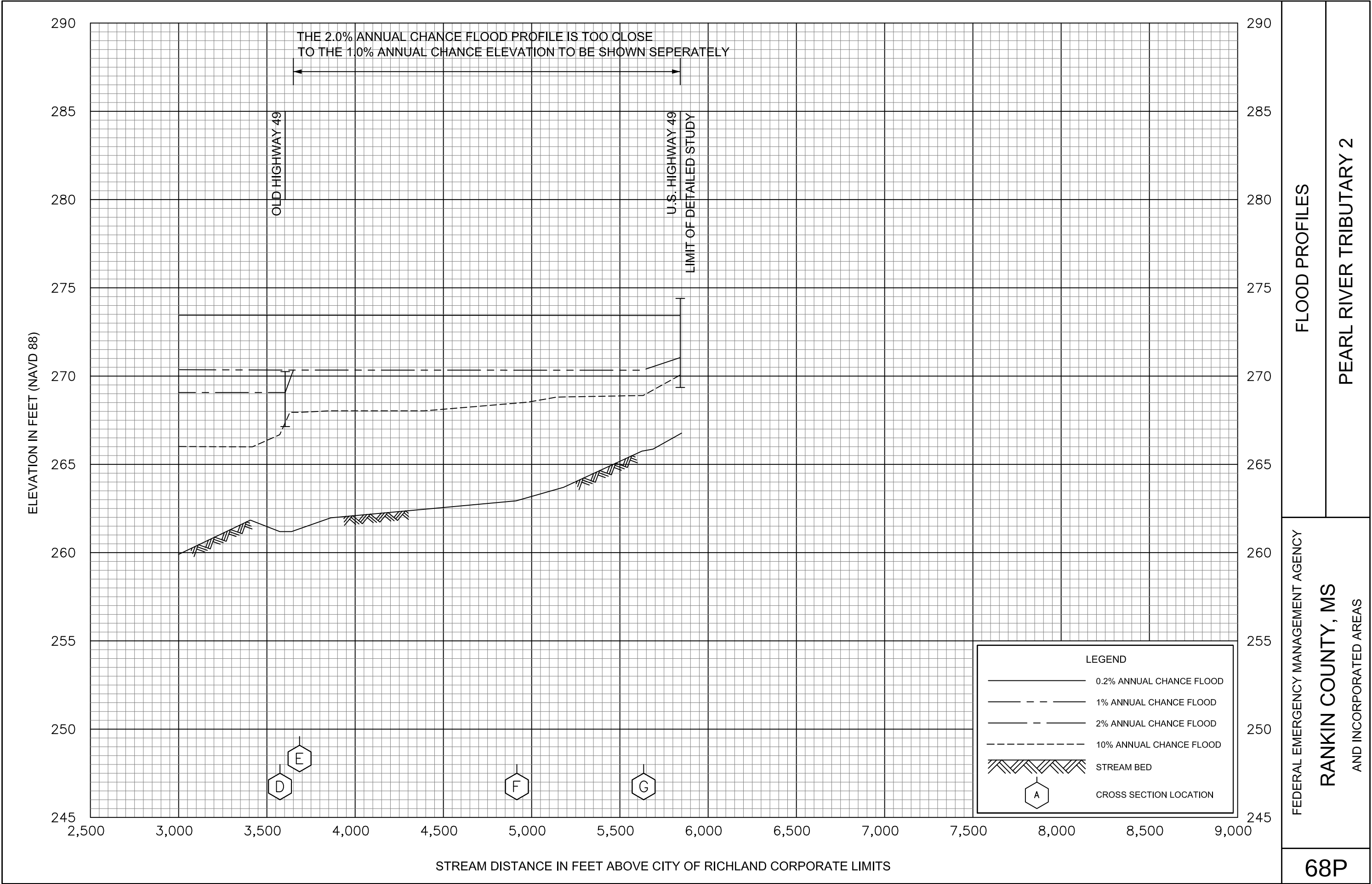


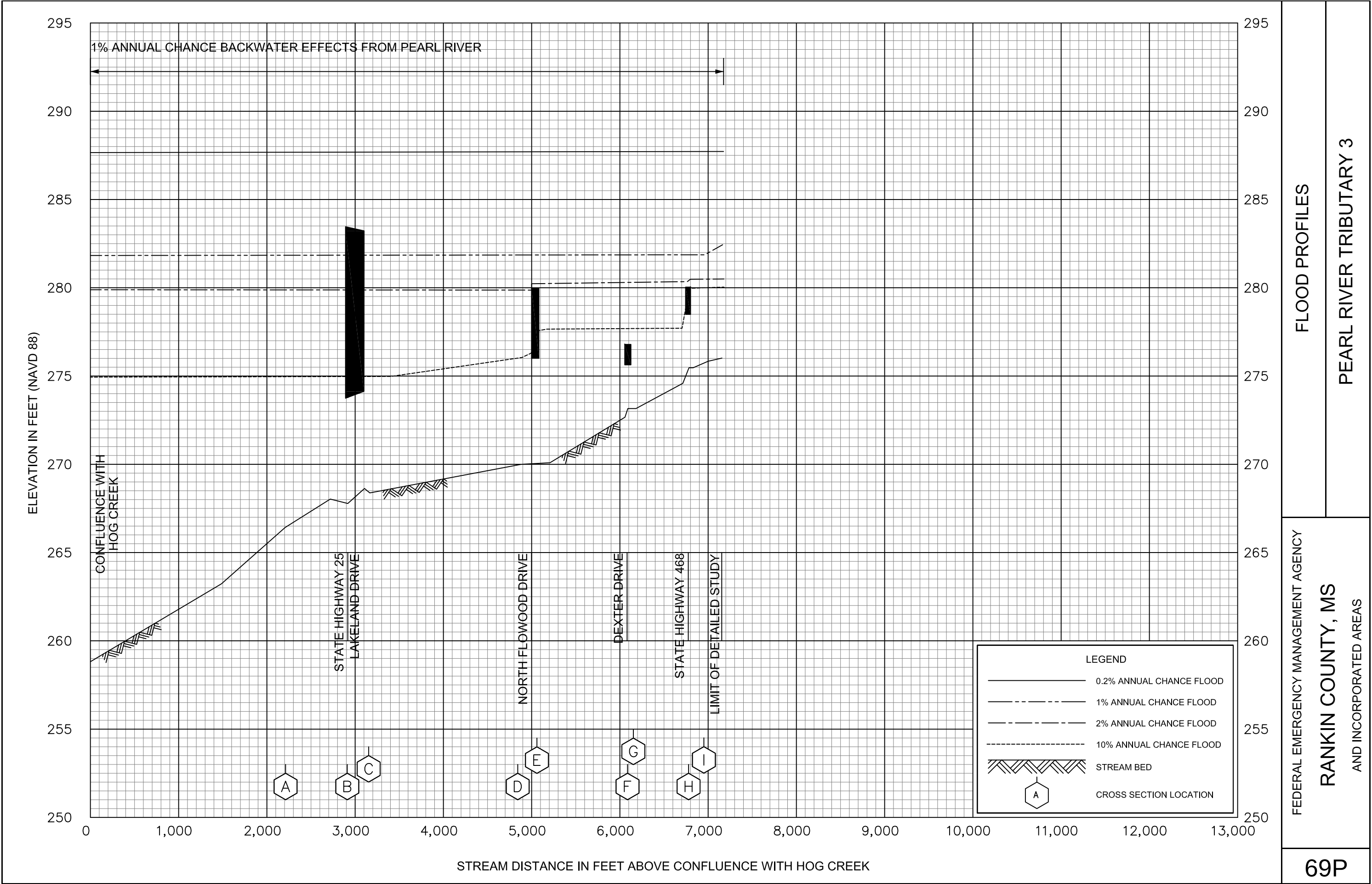










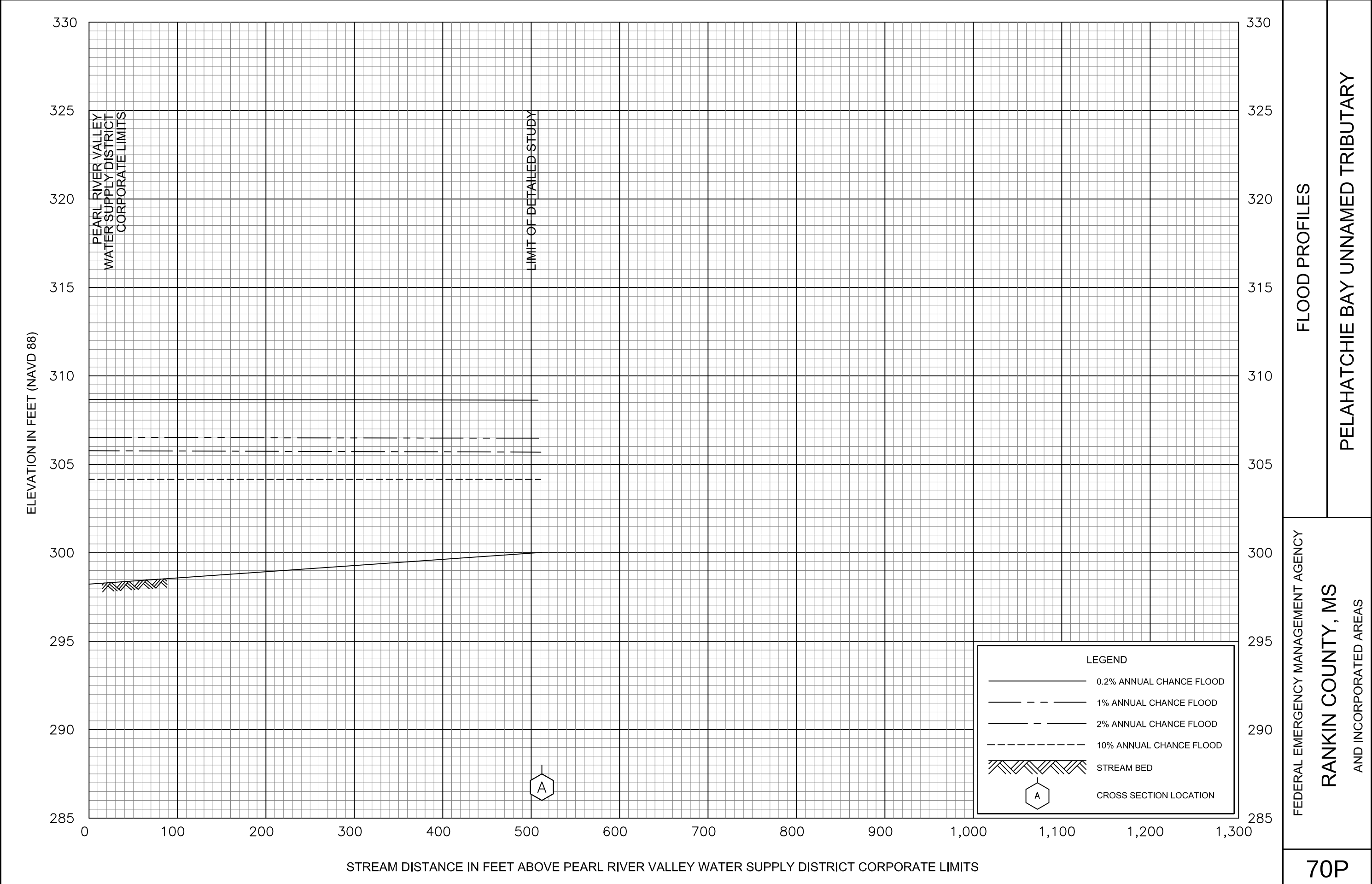


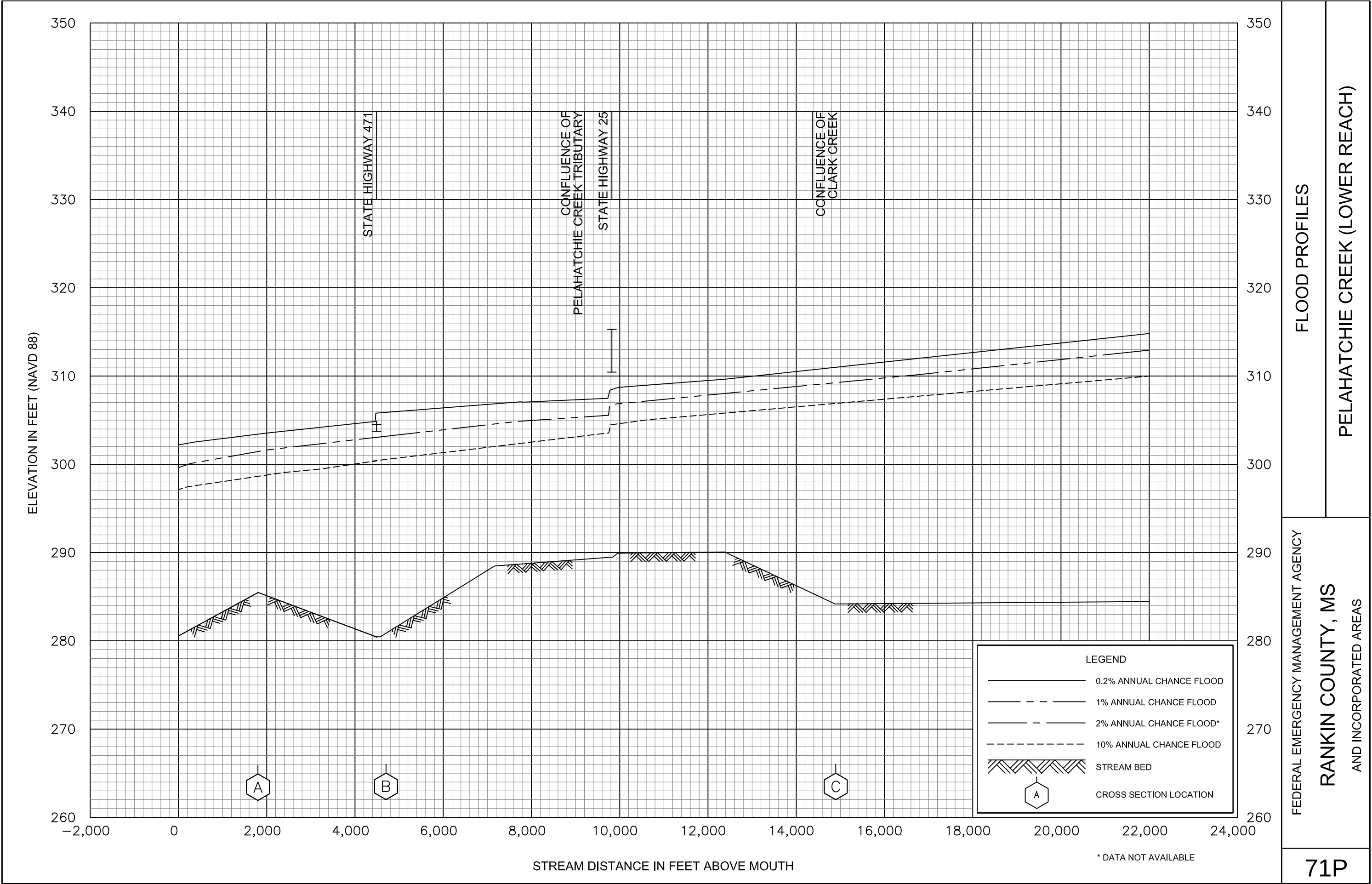
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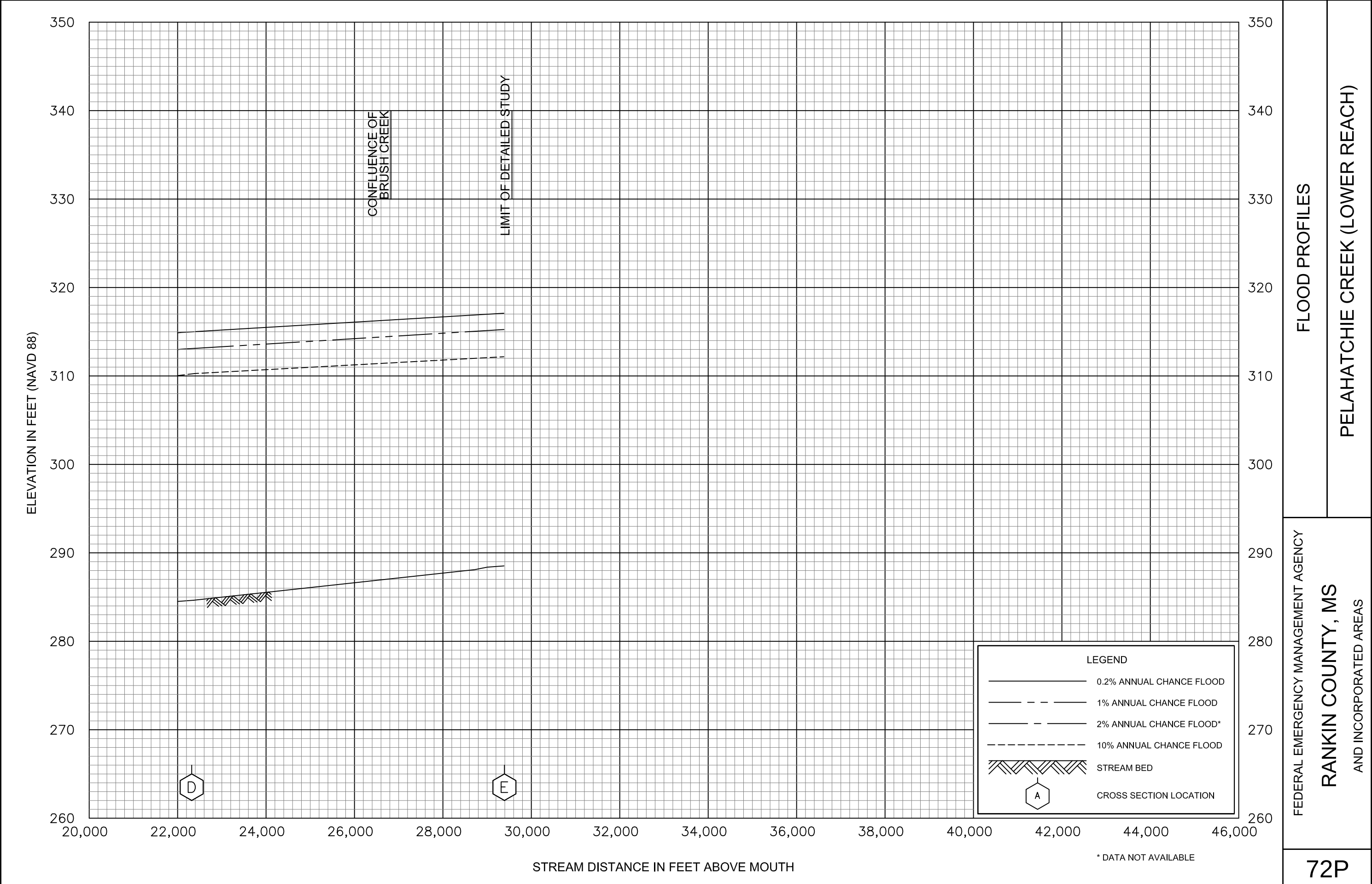
FLOOD PROFILES

RANKIN COUNTY, MS
AND INCORPORATED AREAS

PEARL RIVER TRIBUTARY 3





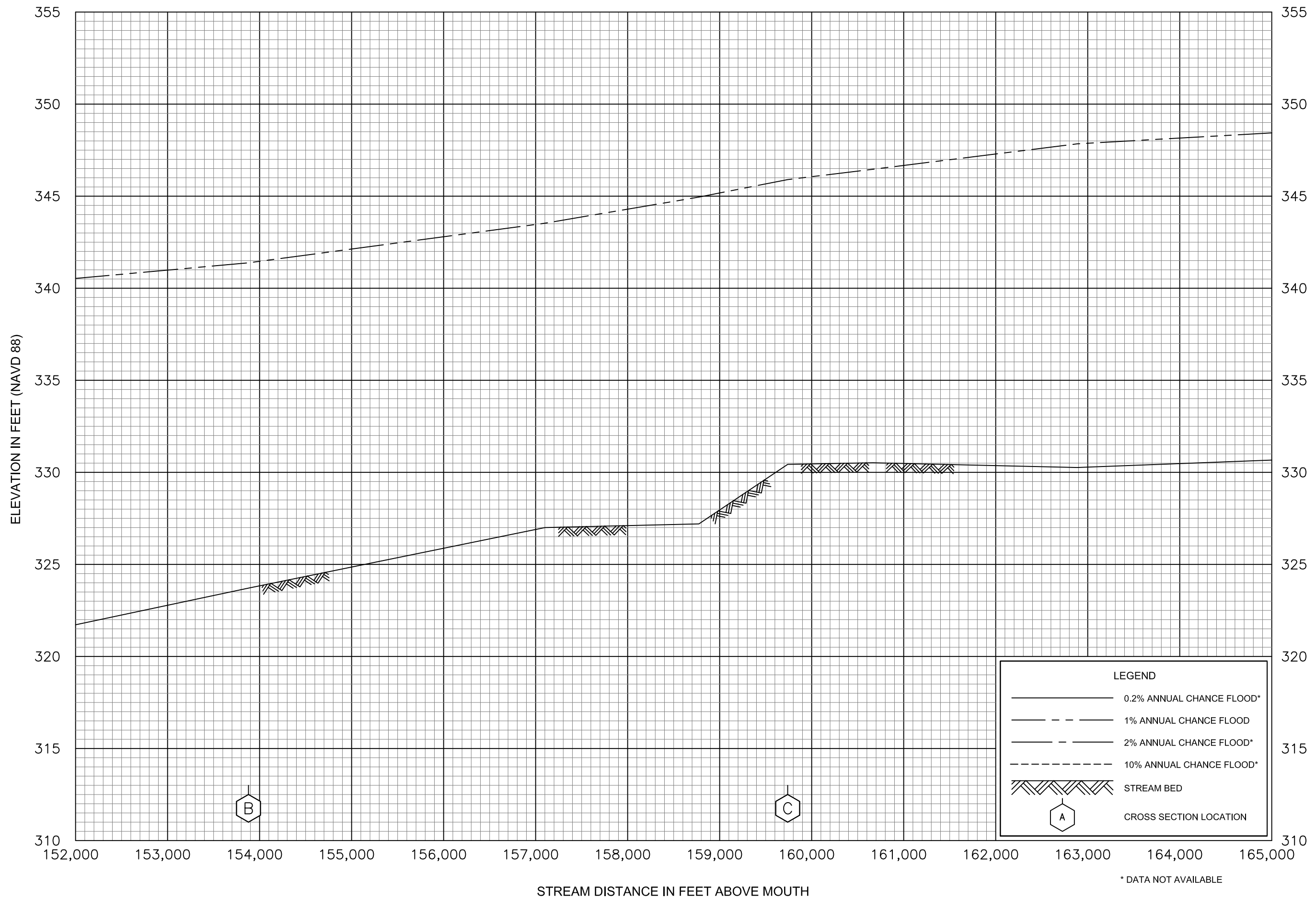


FEDERAL EMERGENCY MANAGEMENT AGENCY

FLOOD PROFILES

RANKIN COUNTY, MS
AND INCORPORATED AREAS

PELAHATCHIE CREEK (LOWER REACH)



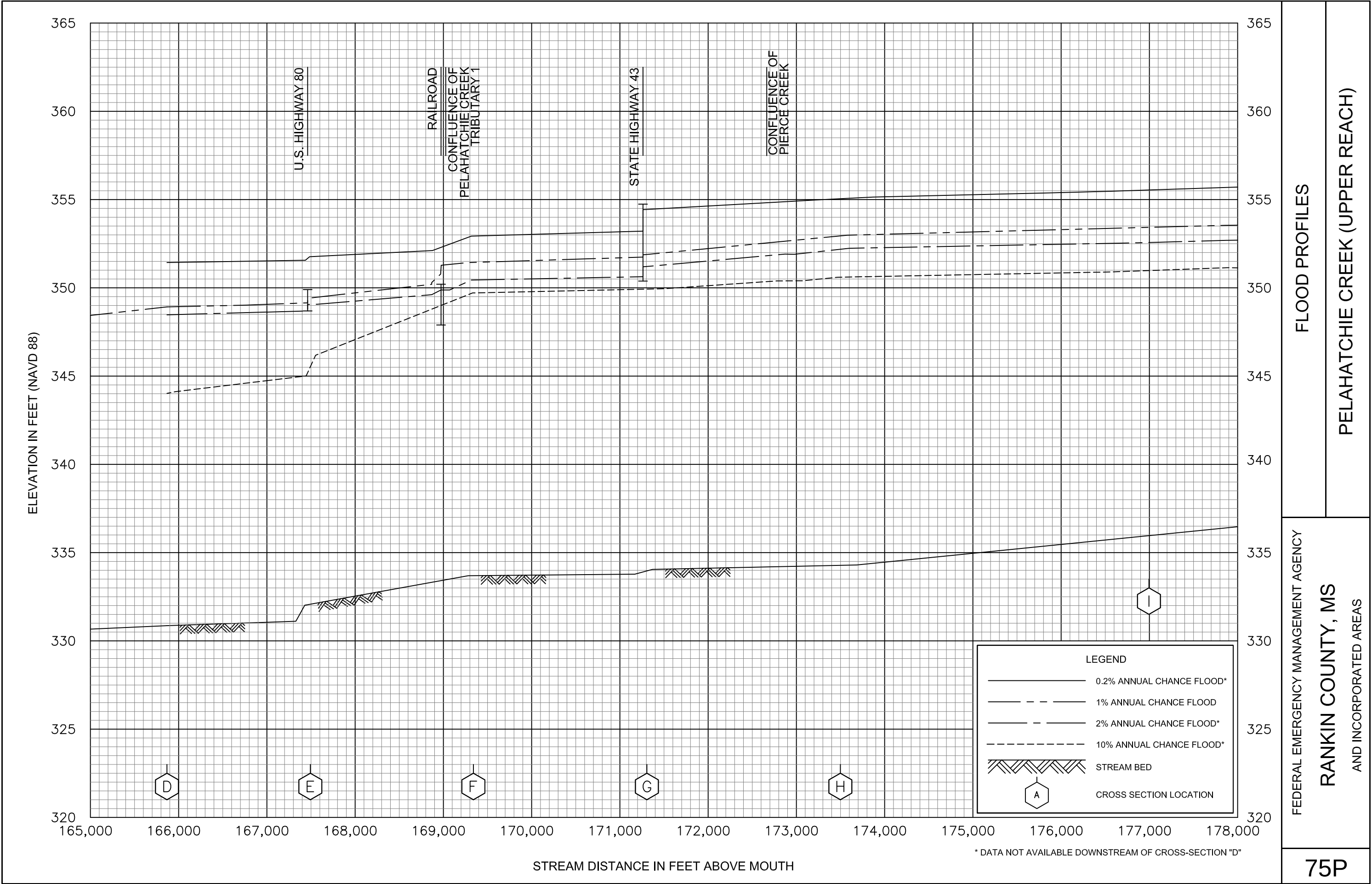
FEDERAL EMERGENCY MANAGEMENT AGENCY

FLOOD PROFILES

PELAHATCHIE CREEK (UPPER REACH)

RANKIN COUNTY, MS AND INCORPORATED AREAS

74P



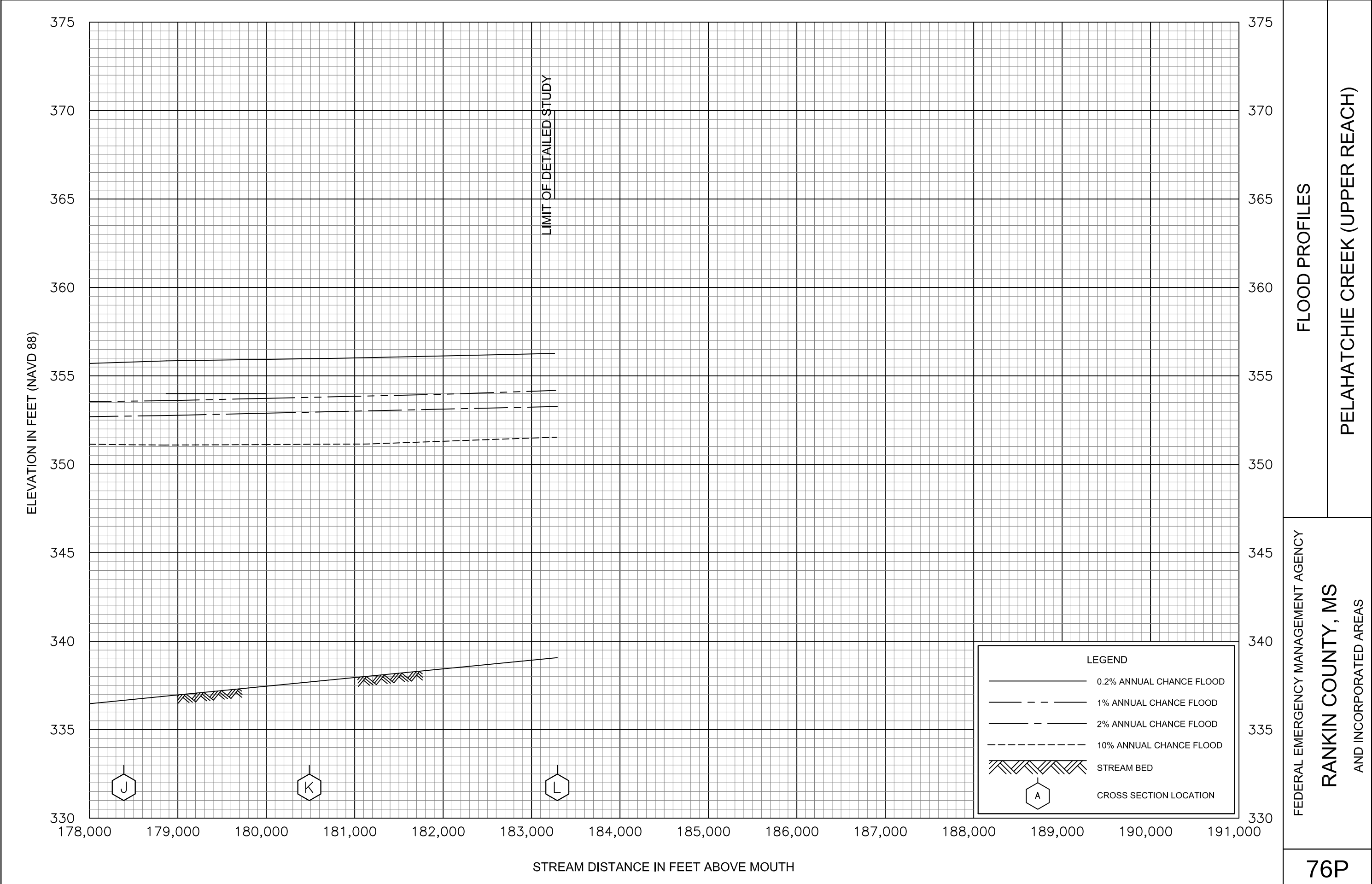
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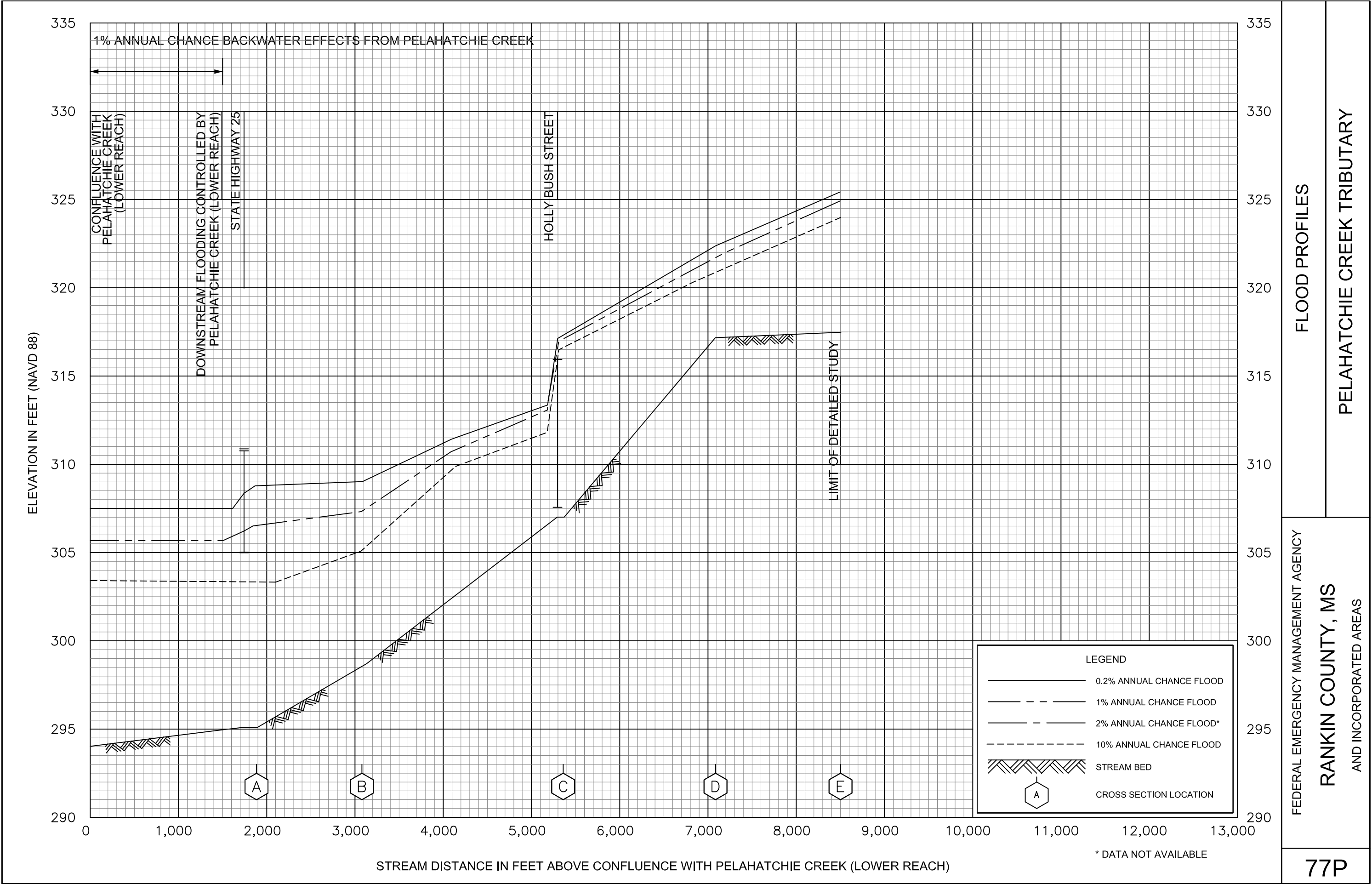
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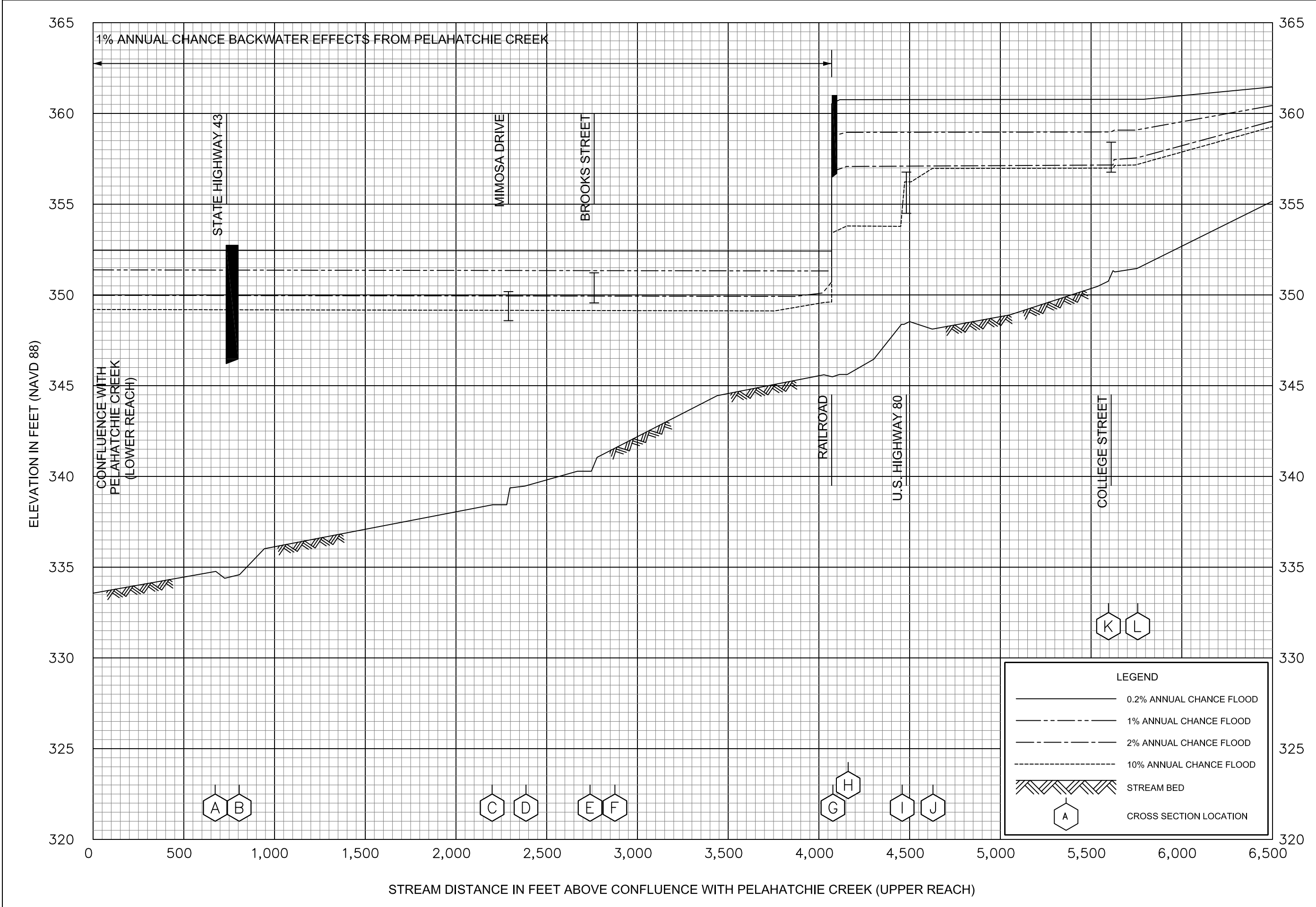
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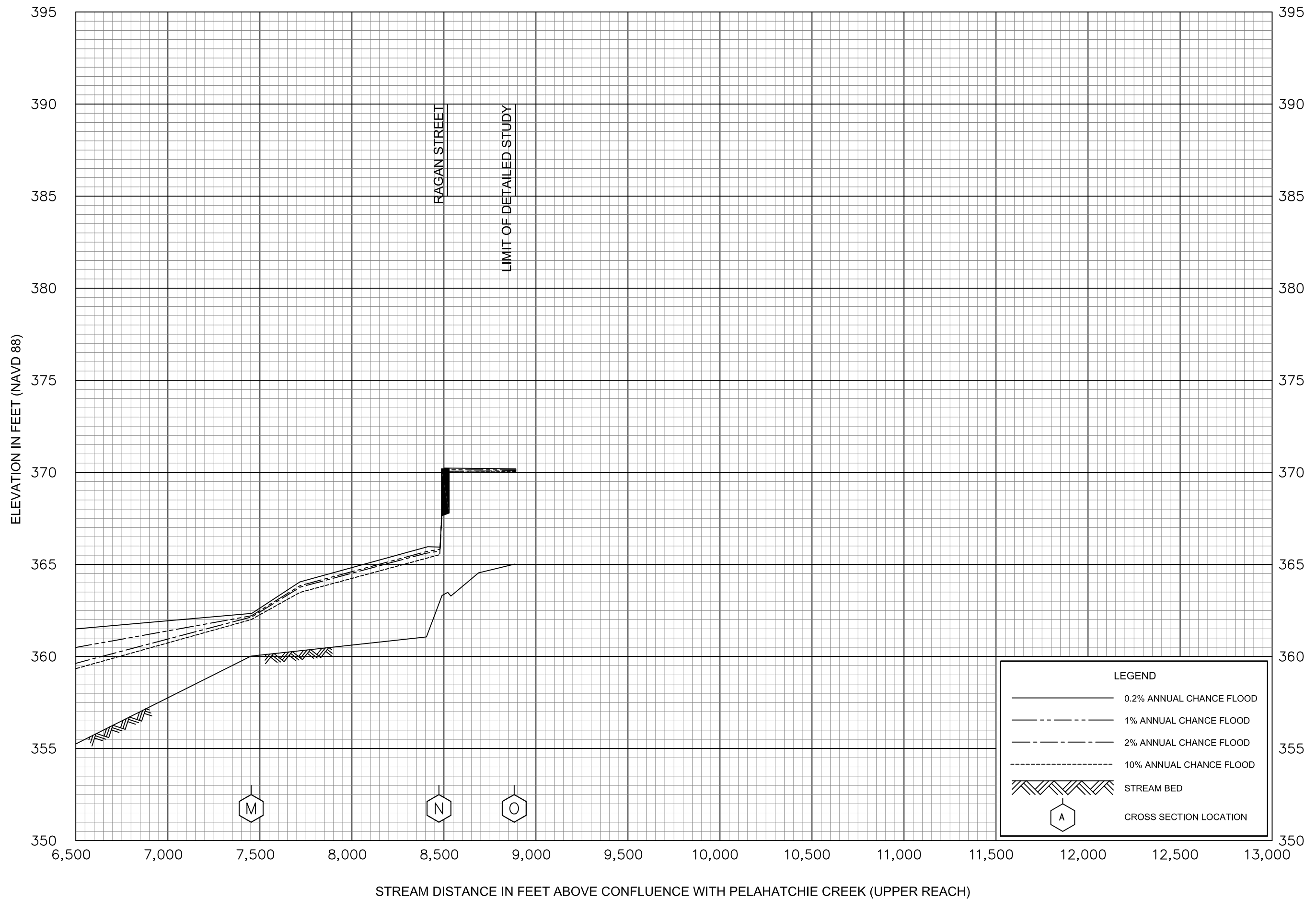
RANKIN COUNTY, MS

AND INCORPORATED AREAS









FEDERAL EMERGENCY MANAGEMENT AGENCY

RANKIN COUNTY, MS

AND INCORPORATED AREAS

FLOOD PROFILES

PELAHATCHIE CREEK TRIBUTARY 1

79P

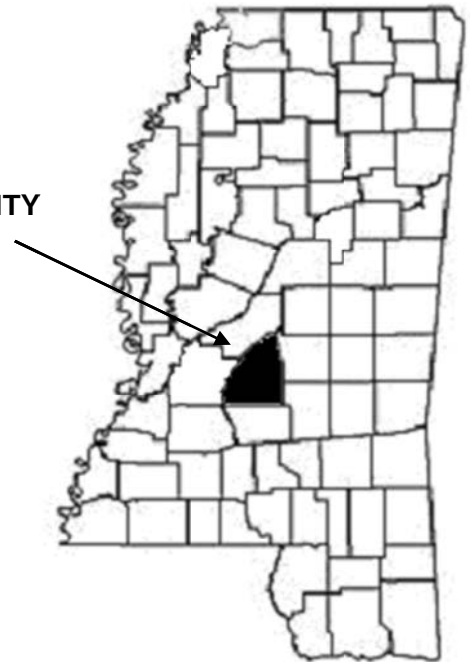
FLOOD INSURANCE STUDY



RANKIN COUNTY, MISSISSIPPI AND INCORPORATED AREAS

VOLUME 2 OF 2

RANKIN COUNTY



COMMUNITY NAME	COMMUNITY NUMBER
BRANDON, CITY OF	280143
FLORENCE, CITY OF	280144
FLOWOOD, CITY OF	280289
JACKSON, CITY OF	280072
PEARL, CITY OF	280145
PEARL RIVER VALLEY WATER SUPPLY DISTRICT	280338
PELAHATCHIE, TOWN OF	280146
PUCKETT, TOWNSHIP OF	280147
RANKIN COUNTY (UNINCORPORATED AREAS)	280142
RICHLAND, CITY OF	280299

REVISED:



Federal Emergency Management Agency

FLOOD INSURANCE STUDY NUMBER
28121CV002B

NOTICE TO
FLOOD INSURANCE STUDY USERS

Communities participating in the National Flood Insurance Program (NFIP) have established repositories of flood hazard data for floodplain management and flood insurance purposes. This Flood Insurance Study (FIS) may not contain all data available within the repository. It is advisable to contact the community repository for any additional data.

Part or all of this FIS may be revised and republished at any time. In addition, part of this FIS may be revised by the Letter of Map Revision process, which does not involve republication or redistribution of the FIS. It is, therefore, the responsibility of the user to consult with community officials and to check the community repository to obtain the most current FIS components.

Initial Countywide FIS Effective Date: November 3, 2003

Revised Countywide FIS Date:

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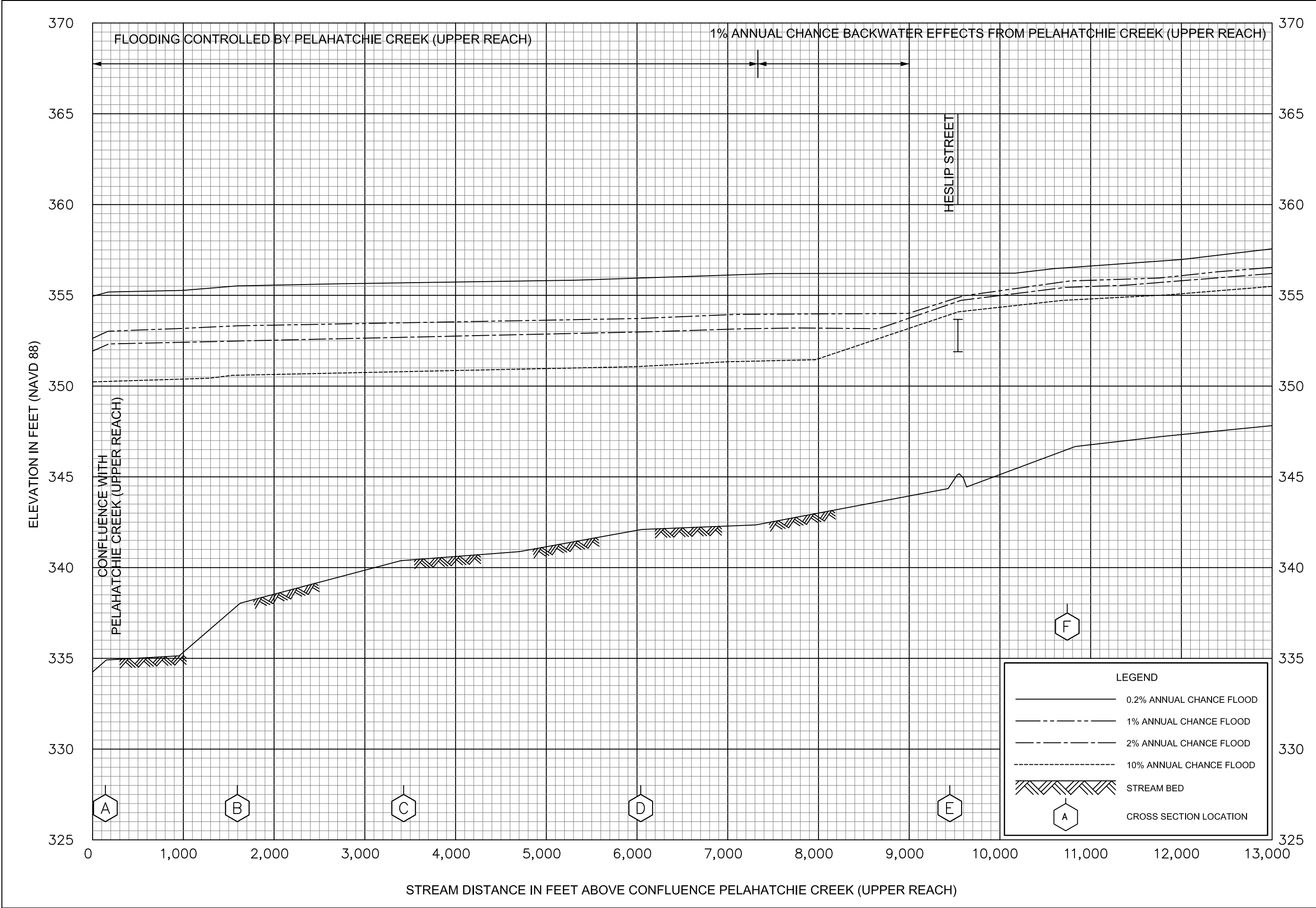
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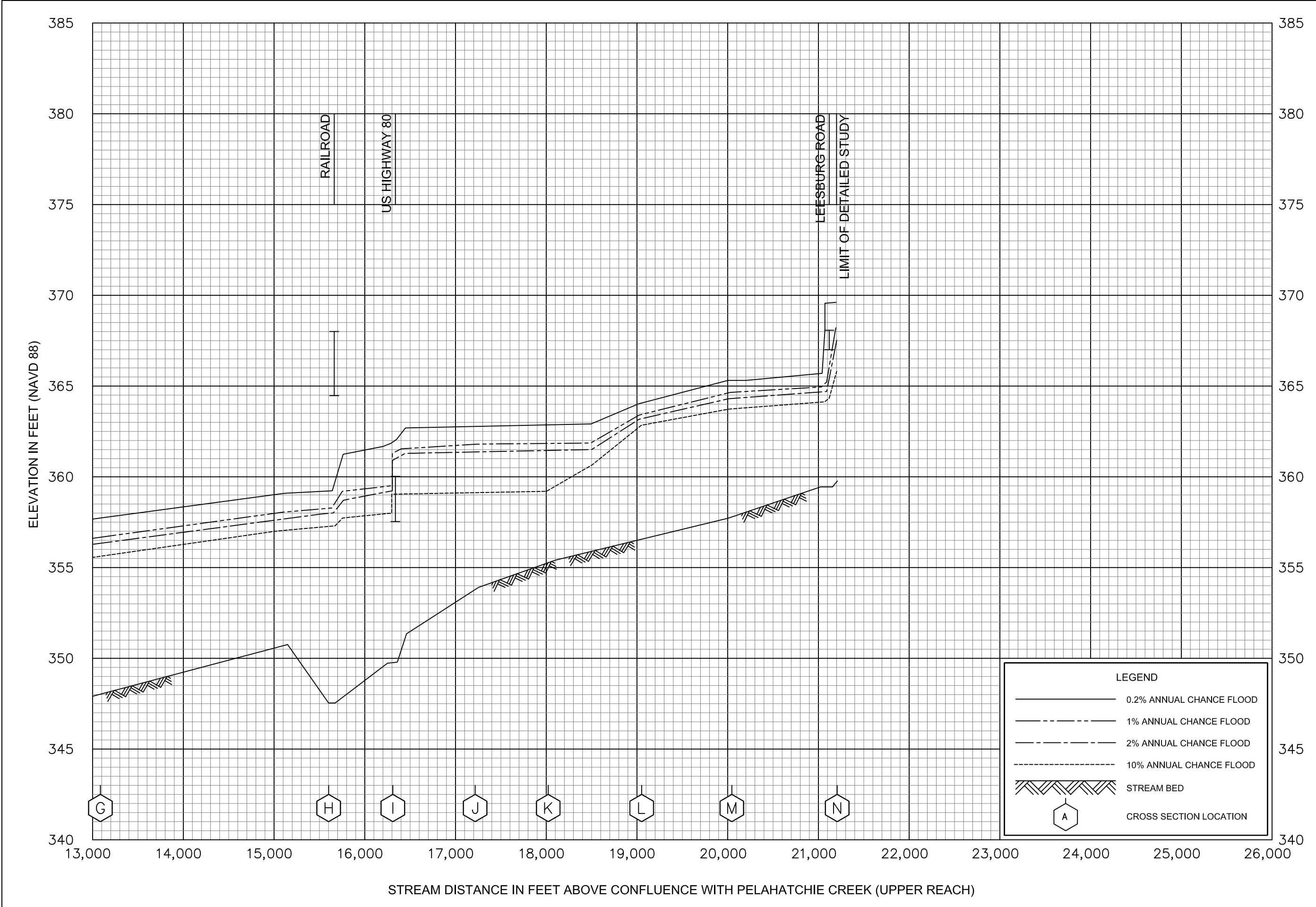
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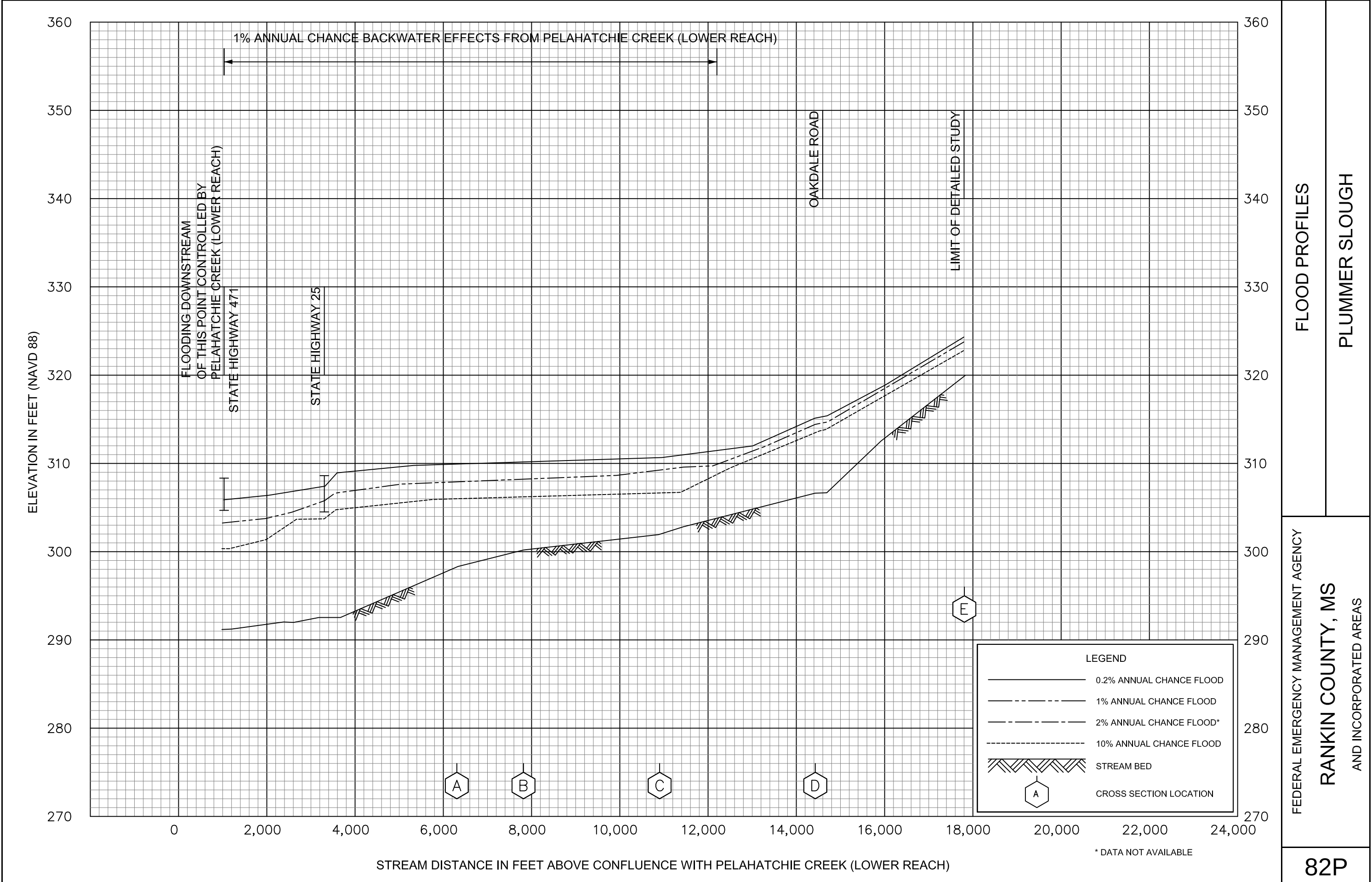
Exhibit 1 - Flood Profiles

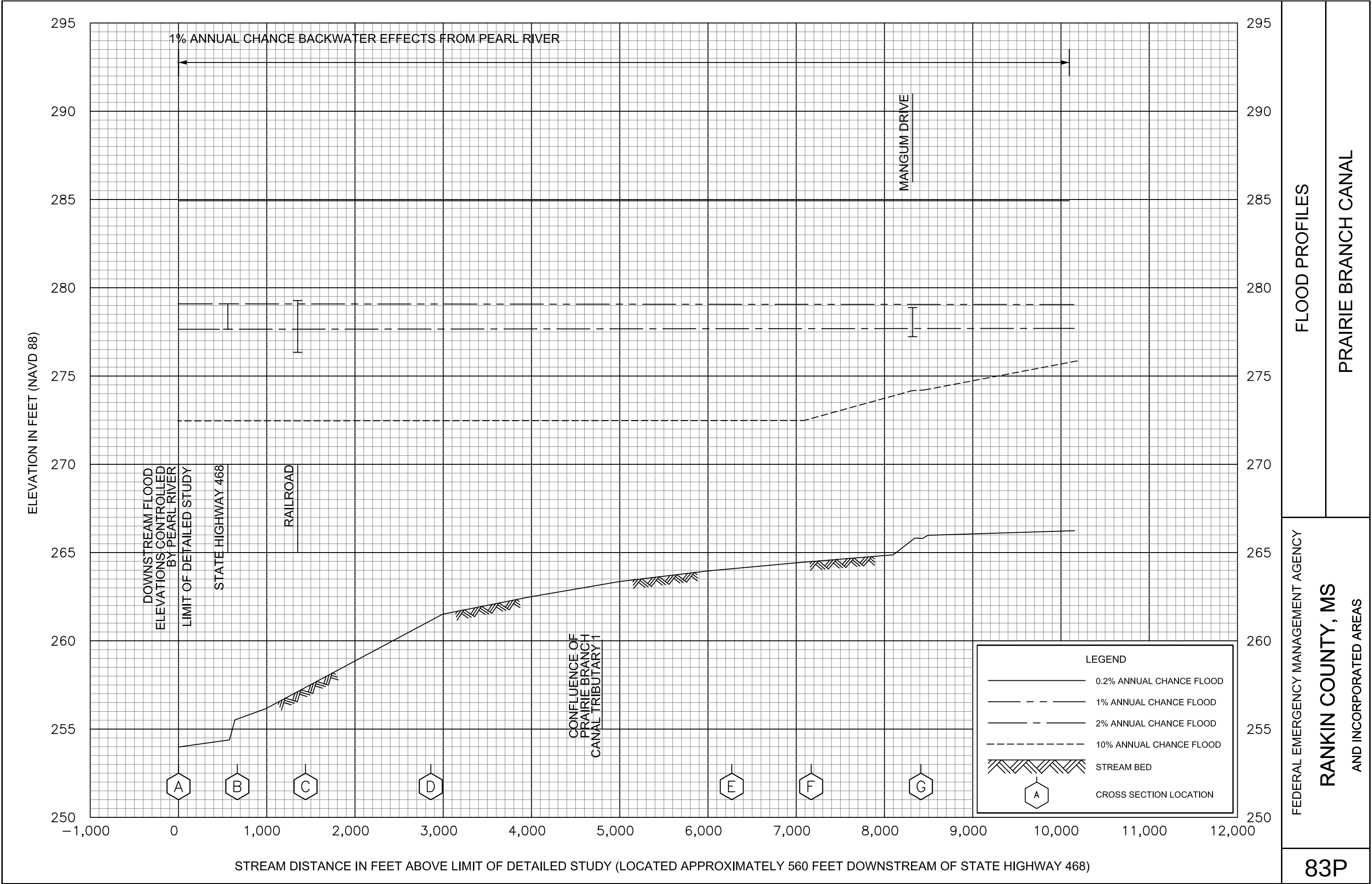
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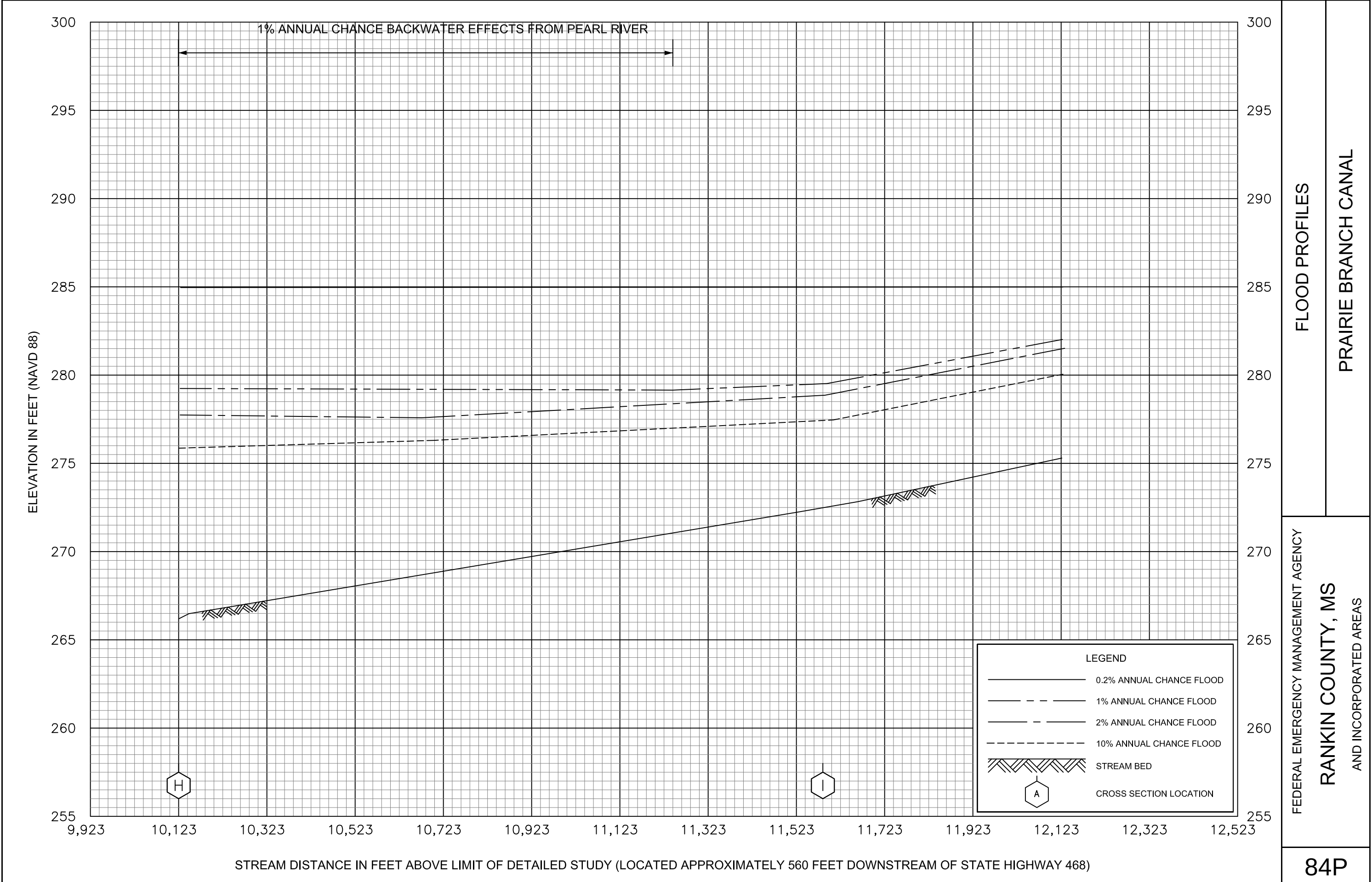
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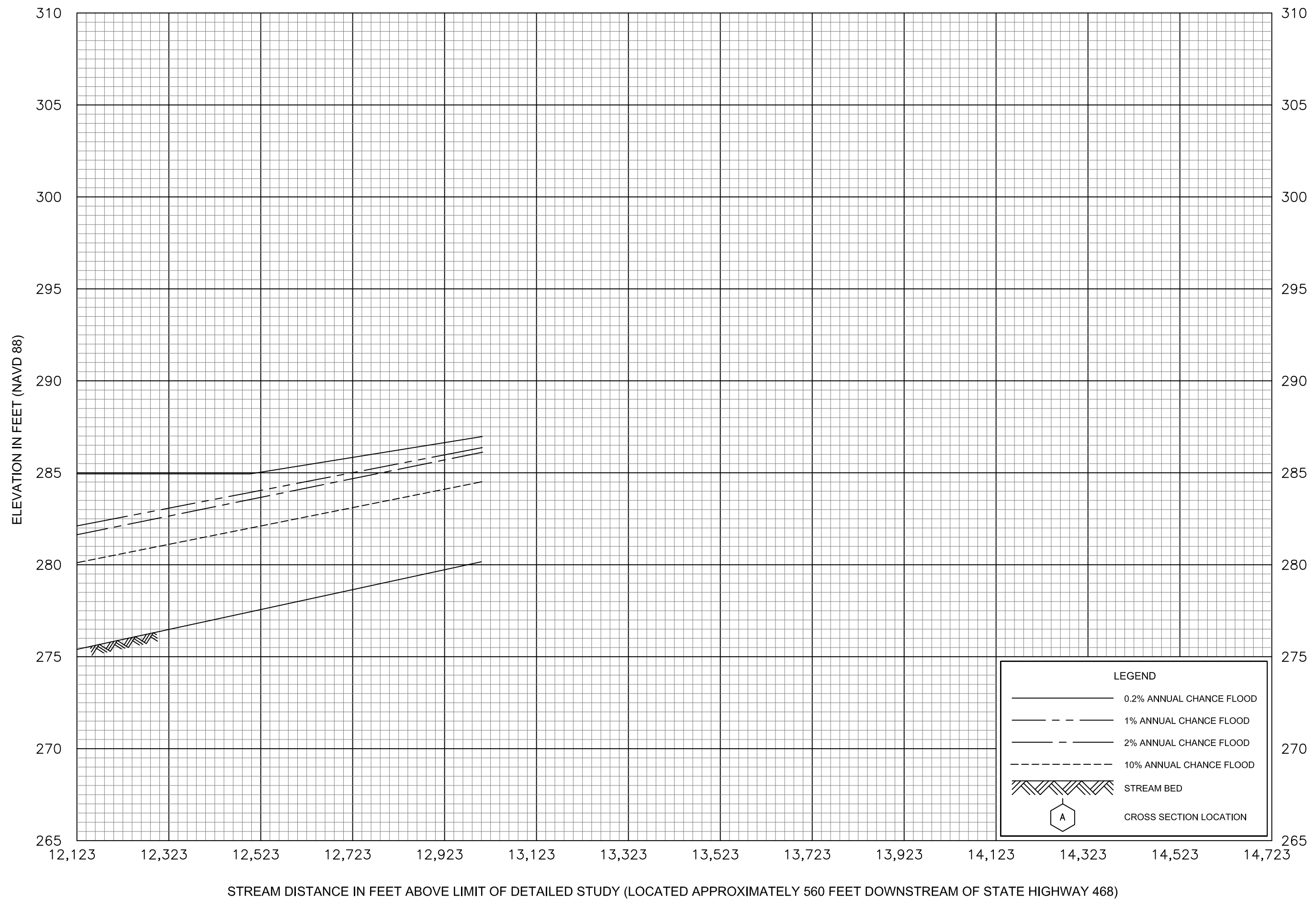












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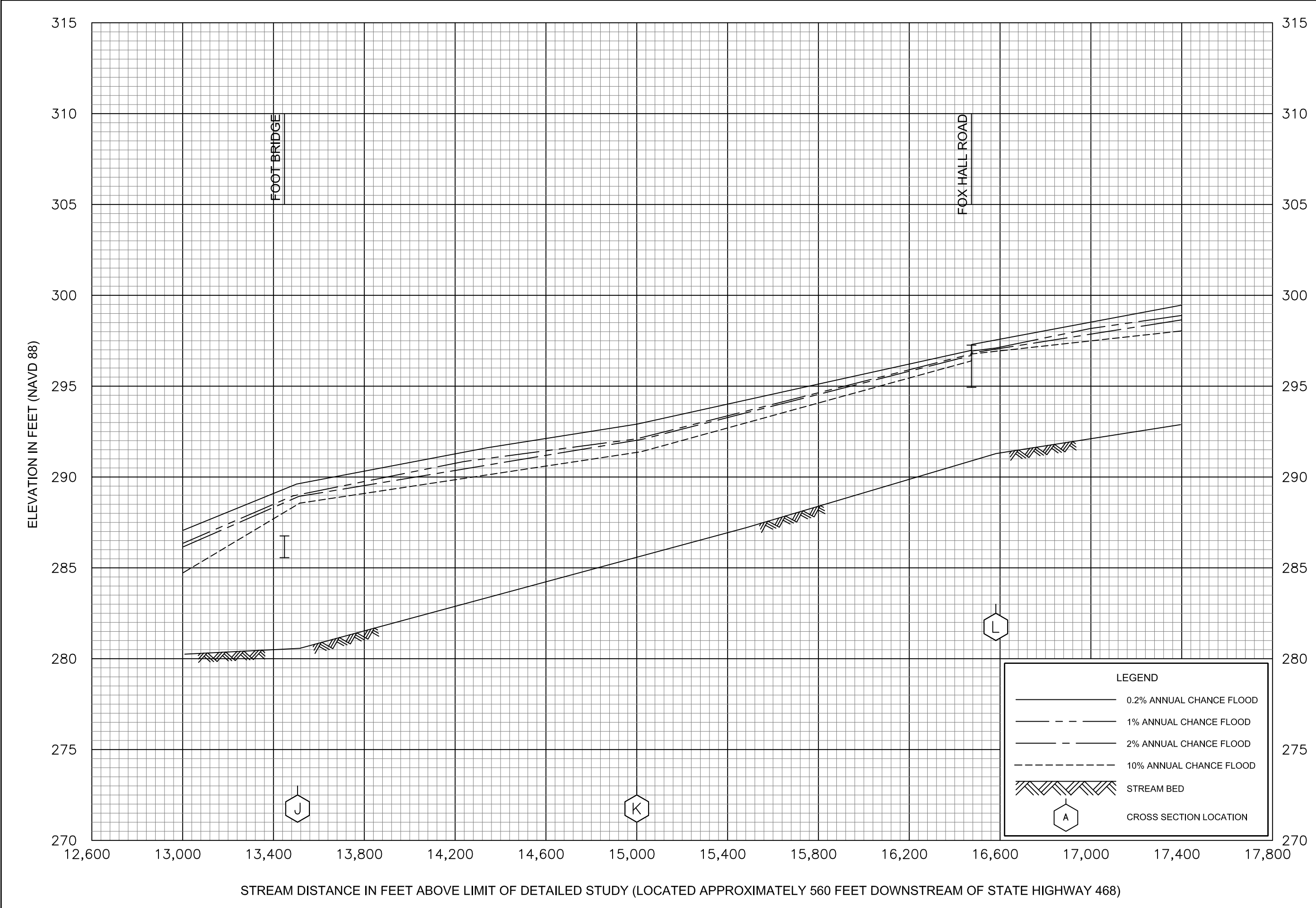
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AND INCORPORATED AREAS

FLOOD PROFILES

PRAIRIE BRANCH CANAL

85P



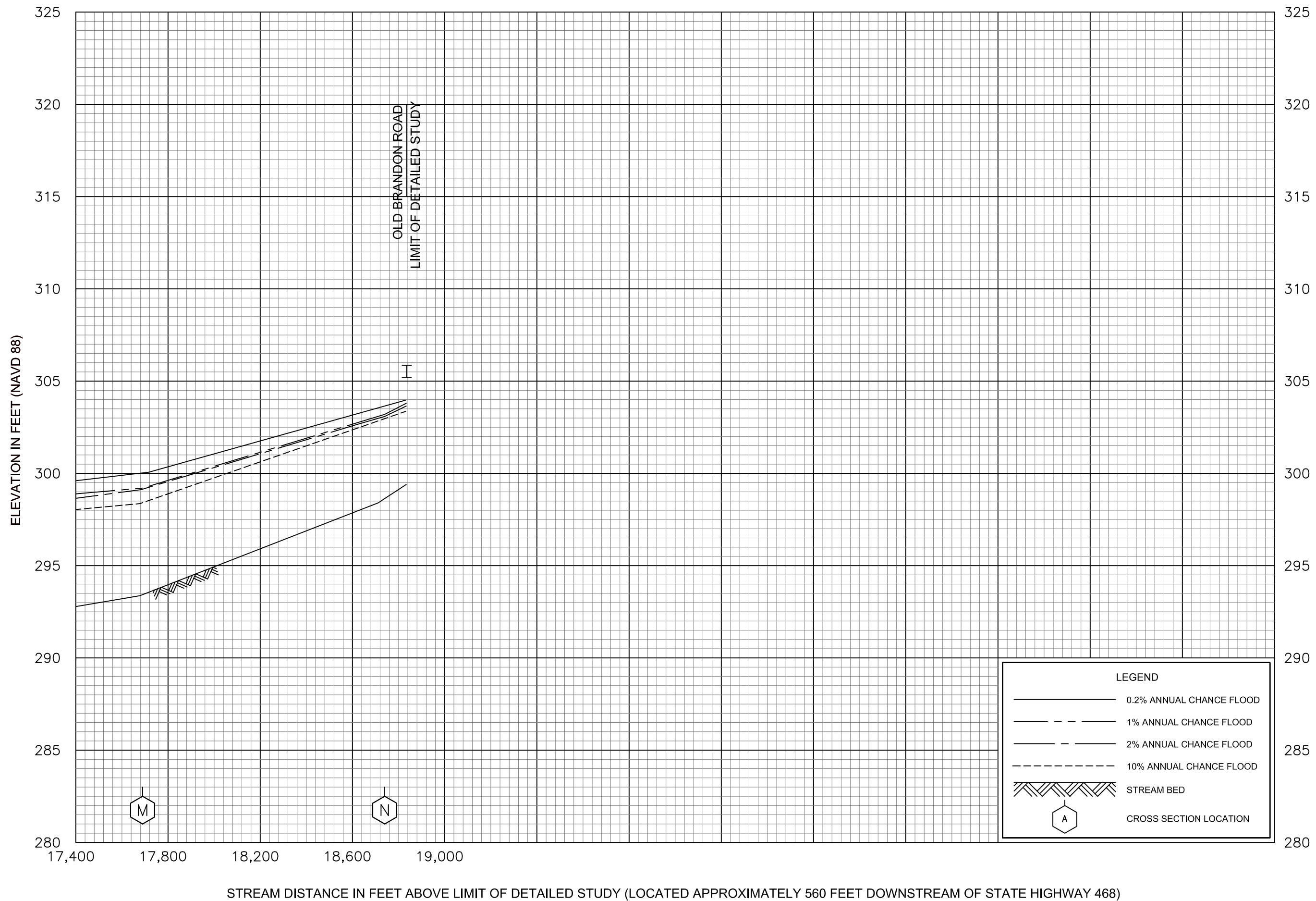
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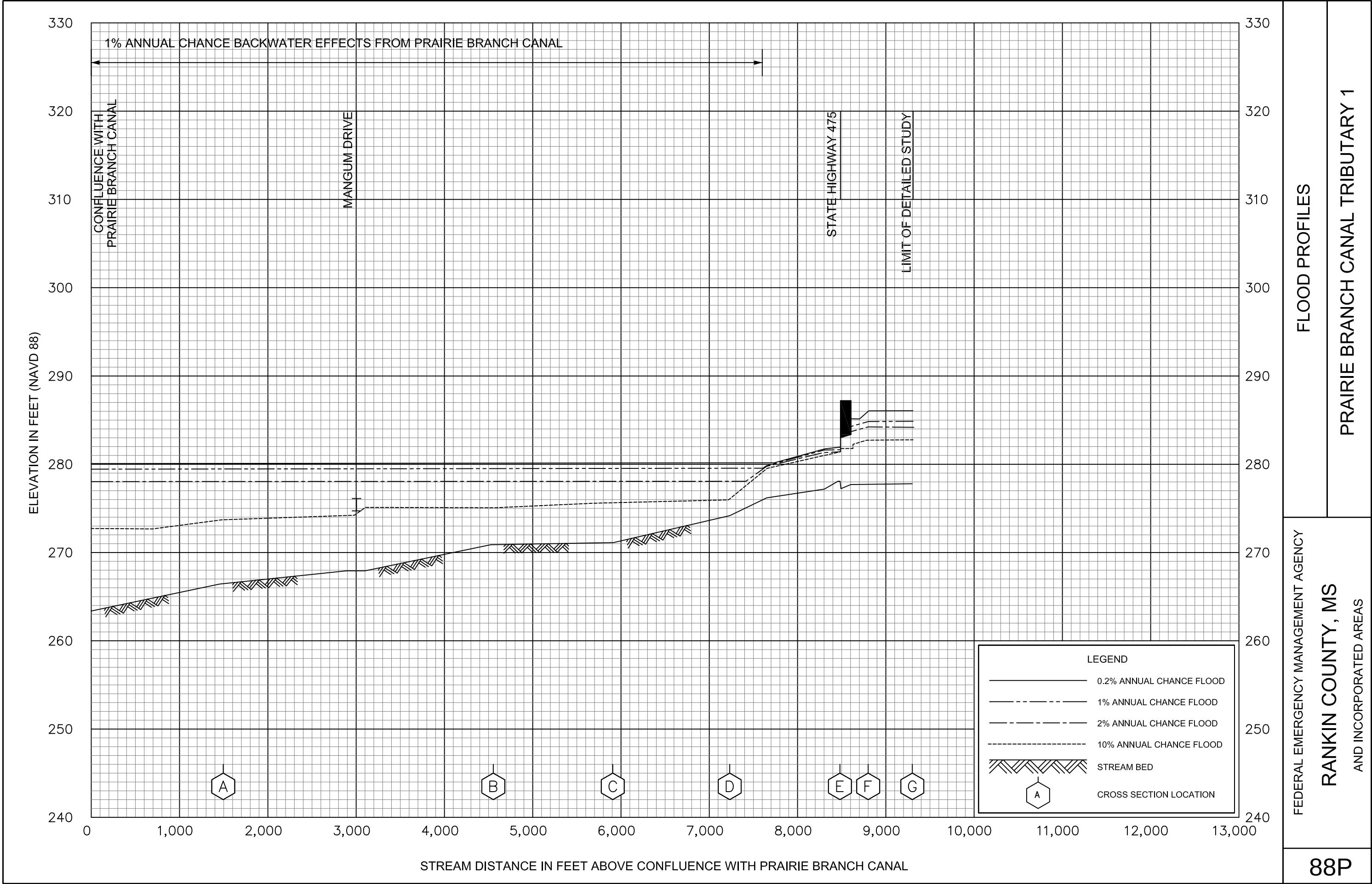
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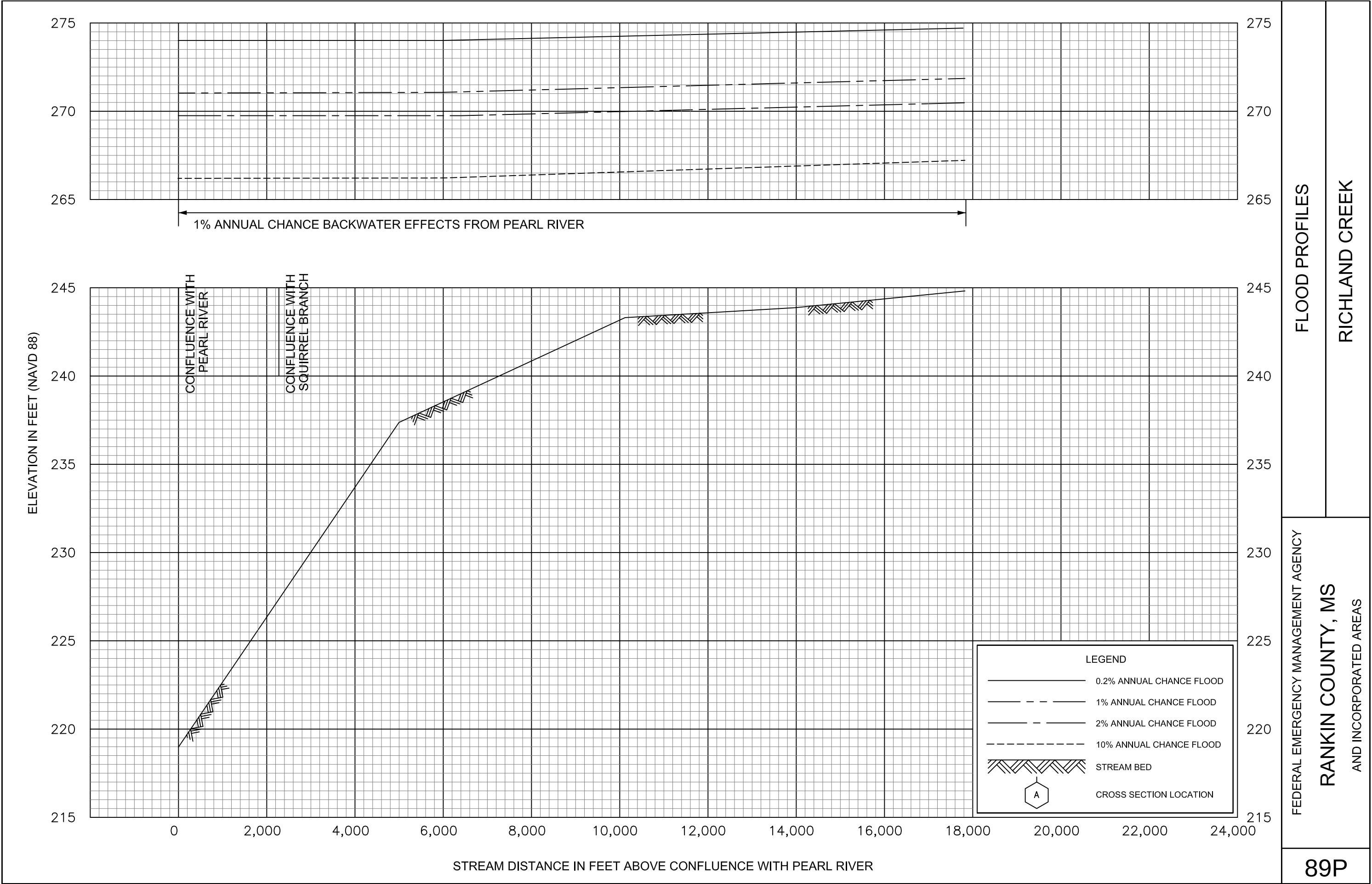
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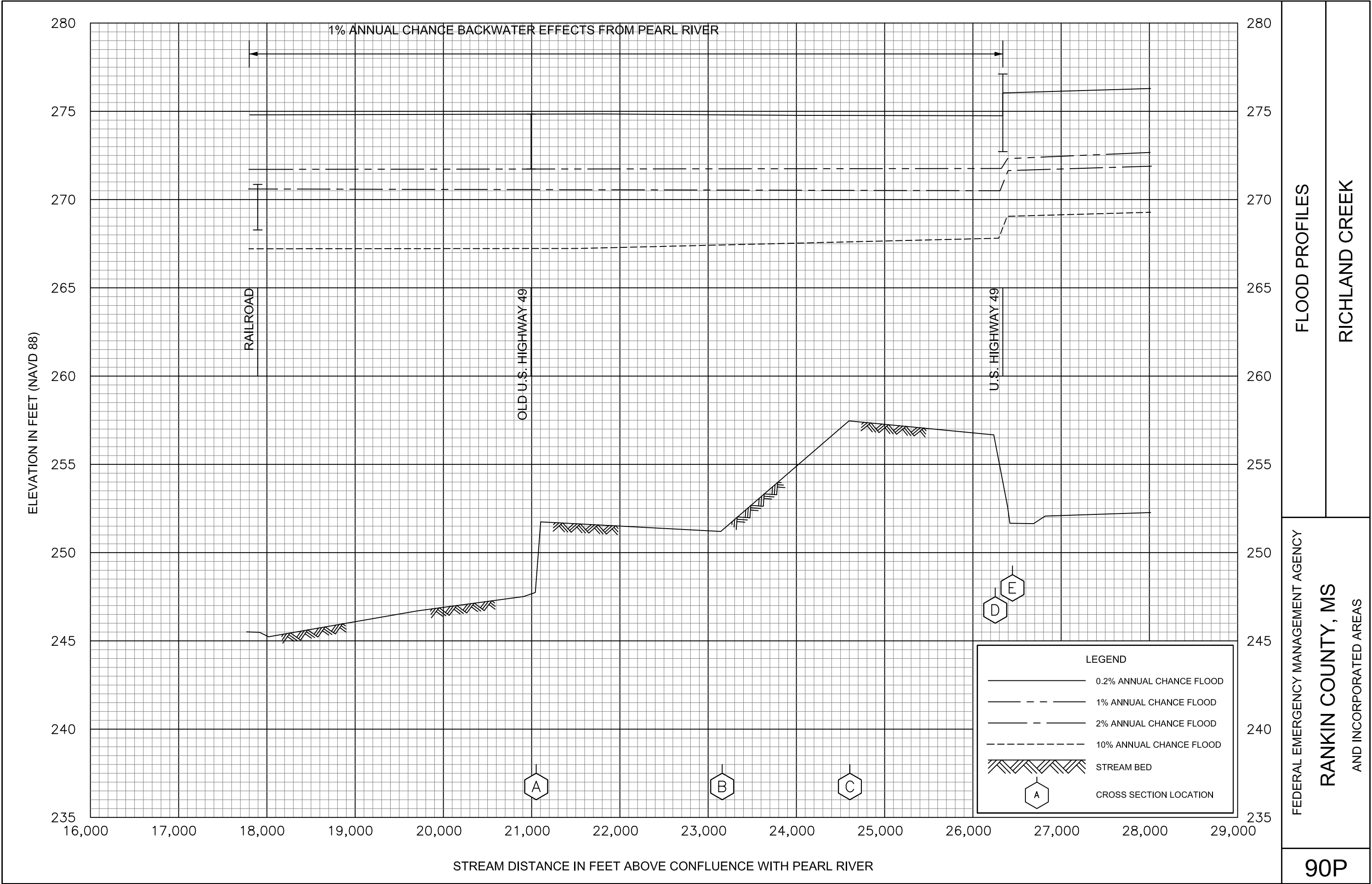
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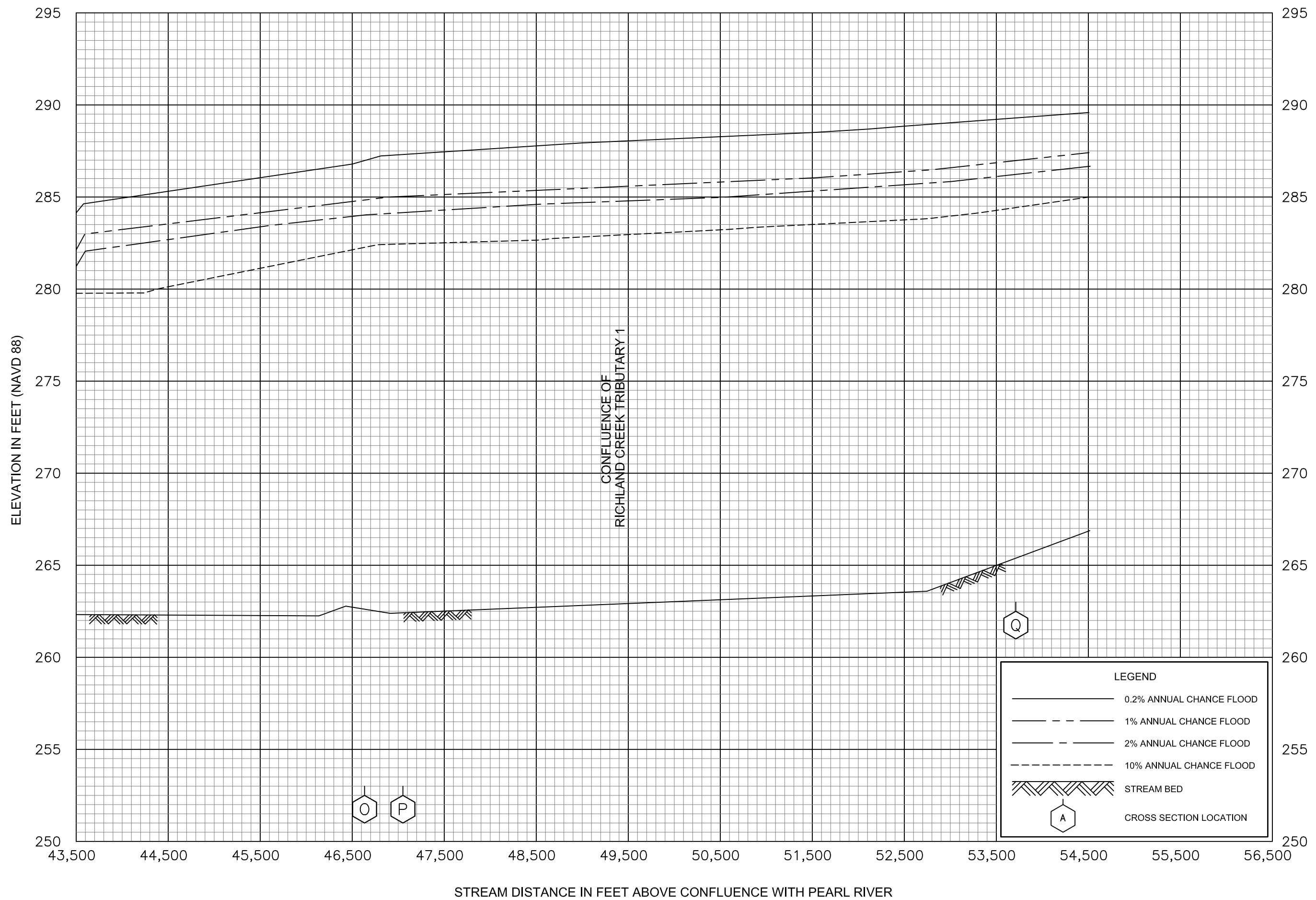
PRAIRIE BRANCH CANAL

87P









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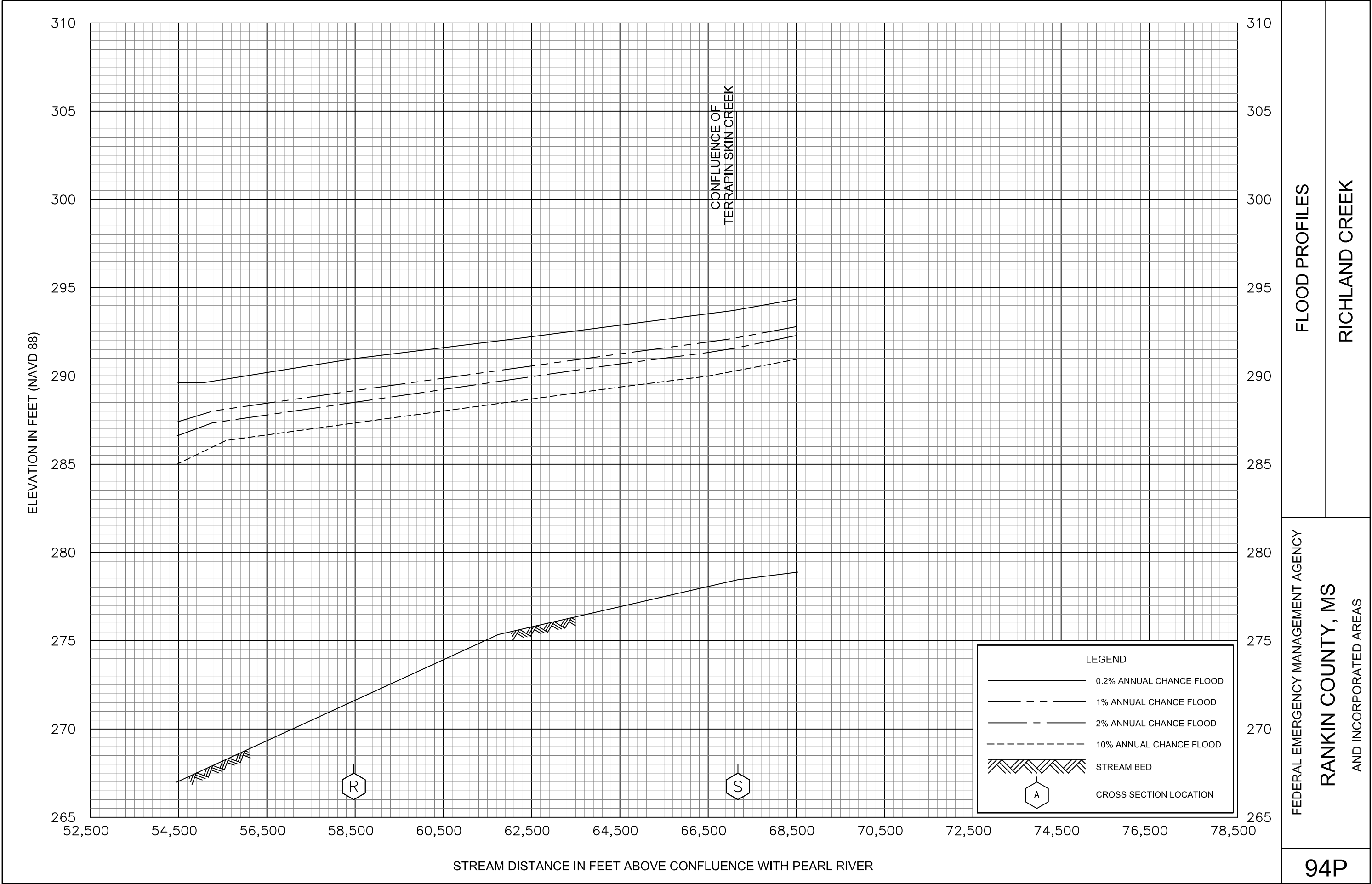
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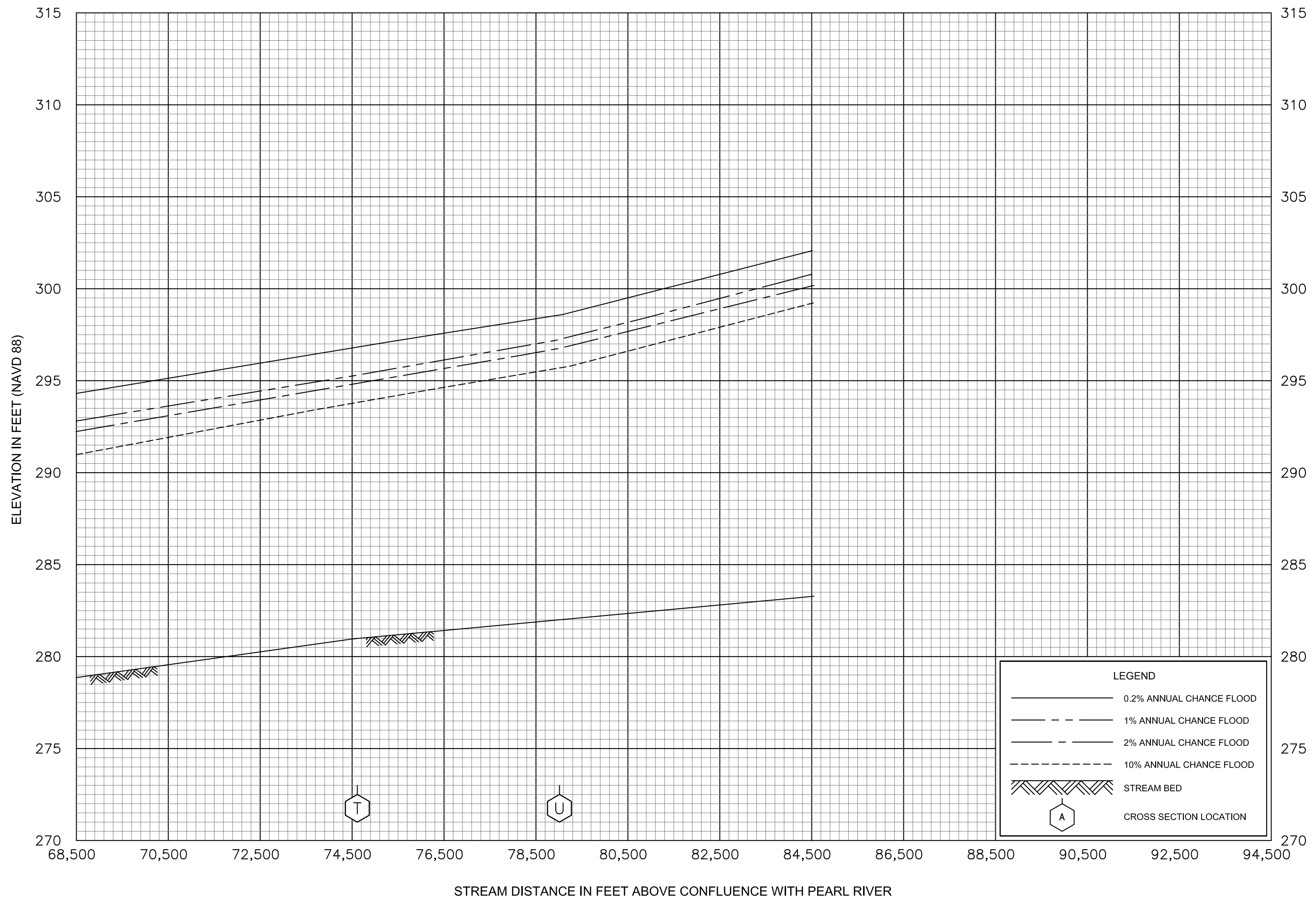
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FLOOD PROFILES

RICHLAND CREEK

93P





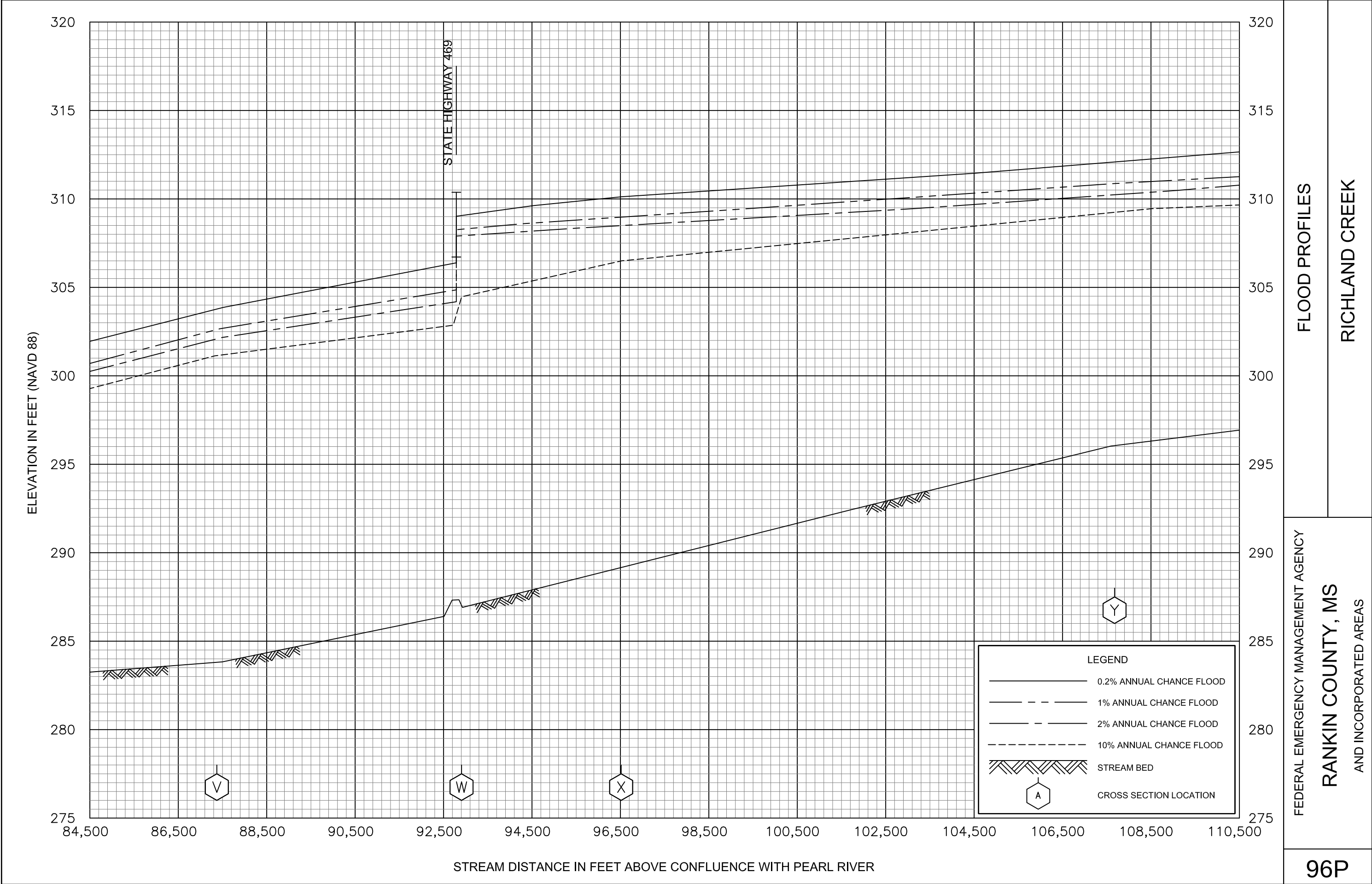
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FLOOD PROFILES

RICHLAND CREEK

95P



FLOOD PROFILES

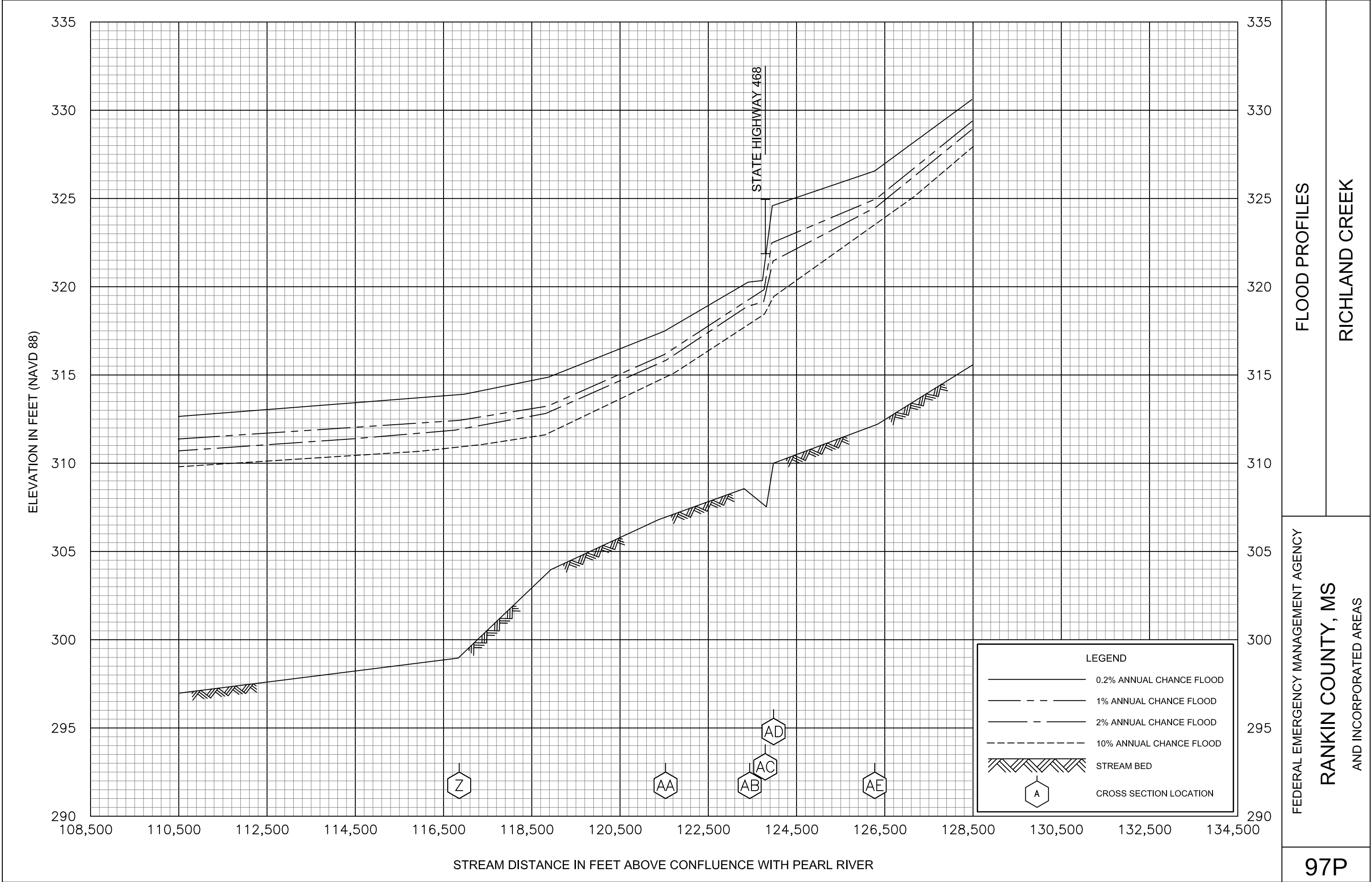
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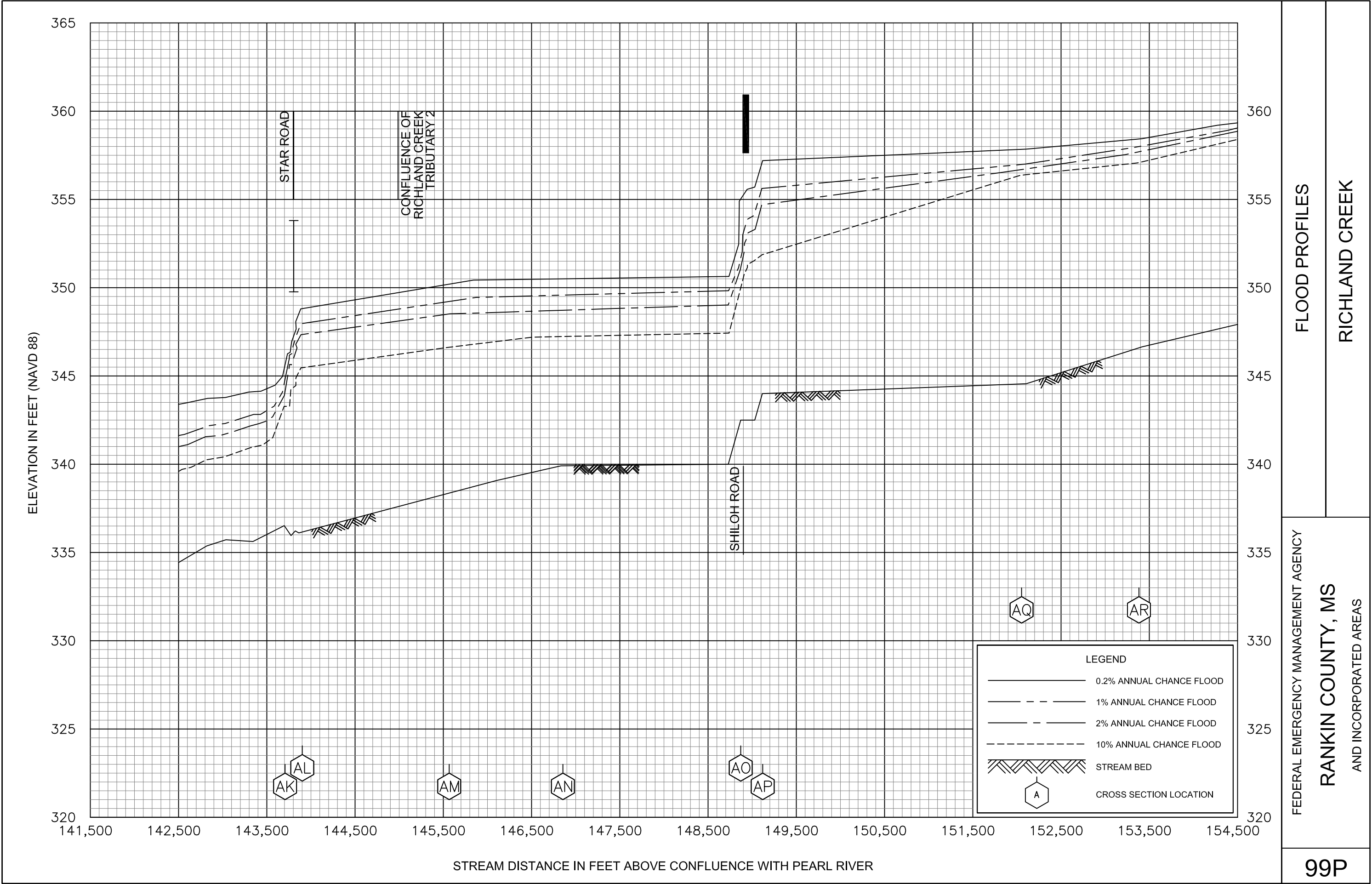
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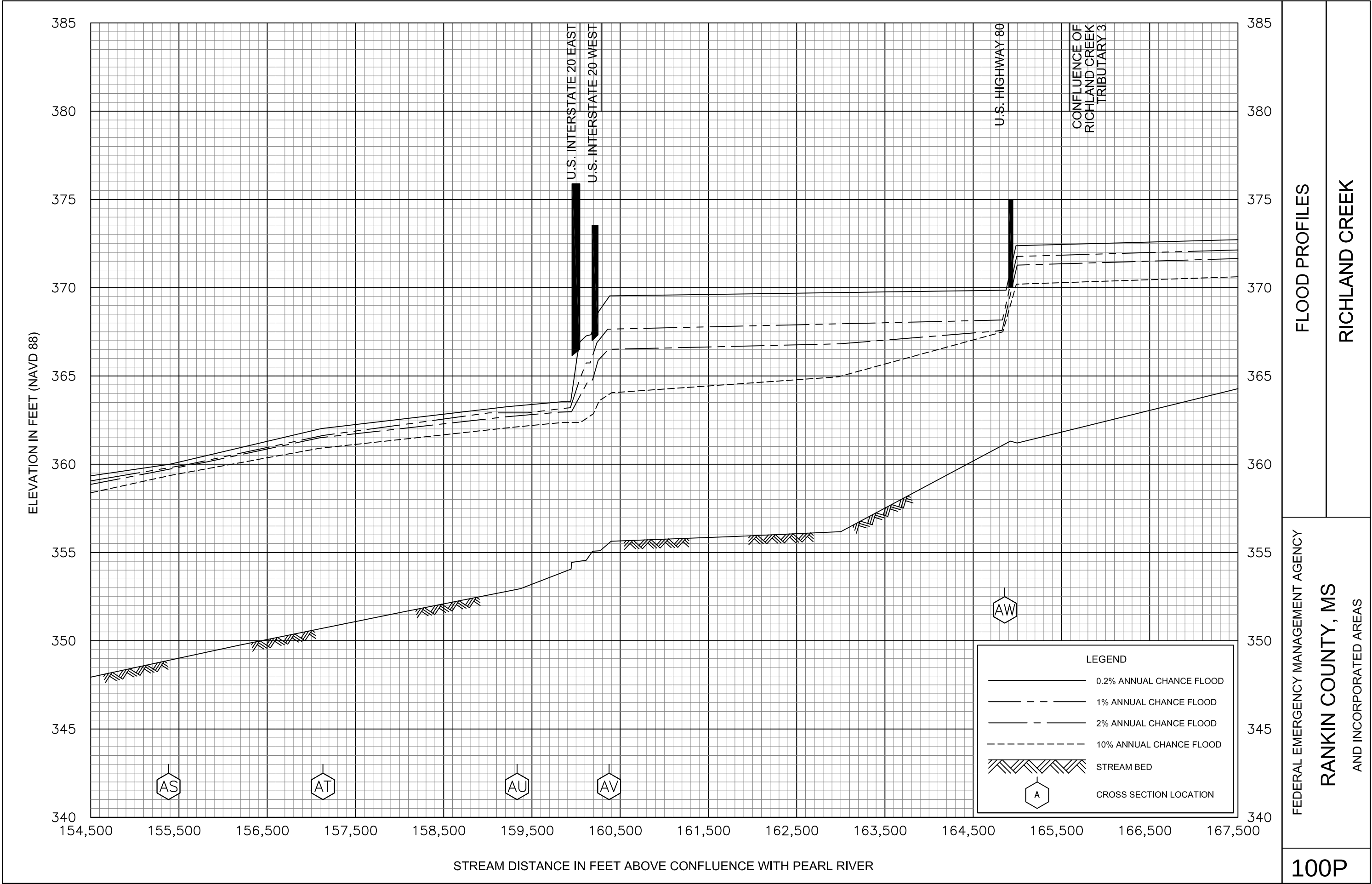
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RICHLAND CREEK

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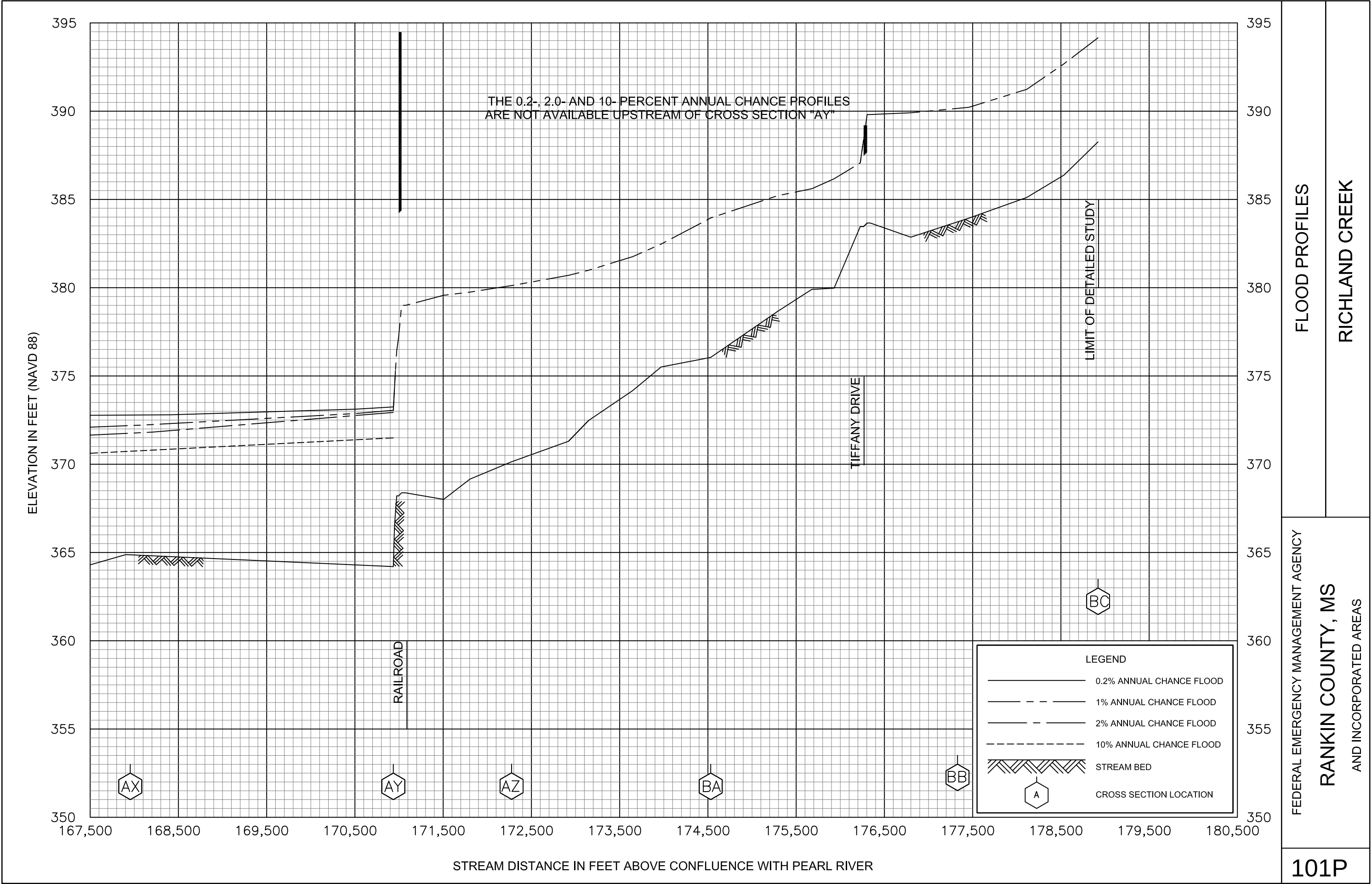
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100P

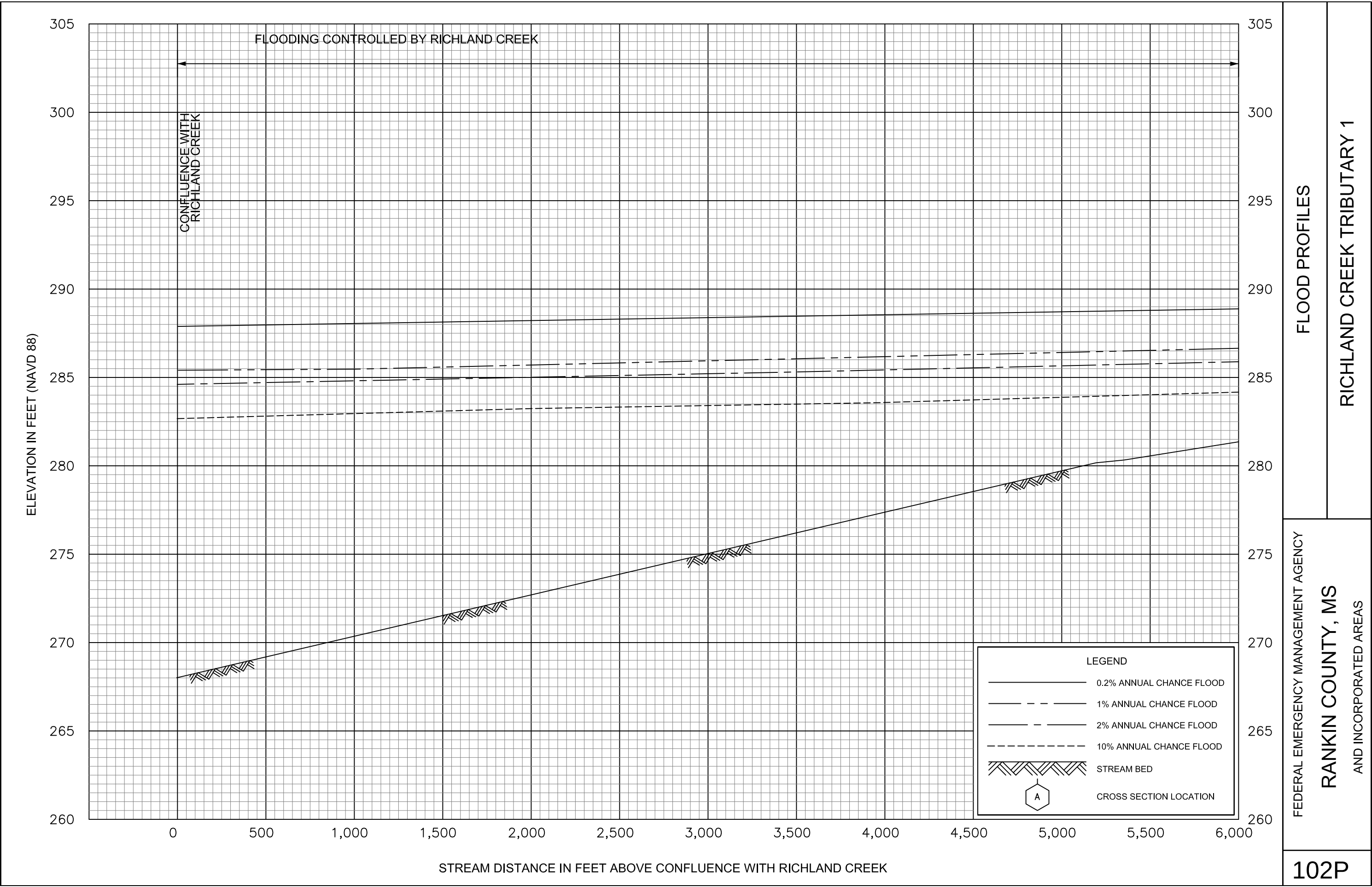


FLOOD PROFILES

RICHLAND CREEK

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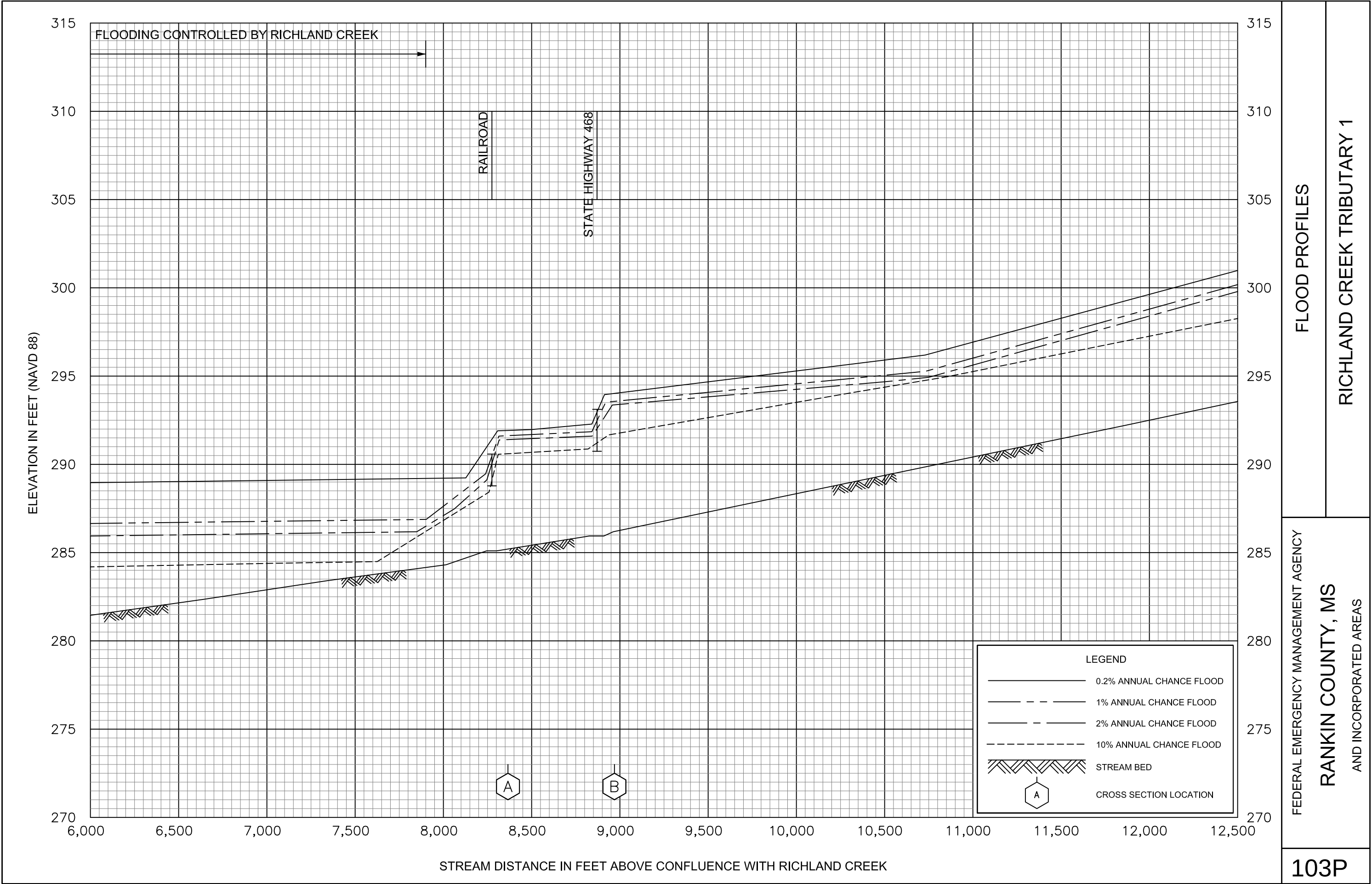
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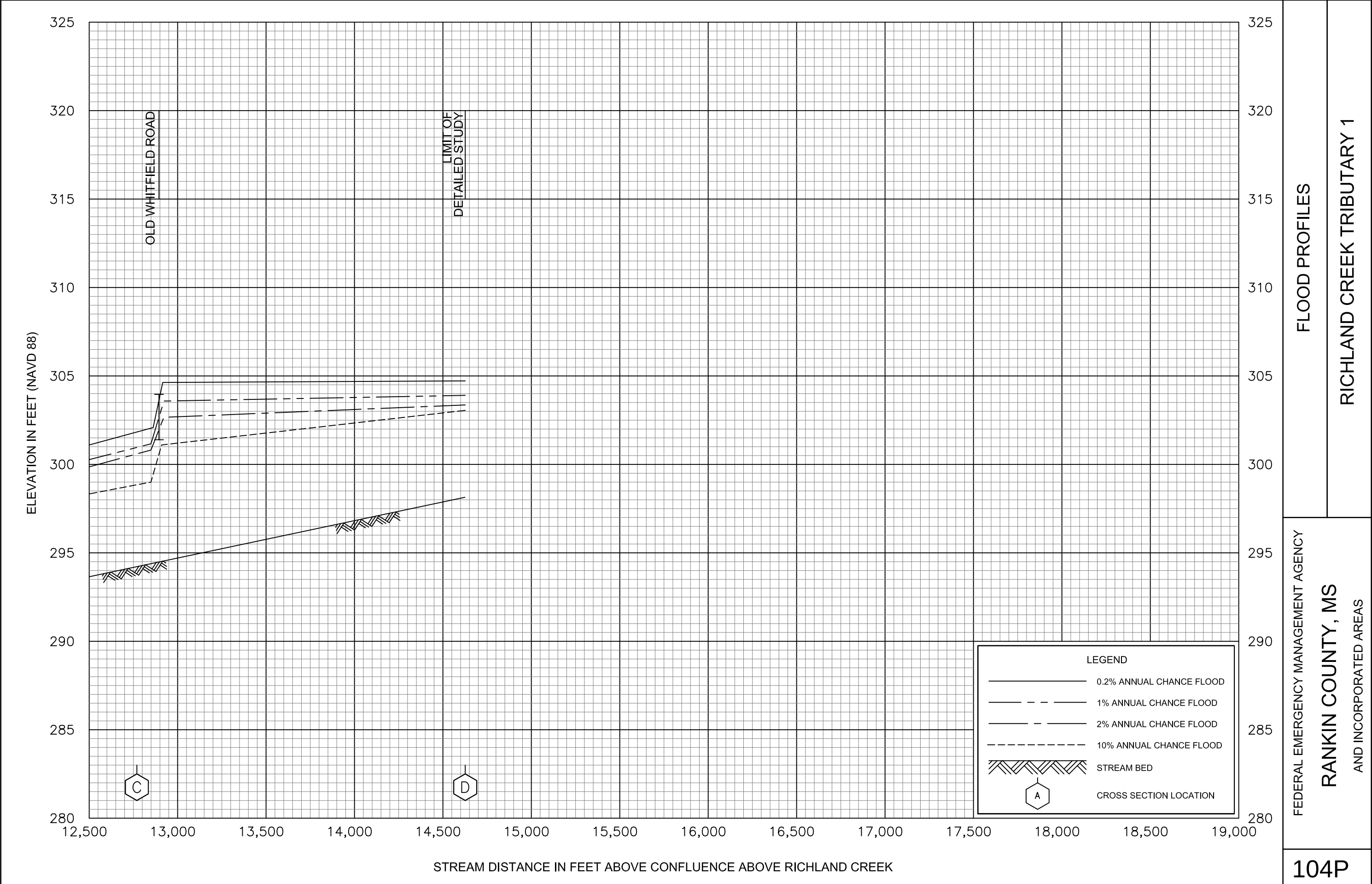
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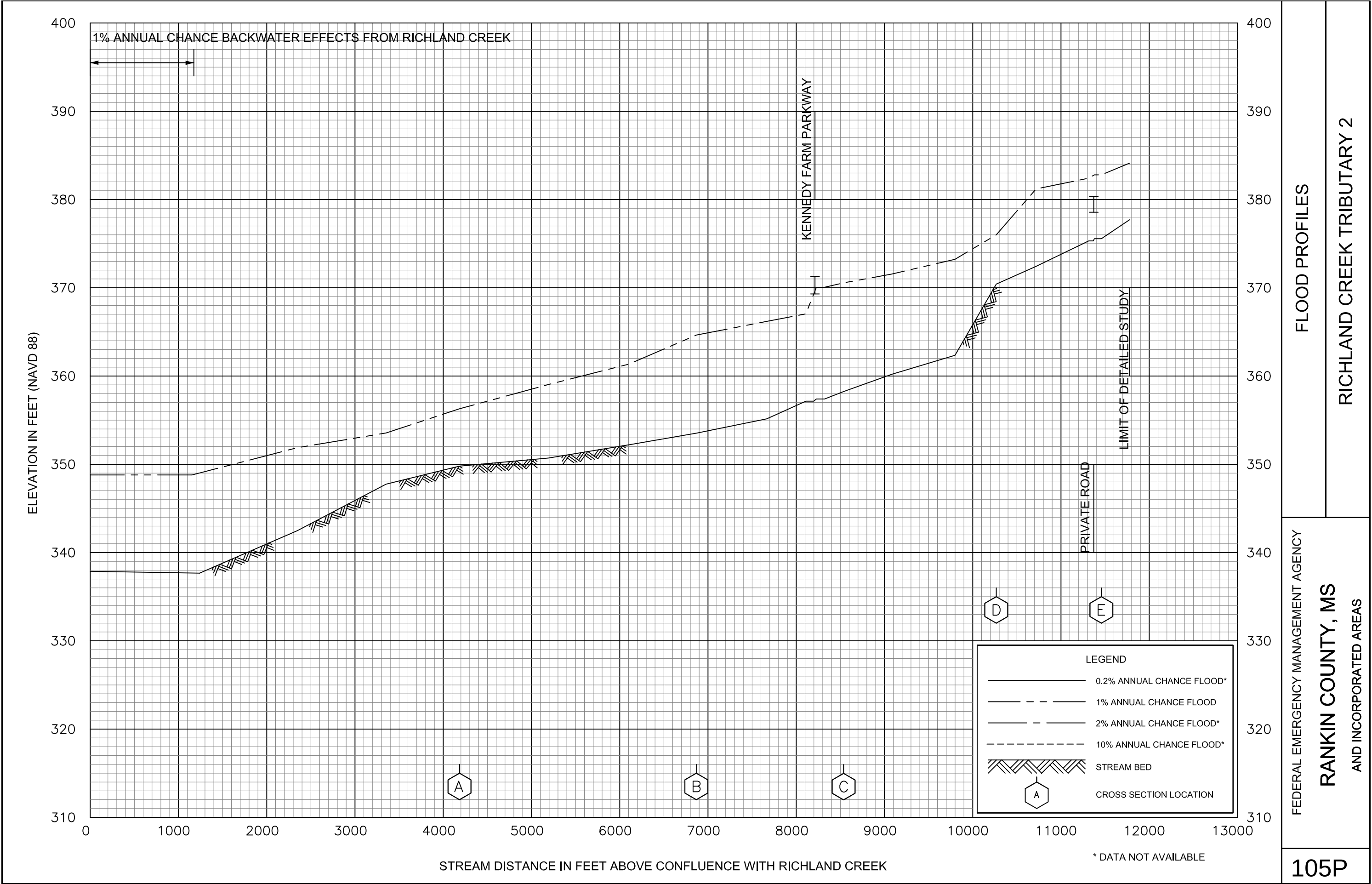
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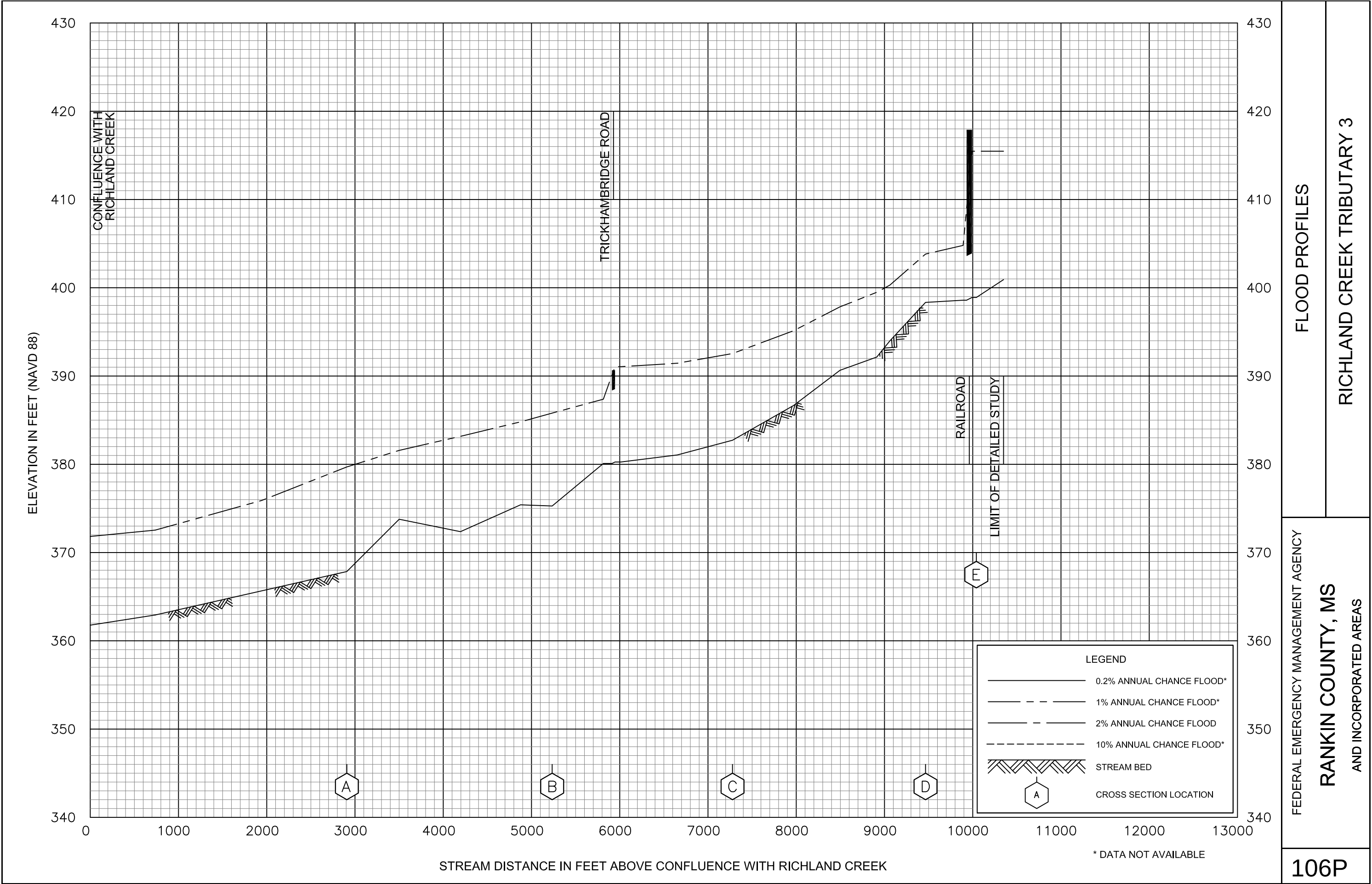


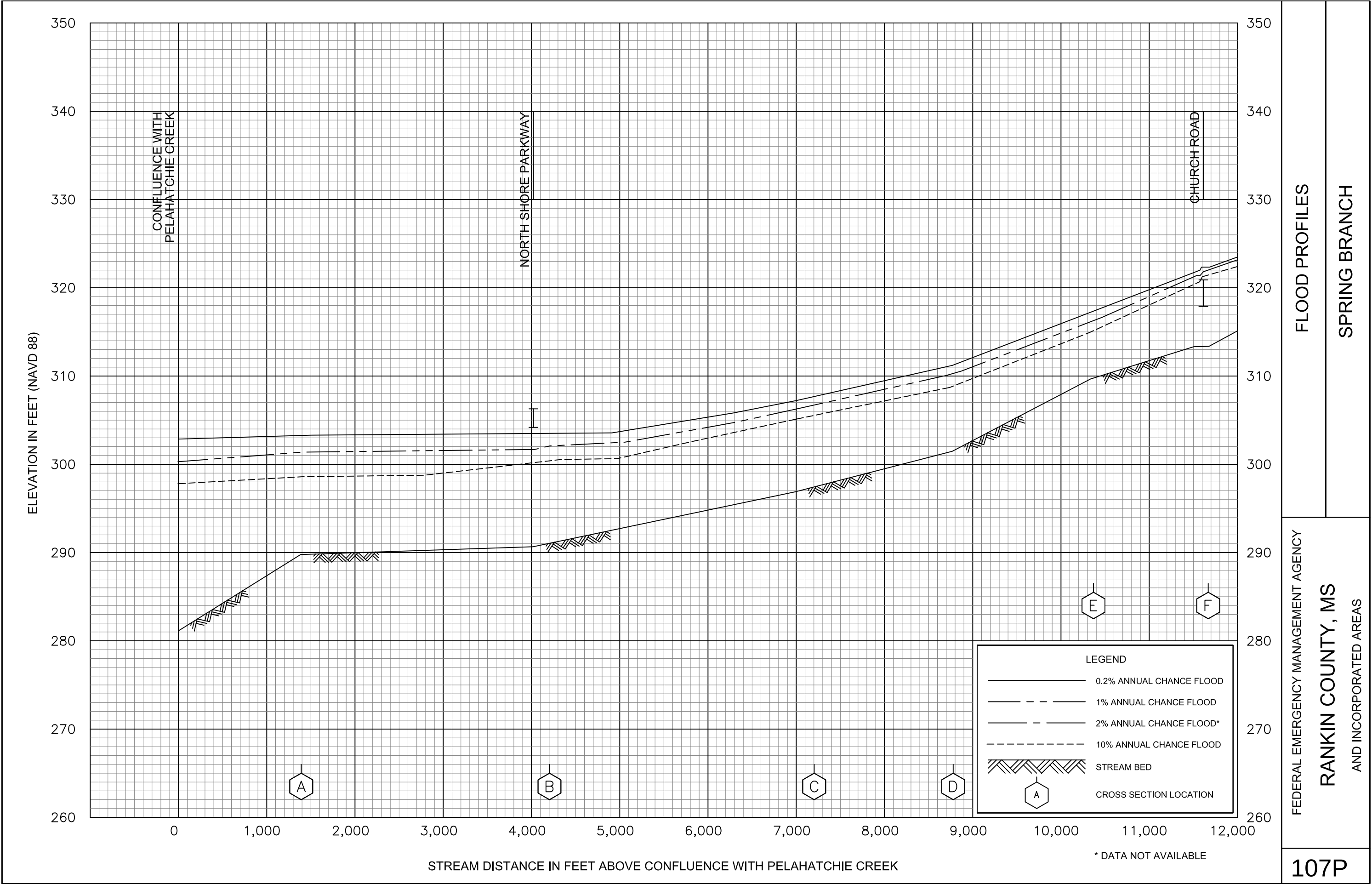
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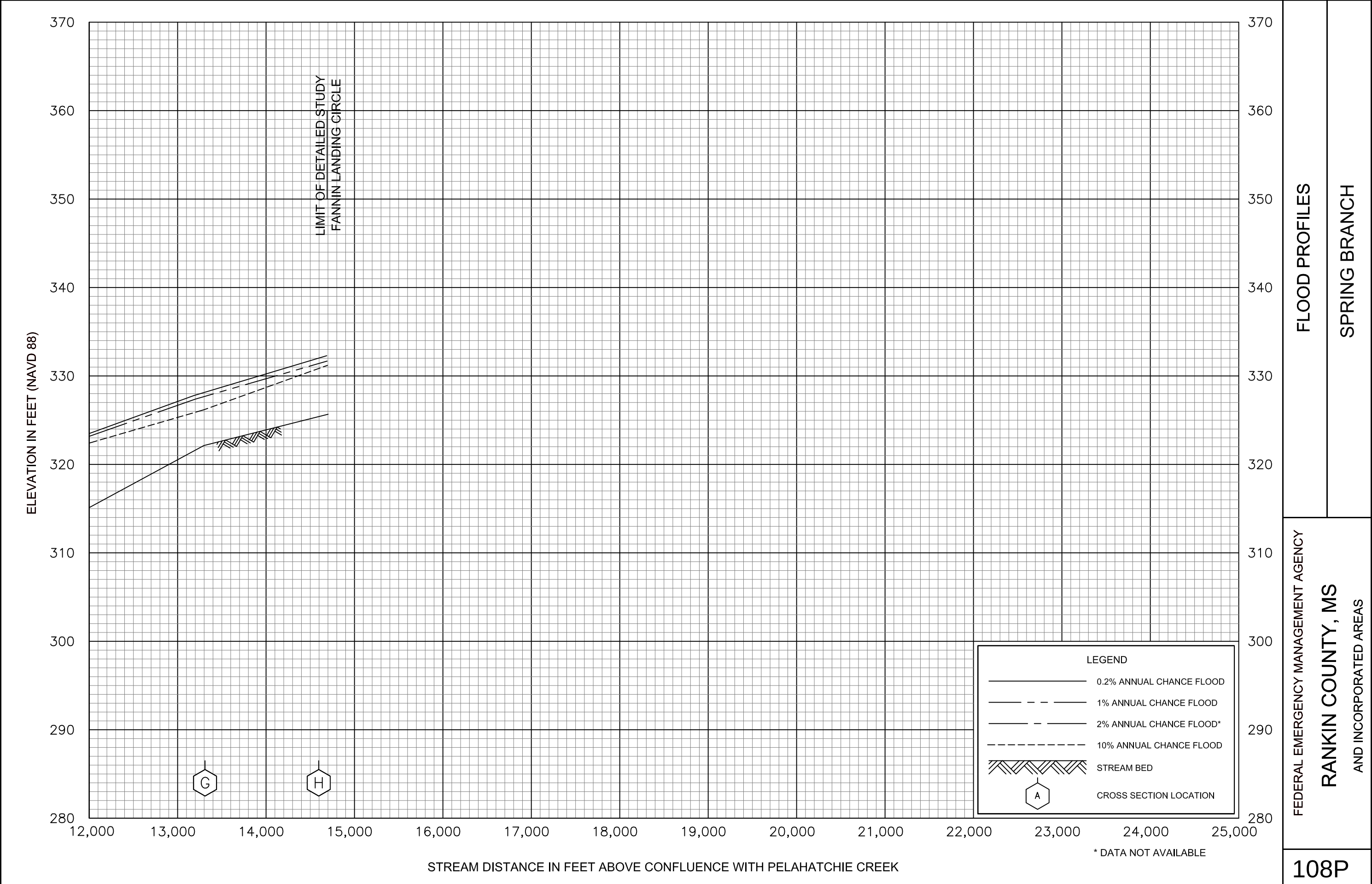
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RICHLAND CREEK TRIBUTARY 2







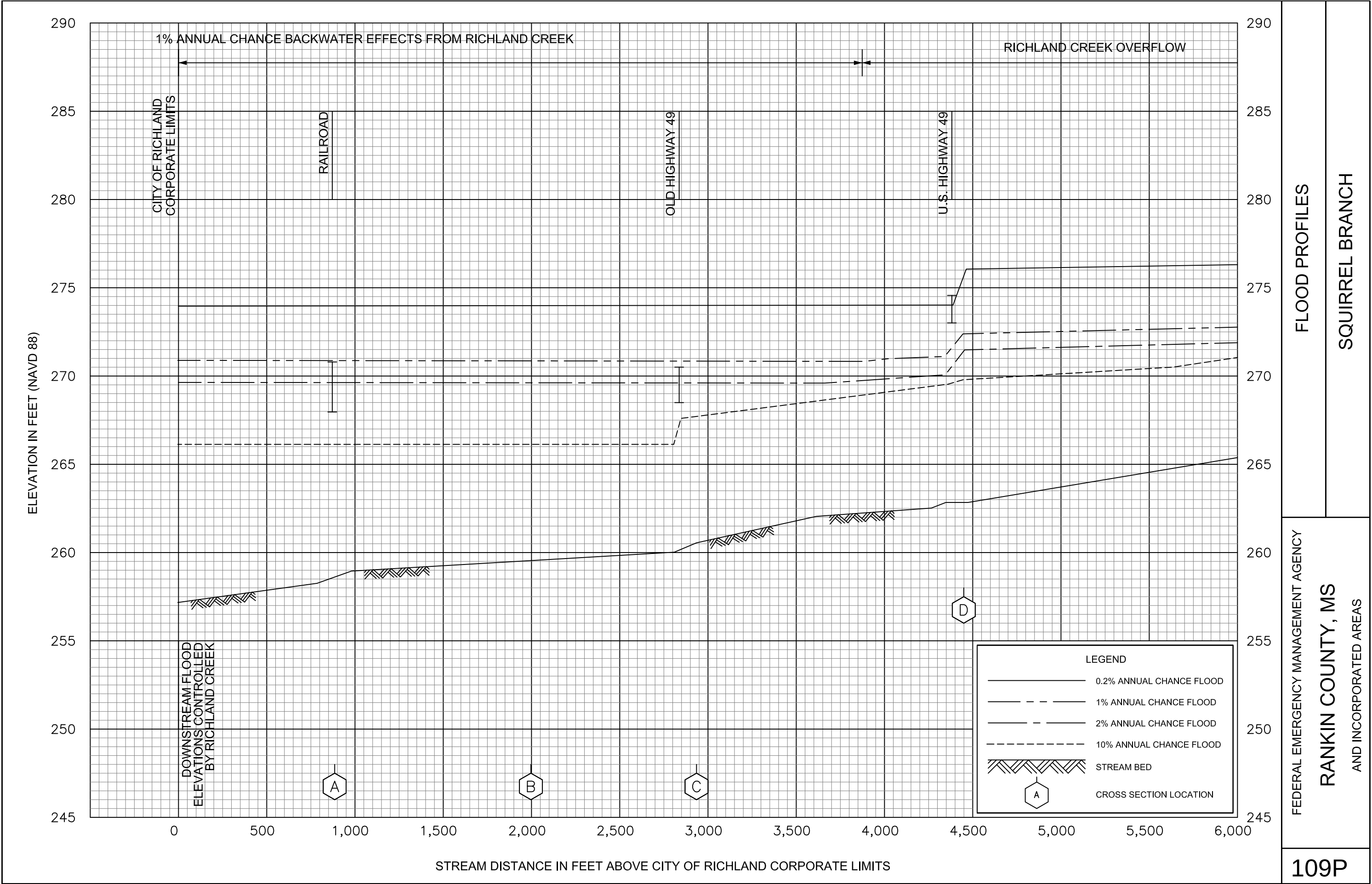
FLOOD PROFILES

SPRING BRANCH

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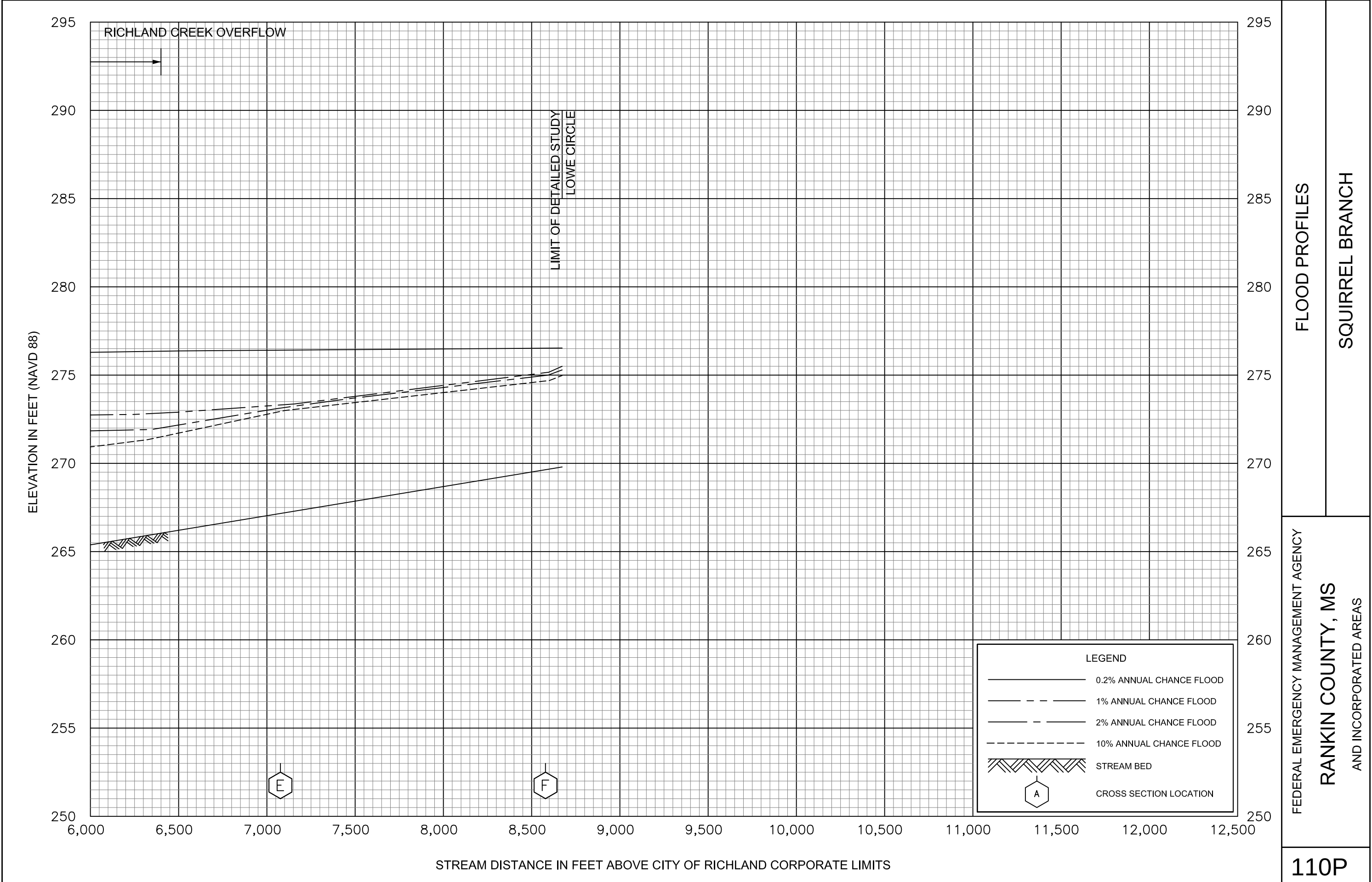
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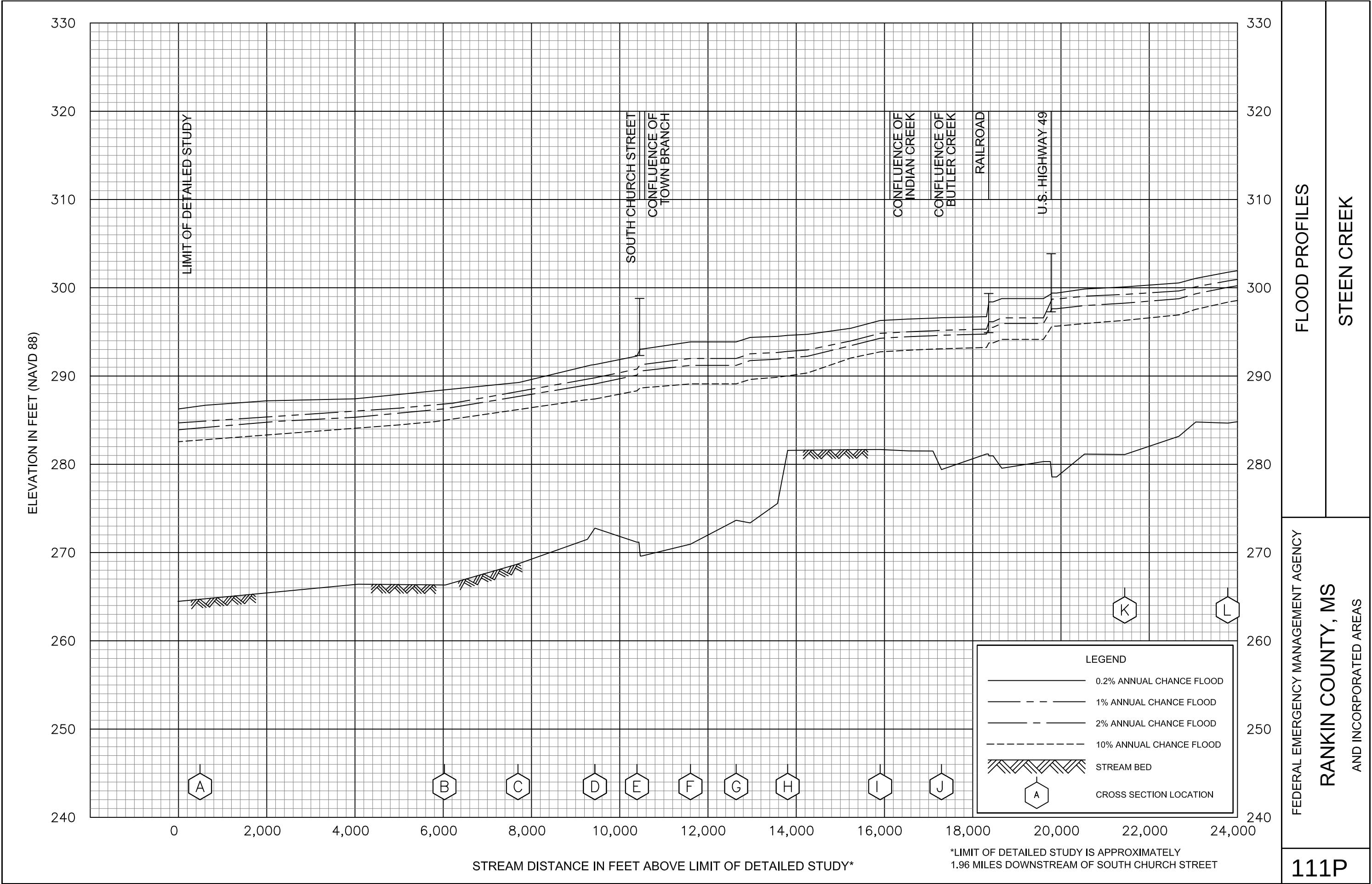
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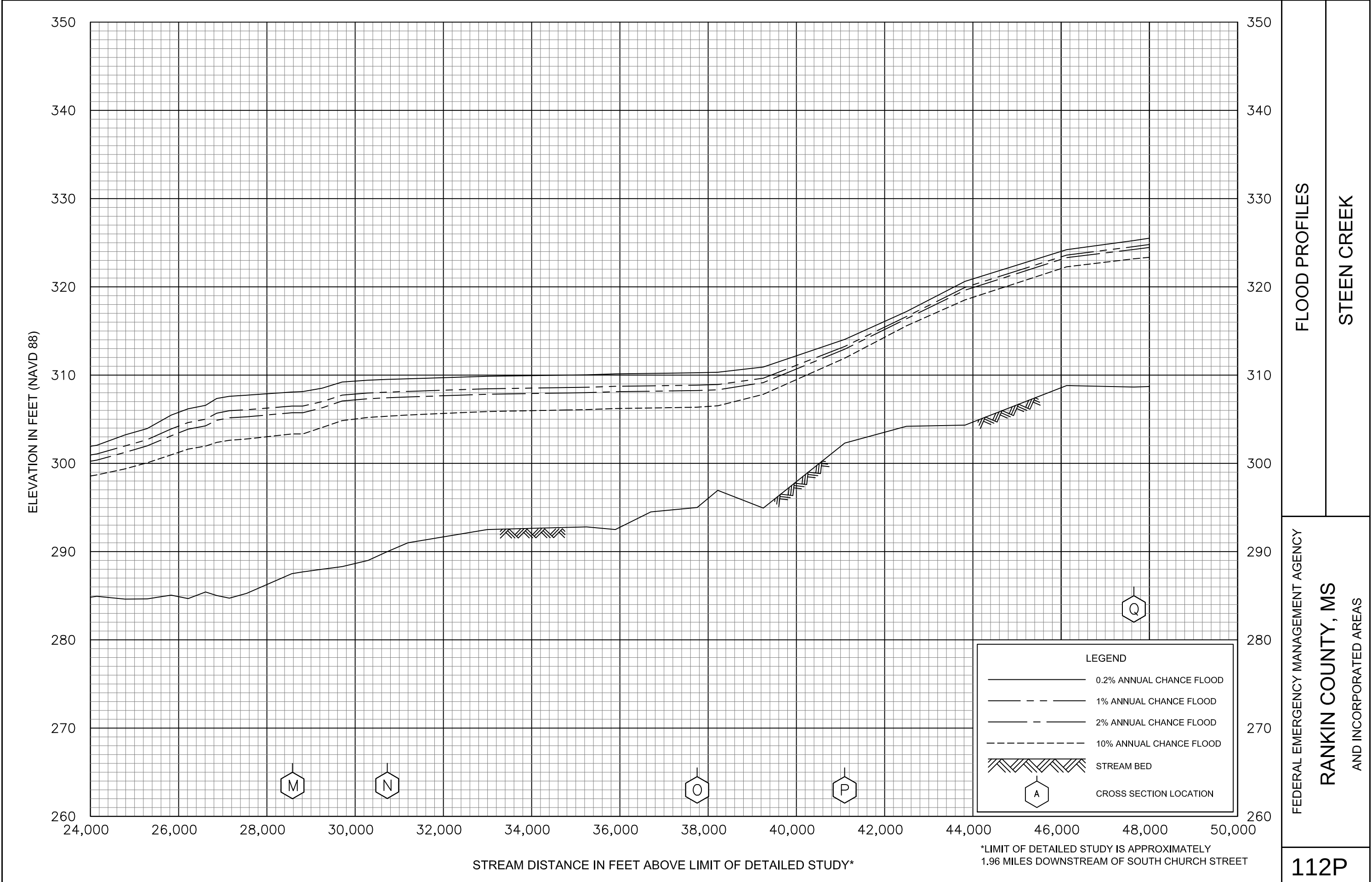
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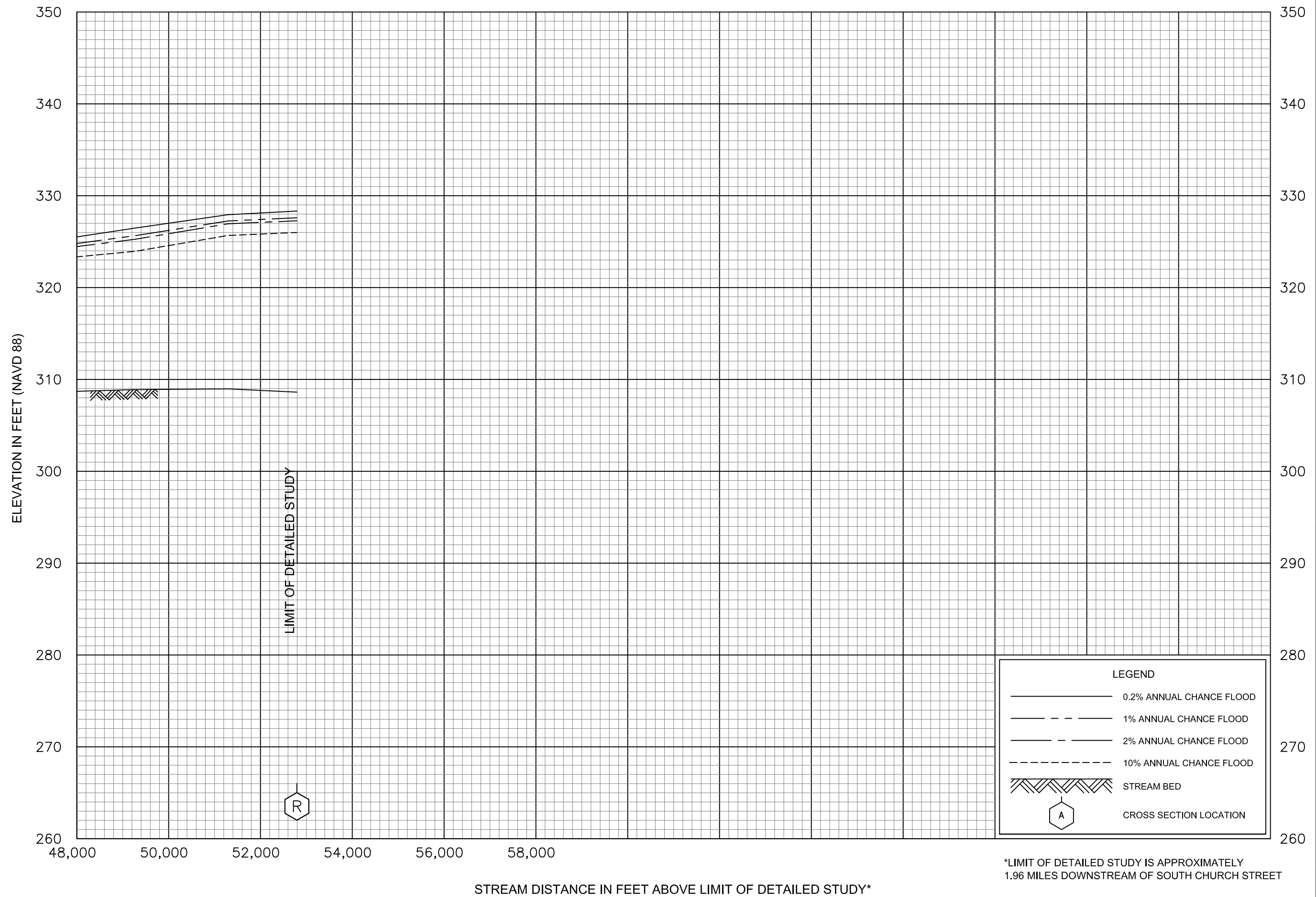
FLOOD PROFILES

STEEN CREEK

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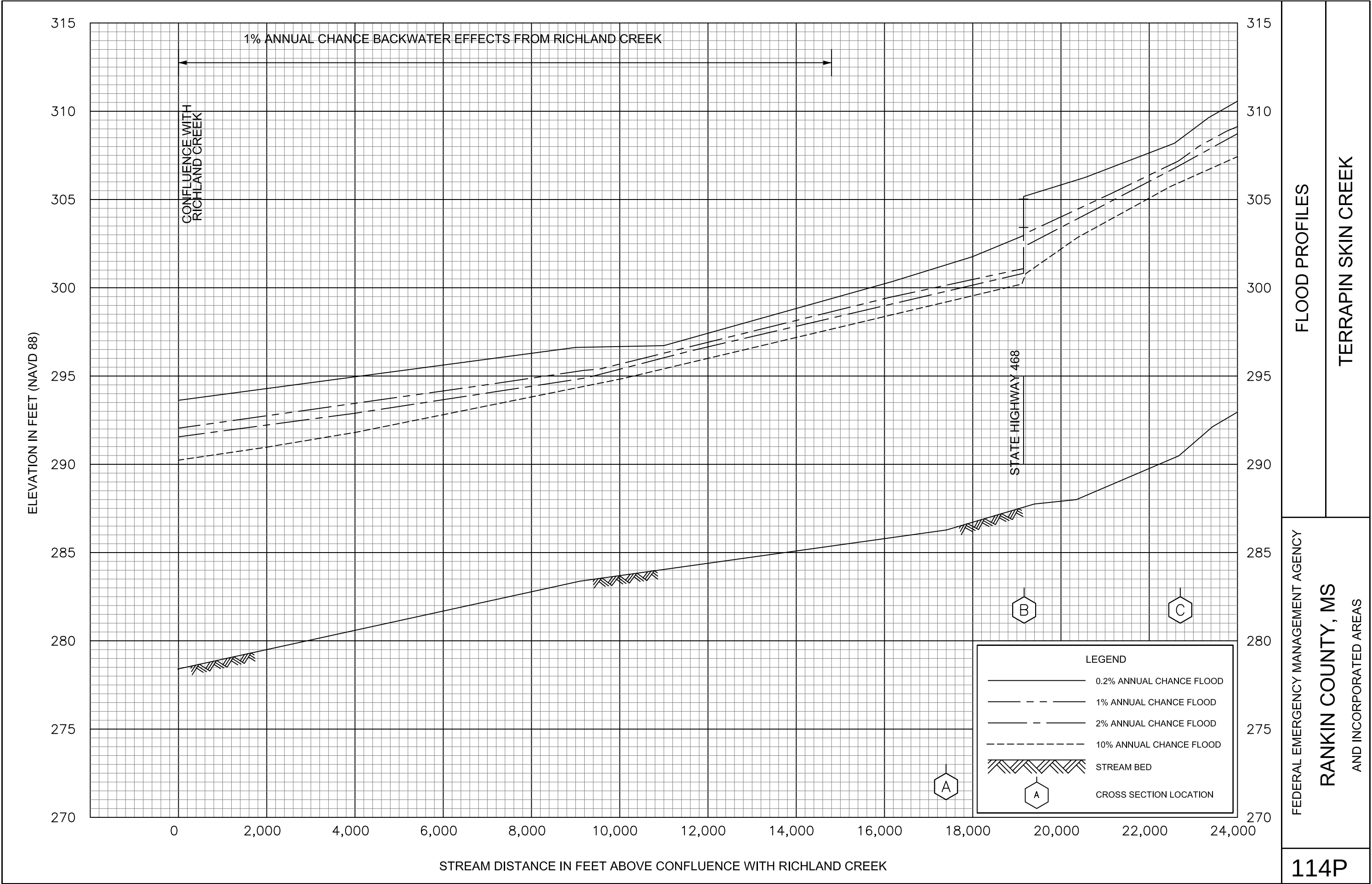
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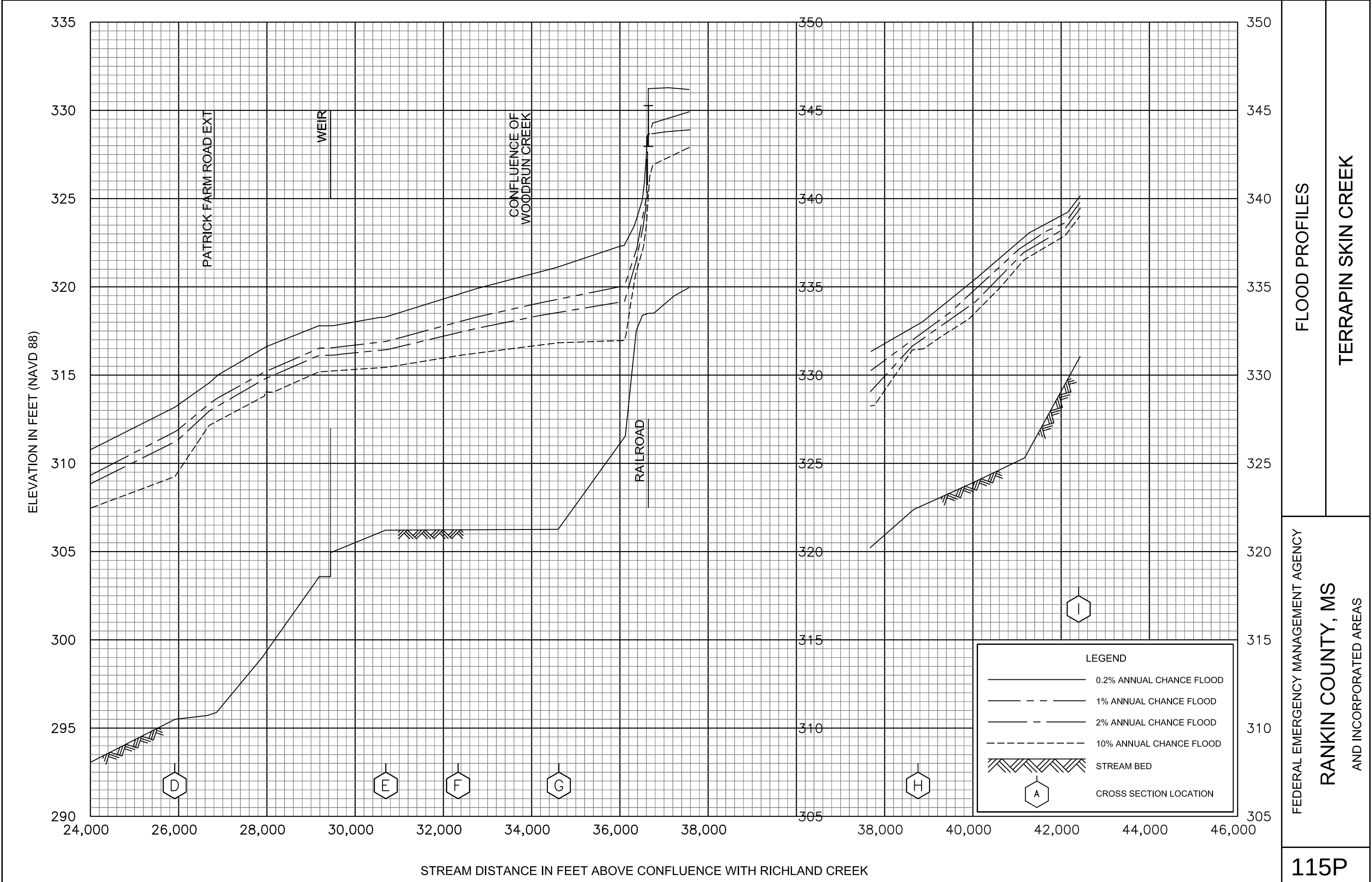
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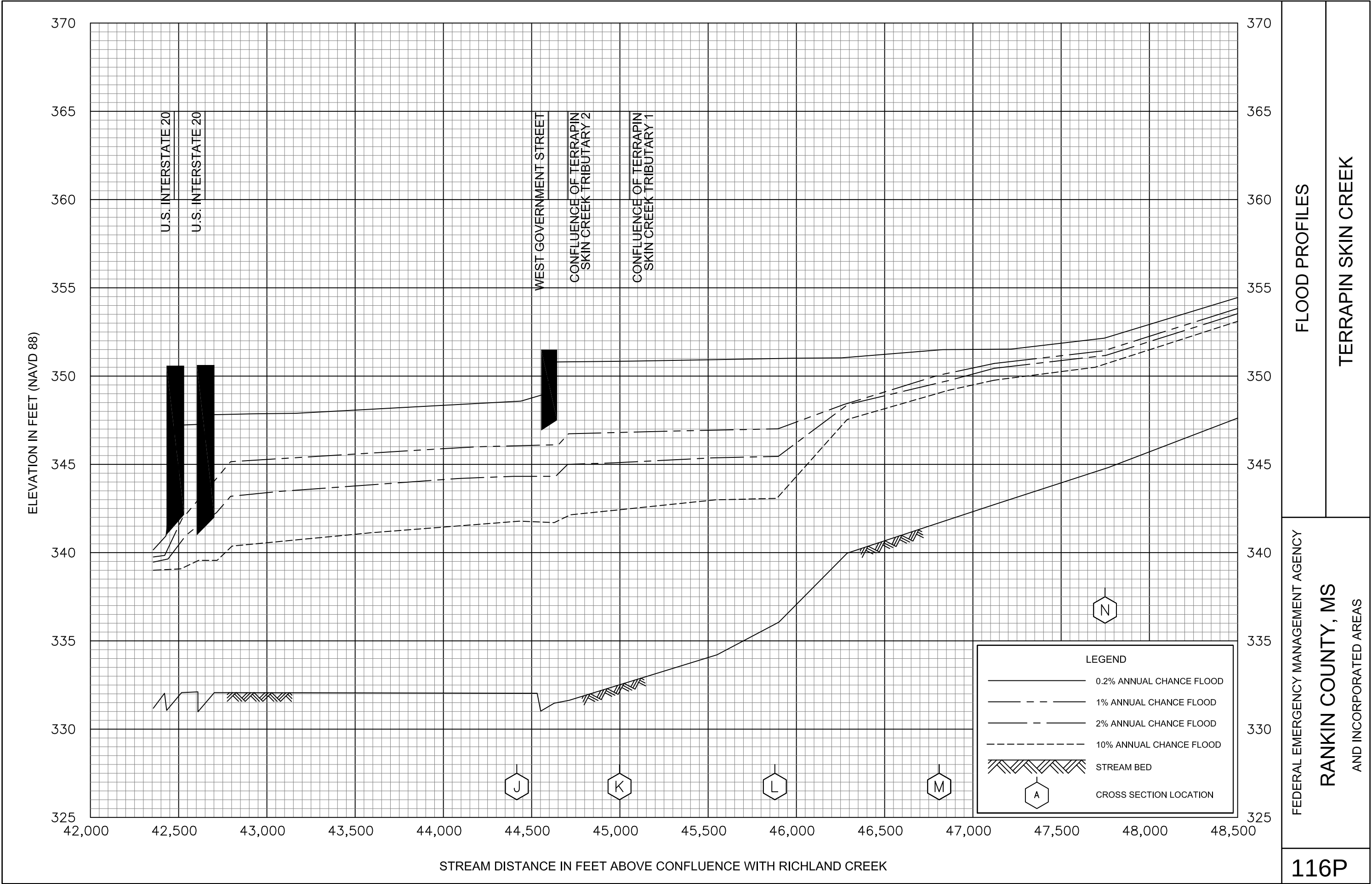
FLOOD PROFILES

STEEN CREEK

113P





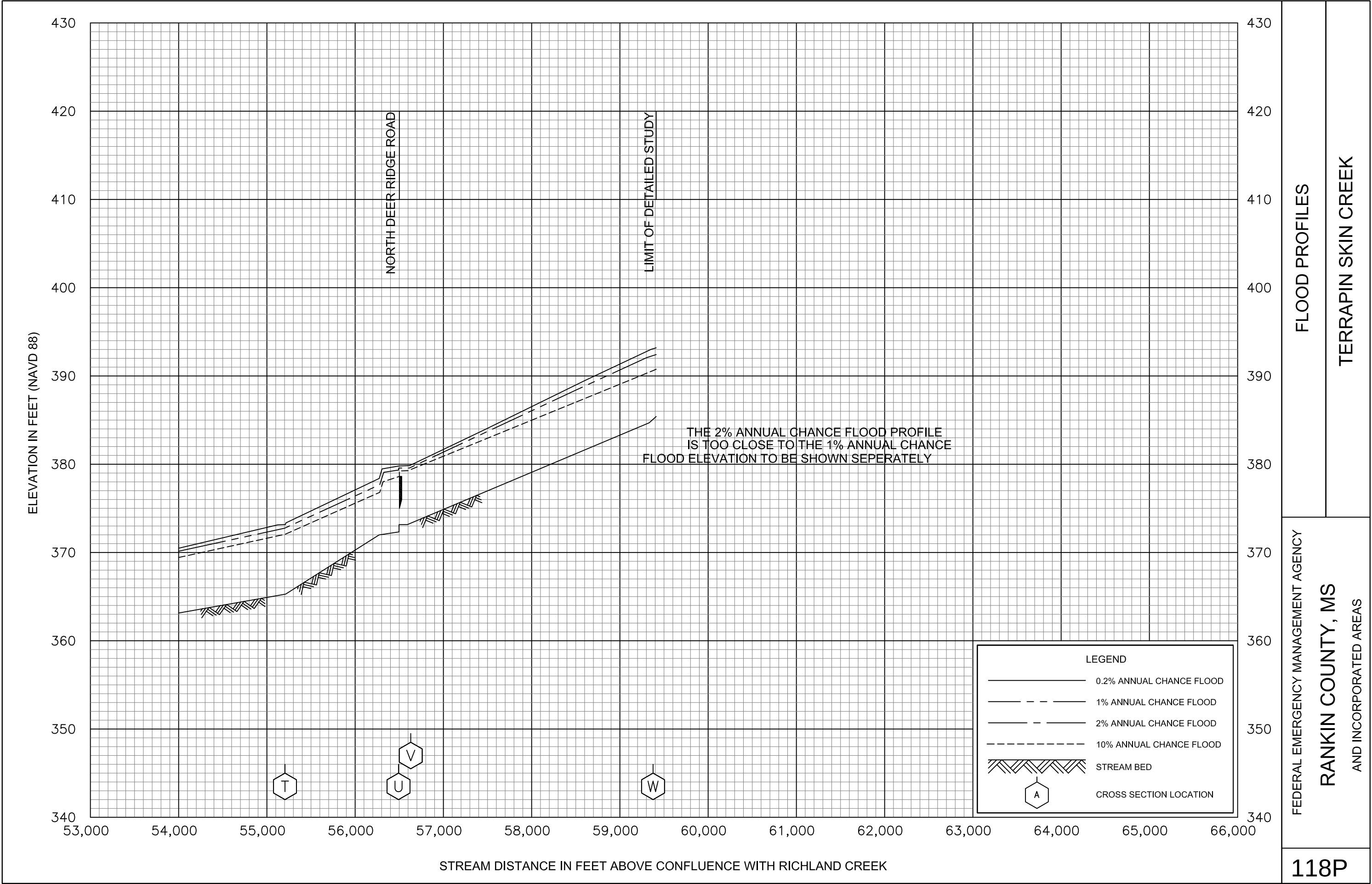


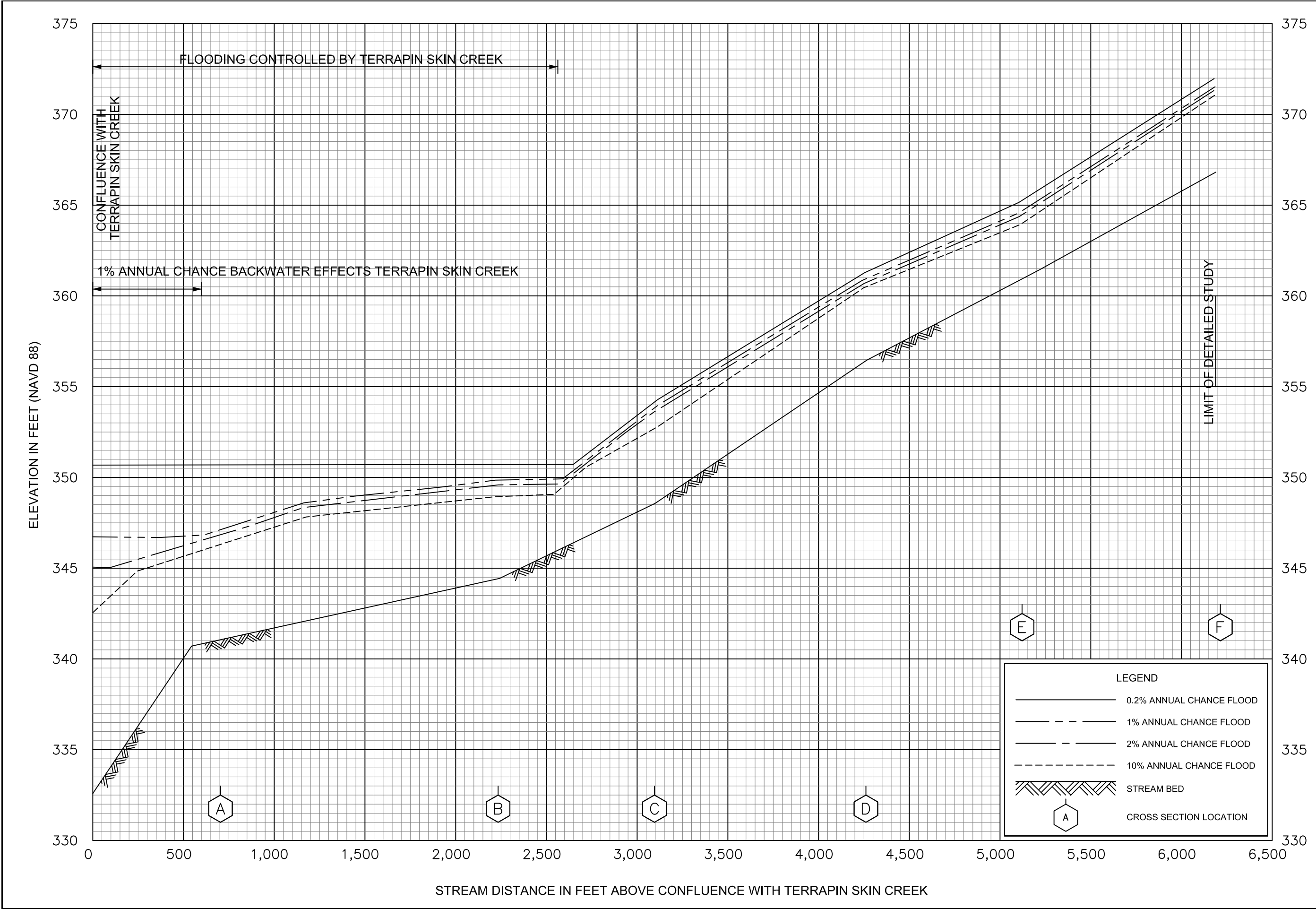
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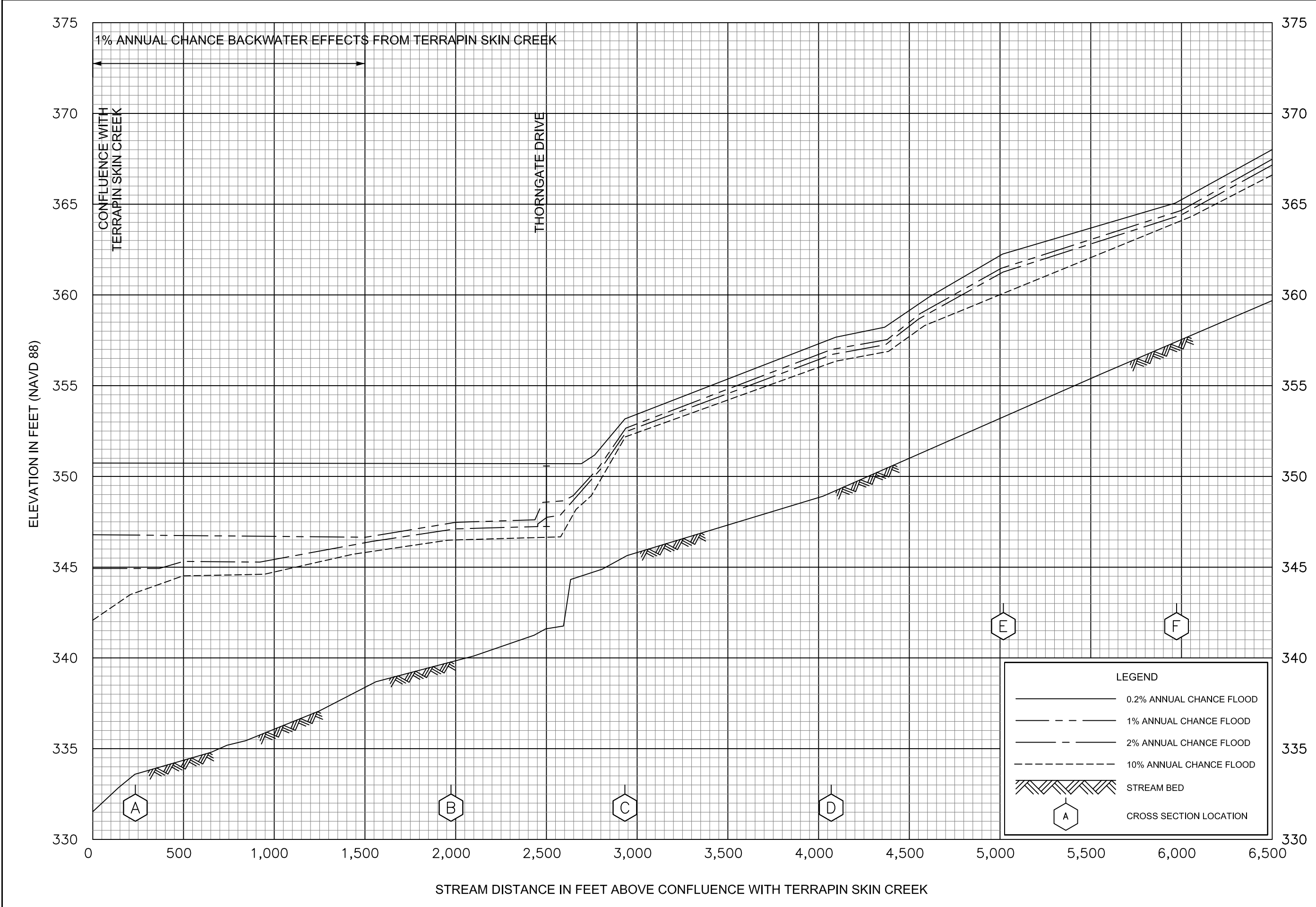
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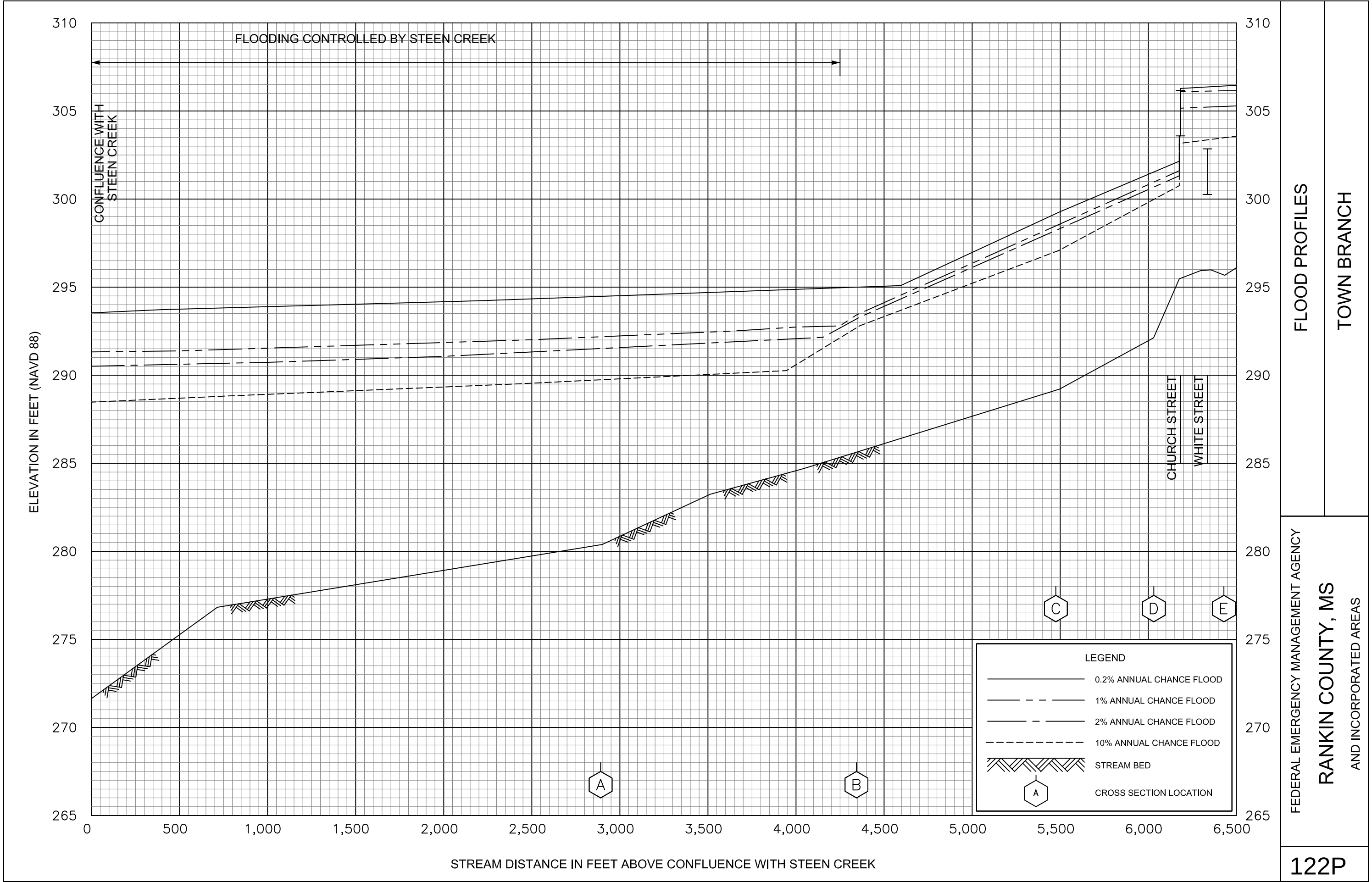
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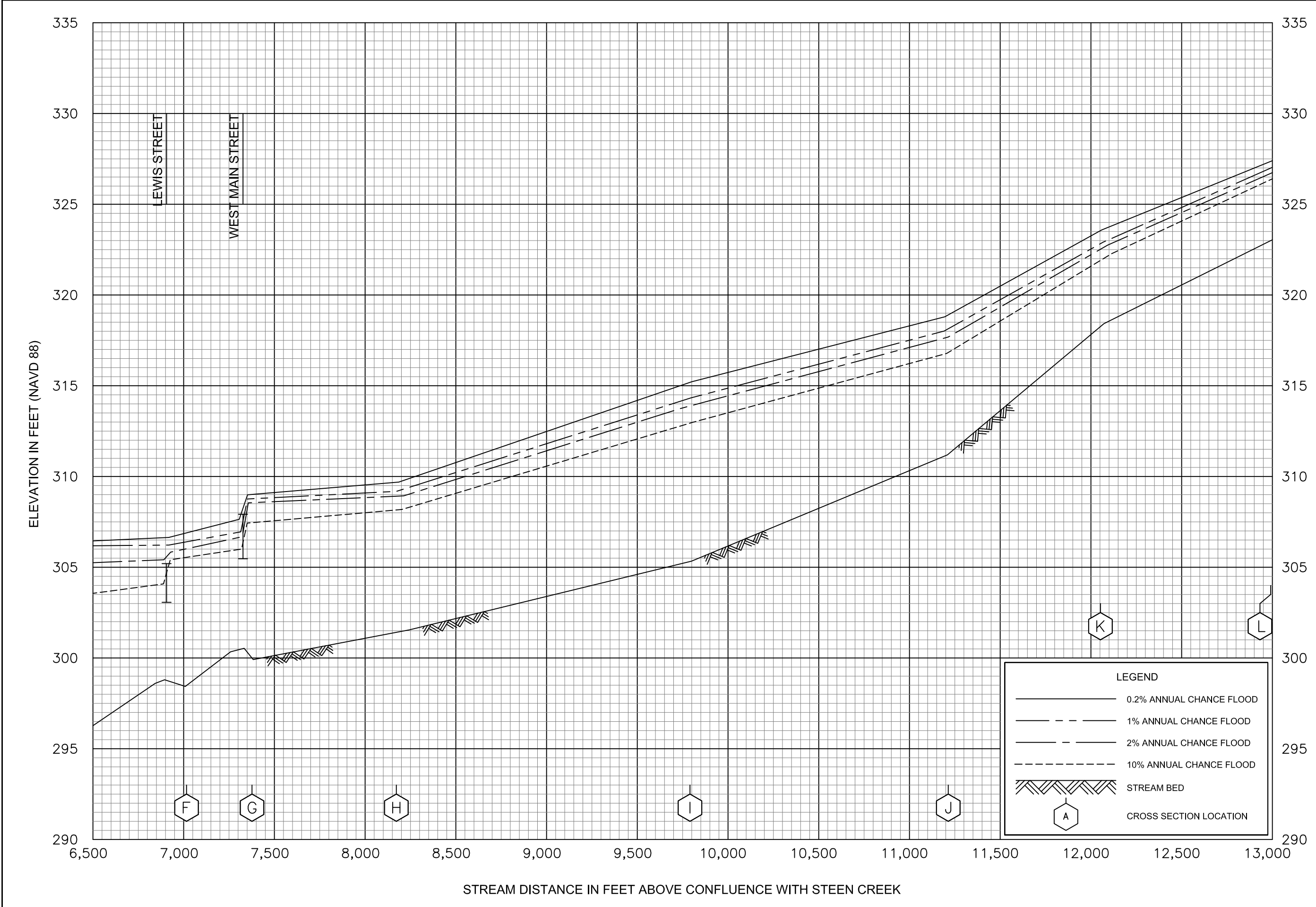
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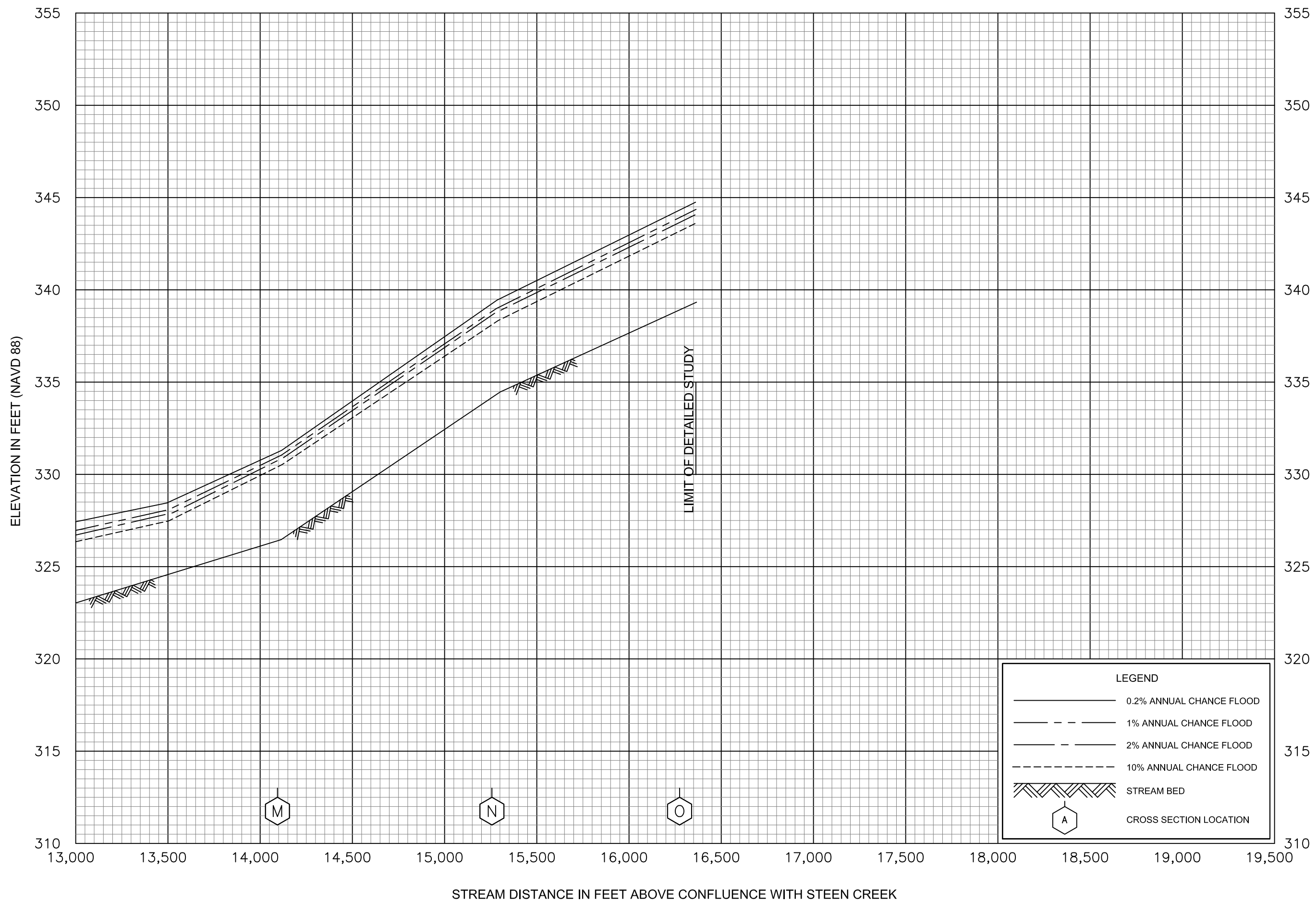












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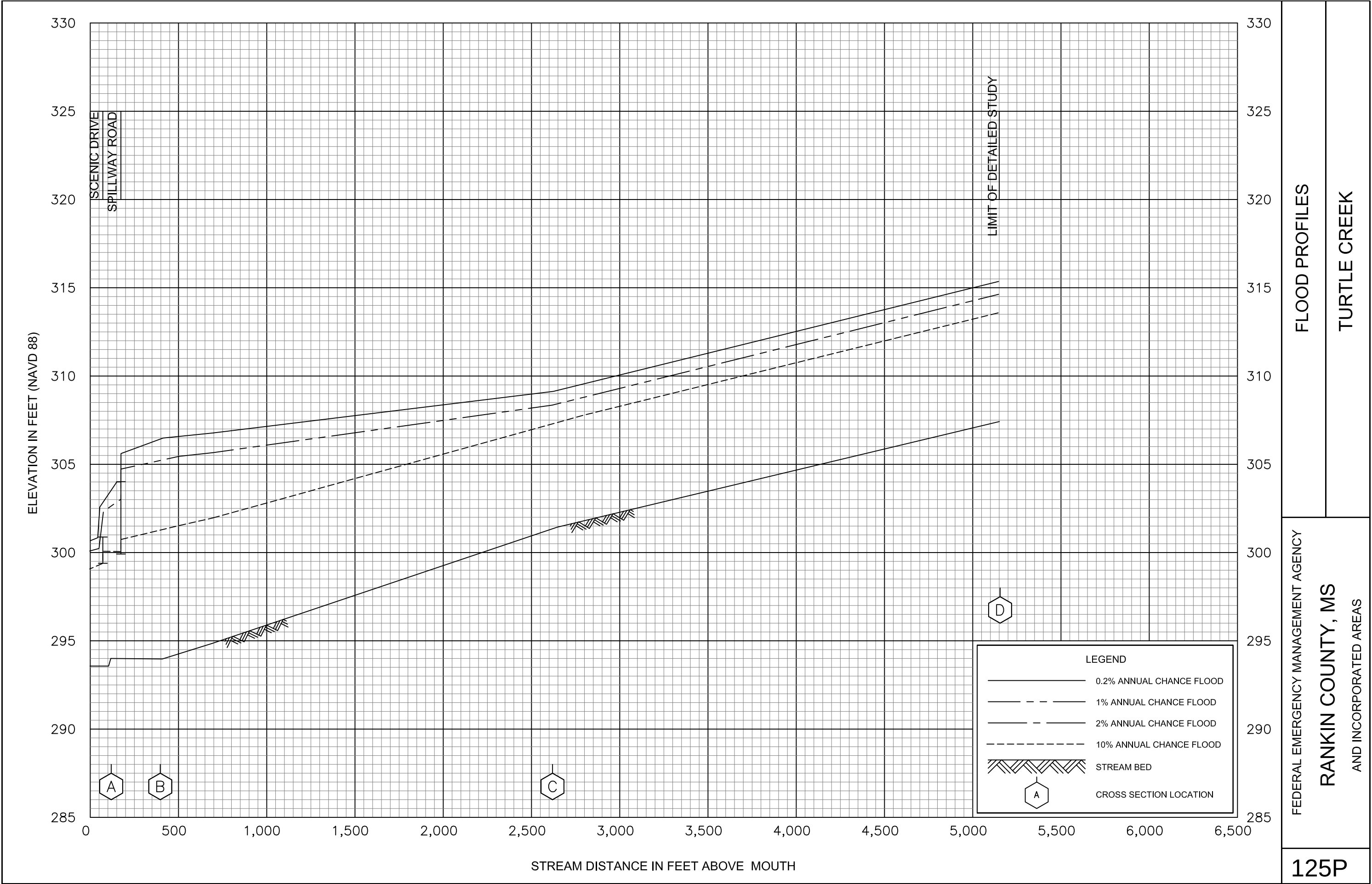
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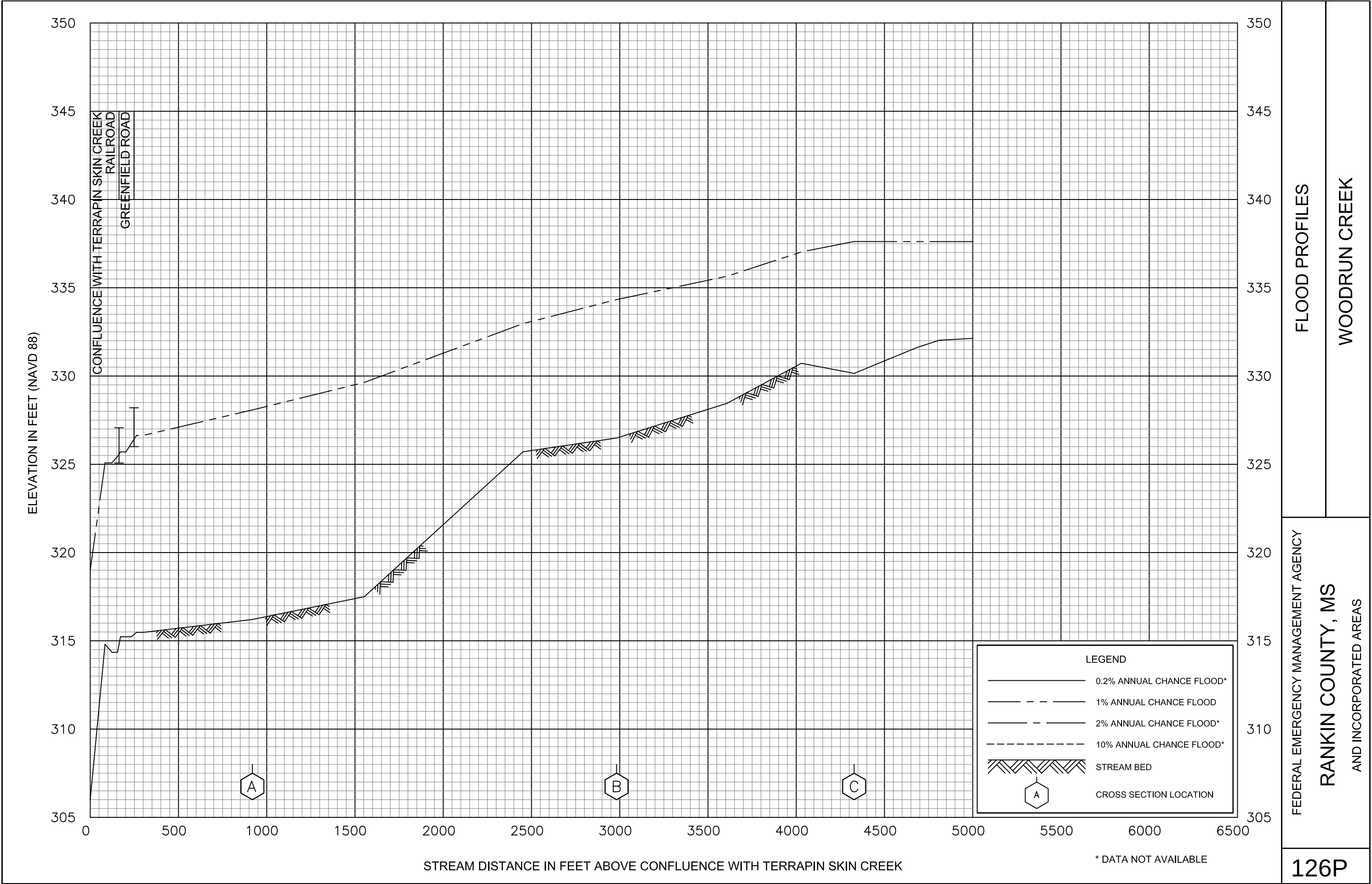
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FLOOD PROFILES

TOWN BRANCH

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FLOOD PROFILES

WOODRUN CREEK

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RANKIN COUNTY, MS

AND INCORPORATED AREAS

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